THERMAL TREATMENT TECHNOLOGY CONCEPTUAL DESIGN

McCormick and Baxter Superfund Site Stockton, California

November 12, 2001

Prepared for



U.S. Environmental Protection Agency Region 9 75 Hawthorne Street San Francisco, California 94105-3901

Prepared by



U.S. Army Corps of Engineers Seattle District 4735 East Marginal Way South Seattle, Washington 98134



1501 4th Avenue, Suite 1400 Seattle, Washington 98101-1616 53F0074206.02



CONTENTS

AB	BREVIA	ΓIONS AND ACRONYMS		xi
1.0	INTROI	DUCTION		1-1
	1.1	PURPOSE OF DOCUMENT		1-1
	1.2	PROJECT DESCRIPTION		1-1
	1.3	OVERALL PROJECT ASSUMPTION	íS	1-2
	1.4	TECHNOLOGY CHOICE		1-2
2.0	BACKG	ROUND AND OBJECTIVES		2-1
	2.1	PERTINENT HISTORY AND SITE C	HARACTERISTICS	2-1
	2.2	REMEDIAL ACTION OBJECTIVES		2-4
	2.3	CRITERIA FOR SELECTION OF TH	ERMAL TREATMENT AREAS	2-5
	2.4	DEFINITION OF TREATMENT VOL	LUMES AND SCENARIOS	2-5
		2.4.1 Treatment Scenarios		2-6
		2.4.2 Treatment Volumes		2-6
	2.5	COMBINED THERMAL TREATMEN	NT AND PUMP-AND-TREAT	2-8
3.0	SUBSUI	RFACE REMEDIAL DESIGN		3-1
	3.1	WELL-FIELD AND VOLUMES		3-1
		3.1.1 Scenario 1		3-2
		3.1.2 Scenario 2		3-3
		3.1.3 Scenario 3		3-3
	3.2	COMBINED STEAM AND ELECTRI	CAL RESISTANCE HEATING	
		BOREHOLES		3-4
		3.2.1 Materials		3-4
		3.2.2 Electrical Isolation		3-5
	3.3	STEAM INJECTION AND MIGRATI	ON	3-7
		3.3.1 Steam Injection Intervals		3-7
		3.3.2 Steam Injection Rates		3-8
		3.3.3 Radius of Influence and Choice	of Well Spacing	3-9
		3.3.4 Predicted Steam Breakthrough	for Each Depth Interval	3-10
	3.4	ELECTRICAL RESISTANCE HEATI	NG FOCUS	3-12
		3.4.1 Electrical Heating Intervals and	Areas	3-12
		3.4.2 Power Injection Rates		3-13
		3.4.3 In Situ Steam Generation and N	APL Displacement	3-14
	3.5	EXTRACTION APPROACH	-	3-15
		3.5.1 Liquid Extraction		3-15
		3.5.2 Vapor Extraction		3-17

3.6	SUBS	URFACE MONITORING	3-17
	3.6.1	ERT Monitoring	3-18
	3.6.2	Thermal Monitoring	3-19
	3.6.3	Groundwater COC Monitoring	3-20
3.7	OLD N	MORMON SLOUGH PROTECTION AND REMEDIATION	3-20
	3.7.1	Dewatering Option	3-21
	3.7.2	Thermal Treatment Under Slough	3-21
	3.7.3	Merits of Sediment Cap Installation in Slough Before or After Therm	nal
		Treatment	3-22
	3.7.4	Monitoring and Safety Features of Design	3-24
3.8	SURF.	ACE CAP	3-25
	3.8.1	Cap Options	3-25
	3.8.2	Recommendations	3-25
3.9	HYDR	AULIC AND PNEUMATIC CONTROL	3-26
	3.9.1	Net Liquid Extraction From Treatment Volumes	3-26
	3.9.2	Vapor Extraction and Pneumatic Control	3-27
	3.9.3	Mobile NAPL Extraction From Outside Treatment Volumes	3-27
3.10	SIZIN	G STEAM, POWER, AND EFFLUENT TREATMENT SYSTEMS	3-28
	3.10.1	Treatment Volumes and Heat Capacity Calculations	3-28
	3.10.2	Well and Electrode Spacing	3-30
	3.10.3	Steam Injection and Electrical Resistance Heating Rates	3-30
	3.10.4	Extraction Rates and Control	3-31
3.11	TREA	TMENT OF SHALLOW SOILS UNDER ASPHALT CAP IN MPA.	3-32
	3.11.1	Properties of Shallow Soils	3-32
	3.11.2	Underlying Layers and Potential for Steam Penetration From Depth.	3-32
	3.11.3	Design Options for Shallow Thermal Treatment	3-33
SURFAC	E PRO	CESS DESIGN	4-1
4 1	STEA	M GENERATION	4-1
	411	Steam Purchased From Outside Source or Service Provider	4-1
	412	Steam Generation Using Purchased or Rented Boiler	4-2
	413	Boiler Type Selection	4-2
	414	Fuel Source	4-2
	415	Fresh Water Source	4-3
	4.1.6	Feed Water Treatment Systems	
	4.1 7	Electrical Cogeneration Option	4-5
4.2	ABOV	EGROUND STEAM AND AIR CONVEYANCE	
	4.2.1	Steam Injection Pressures	4-6

4.0

	4.2.2	Steam Quality	
	4.2.3	Air Injection	
	4.2.4	Piping Systems and Control	
4.	3 ELEC	TRICAL HEATING POWER SUPPLY AND DELIVERY TO	
	ELEC	TRODES	
4.	4 CONT	AMINATED LIQUIDS AND VAPOR EXTRACTION AND	
	CONV	/EYANCE	
	4.4.1	Wellhead Design and Down-Hole Pumps	
	4.4.2	Liquid Conveyance	
	4.4.3	Vapor Conveyance	
4.	5 EFFLU	UENT TREATMENT SYSTEM	4-10
	4.5.1	Effluent Streams and Composition	4-10
	4.5.2	Treatment System Components	4-10
	4.5.3	Cooling and Condensation	4-10
	4.5.4	Liquid-Vapor Separation	4-11
	4.5.5	NAPL Removal From Water	4-11
	4.5.6	Dissolved-Phase Treatment Alternatives	4-12
	4.5.7	Vapor Treatment Alternatives	4-13
4.	6 UTILI	TY REQUIREMENTS	4-15
	4.6.1	Power	4-15
	4.6.2	Fuel	4-16
	4.6.3	Water	4-16
4.	7 WAST	TE GENERATION AND DISPOSAL	4-17
	4.7.1	Solids From Subsurface	4-17
	4.7.2	Nonaqueous-Phase Liquids	4-17
	4.7.3	Spent Activated Charcoal	4-17
	4.7.4	Personal Protective Equipment	4-18
	4.7.5	Other Solid Waste	4-18
4.	8 SITE 1	INFRASTRUCTURE AND LAYOUT	4-18
5.0 PRO	CESS CON	TROL. OPERATIONS. AND MAINTENANCE	
5.	1 OPER	ATIONAL STRATEGY	
	5.1.1	Overall Operational Goals	
	5.1.2	Phases of Operation	
	5.1.3	Prevention of NAPL Spread	
	5.1.4	Injection of Air and Oxygenated Water	
5.	2 STEA	M AND AIR DELIVERY	
	5.2.1	Temperature and Pressure Regulation	

	5.2.2	Injection Rate Control and Measurement	
	5.2.3	Safety Measures	5-5
5.3	ERH I	PROCESS CONTROL, OPERATIONS, AND MAINTENANCE	5-5
	5.3.1	ERH Process Control	5-5
	5.3.2	ERH Operations	5-6
	5.3.3	ERH Maintenance	
5.4	CONT	TAMINANT EXTRACTION	5-8
	5.4.1	Controlling Liquid Levels in Wells	5-8
	5.4.2	Metering Extraction Rates and Fluid Properties	5-8
	5.4.3	Hydraulic Control	
	5.4.4	Pneumatic Control	5-9
5.5	TREA	TMENT PLANT OPERATIONS	5-9
	5.5.1	Cooling Efficiency and Control	5-9
	5.5.2	Phase Separation Efficiency and Control	5-10
	5.5.3	Water Treatment Efficiency and Control	5-10
	5.5.4	Vapor Treatment Efficiency and Control	5-10
	5.5.5	Adding or Removing Treatment Units During Operation	5-10
5.6	BASI	C MAINTENANCE REQUIREMENTS	5-11
5.7	DURA	ATION OF THERMAL TREATMENT	5-11
	5.7.1	Criteria for Discontinuing Steam and Power Injection in Areas/Zo	ones 5-11
	5.7.2	NAPL Removal Rates	5-12
	5.7.3	Dissolved Total Organic Carbon and COC Removal Rates	5-13
	5.7.4	In Situ Destruction Rate Evaluation	5-13
	5.7.5	Redox Level Measurements and Air Injection Strategy	5-13
	5.7.6	General Considerations Regarding Steam Pore Volumes	5-14
	5.7.7	Cool-Down Options and Stimulation of In Situ Destruction React	tions 5-15
	5.7.8	Estimates of Total Operation Time	5-16
PERFOR	MANC	E AND COMPLIANCE MONITORING	6-1
6.1	TEMP	PERATURE AND STEAM DISTRIBUTION MONITORING	6-1
6.2	SUBS	URFACE CONTAMINANT REMOVAL RATE MONITORING.	
6.3	REME	EDY EFFECTIVENESS MONITORING	
6.4	BOILI	ER AIR EMISSIONS MONITORING	6-4
6.5	SITE	FUGITIVE EMISSIONS MONITORING	
6.6	TREA	TMENT PLANT DISCHARGE MONITORING	
6.7	WAST	FE DISPOSAL CHARACTERIZATION	
	6.7.1	NAPL Disposal Characterization	
	6.7.2	Solid Waste Characterization	

6.0

	6.8	SITE I	PERIMETER ENVIRONMENTAL MONITORING	
		6.8.1	Noise Monitoring	6-9
		6.8.2	Air Quality	6-9
	6.9	GROU	JNDWATER MONITORING	
	6.10	SOIL	CONFIRMATION SAMPLING	6-11
7.0	DISCUS	SION O	F DESIGN	7-1
	7.1	EVAL	UATION OF LIQUID-PHASE GAC USAGE AND REGENERA	ATION7-1
	7.2	DISCI	HARGE OPTIONS FOR EXCESS TREATED WATER	
		7.2.1	Injection of Treated Water Into E-Zone	
		7.2.2	Discharge to Surface Water (Old Mormon Slough)	
		7.2.3	Discharge to City Wastewater Facility	
		7.2.4	Evaporation in Large Holding Ponds	
	7.3	POTE	NTIAL FOR RECYCLING RECOVERED NAPL FOR STEAM	
		GENE	RATION	7-4
		7.3.1	Destruction of NAPL as Supplemental Fuel	7-4
		7.3.2	Steam Generator Combustion Chamber Conditions	7-4
		7.3.3	Off-Gas Polishing Need	
	7.4	REVIS	SION OF TARGET VOLUMES DURING	
		DRILI	LING/INSTALLATION	
	7.5	OPPO	RTUNITIES FOR PHASING CONSTRUCTION AND OPERAT	ΓΙΟΝ 7 - 7
		7.5.1	Reason for Discussing Longer Spending Period	
		7.5.2	Description of Example Phasing Alternative	7-9
		7.5.3	Cost Implications of Phasing Example	7-13
8.0	REFERE	NCES .		

APPENDICES

- A Evaluation of Heavy Metals Treatment Need
- B Steam Injection Rate Modeling
- C Safe Injection Pressures Under Old Mormon Slough
- D Electrical Resistivity Tomography
- E Slough Dewatering Evaluation
- F Memorandum: Evaluation of Angled Drilling under Old Mormon Slough McCormick & Baxter Superfund Site, Stockton, CA

FIGURES

2-1	General Location Map	2-9
2-2	Detail of Site, Showing Historical Usage	. 2-10
2-3	South-to-North Section, Based on USACE Data	. 2-11
2-4	West-to-East Section, Based on USACE Data	. 2-12
2-5	Map of Main Treatment Zones, Showing Subdivisions and Abbreviations	. 2-13
2-6	Remediation Priorities Assigned to Each Treatment Zone	. 2-15
2-7	Approximate Depth Intervals of NAPL-Impacted Soil and Groundwater	. 2-17
2-8	Overview of Areas Treated in Each of Three Scenarios	. 2-18
2-9	Scenario 1: Areas of Thermal Treatment in Each of Five Aquifer Zones	. 2-19
2-10	Scenario 2: Areas of Thermal Treatment in Each of Five Aquifer Zones	. 2-20
2-11	Scenario 3: Areas of Thermal Treatment in Each of Five Aquifer Zones	. 2-21
3-1	Cross Section A-A' Showing Well Layout in Scenario 1	. 3-35
3-2	Cross Section A-A' Showing Well Layouts in Scenarios 2 and 3	. 3-37
3-3	Scenario 1: A-Zone Well-Field—53 Injection Wells, 2 Extraction Wells, 55 Total	. 3-39
3-4	Scenario 1: B-Zone Well-Field—54 Injection Wells, 69 A/B Extraction Wells,	
	123 Total	. 3-41
3-5	Scenario 1: C-Zone Well-Field—13 Injection Wells, 14 Extraction Wells, 27 Total.	. 3-43
3-6	Scenario 1: D-Zone Well-Field—35 Injection Wells (32 Collocated With	
	Electrodes), 24 Extraction Wells (13 Collocated With Electrodes, 59 Total)	. 3-45
3-7	Scenario 1: E-Zone Well-Field—10 Injection Wells, 4 Extraction Wells,	
	1 Dual-Purpose, 15 Total	. 3-47
3-8	Scenario 2: A-Zone Well-Field—57 Injection Wells, 2 Extraction Wells, 59 Total	. 3-49
3-9	Scenario 2: B-Zone Well-Field—73 Injection Wells, 64 A/B Extraction Wells	
	(9 Collocated With Electrodes), 19 B Extraction Wells, 156 Total	. 3-51
3-10	Scenario 2: C-Zone Well-Field—22 Injection Wells, 20 Extraction Wells, 42 Total.	. 3-53
3-11	Scenario 2: D-Zone Well-Field—35 Injection Wells (32 Collocated With	
	Electrodes), 27 Extraction Wells (13 Collocated With Electrodes), 62 Total	. 3-55
3-12	Scenario 2: E-Zone Well-Field—10 Injection Wells, 4 Extraction Wells,	
	1 Dual-Purpose, 15 Total	. 3-57
3-13	Scenario 3: A-Zone Well-Field—69 Injection Wells, 2 Extraction Wells, 1 Dual-	
	Purpose, 71 Total	. 3-59
3-14	Scenario 3: B-Zone Well-Field—85 Injection Wells, 87 A/B Extraction Wells	
	(9 Collocated With Electrodes), 19 B Extraction Wells, 191 Total	. 3-61
3-15	Scenario 3: C-Zone Well-Field—28 Injection Wells, 22 Extraction Wells, 50 Total.	. 3-63
3-16	Scenario 3: D-Zone Well-Field—55 Injection Wells (3 Collocated With	
a 1 -	Electrodes), 29 Extraction Wells (12 Collocated With Electrodes), 84 Total	. 3-65
3-17	Scenario 3: E-Zone Well-Field—12 Injection Wells, 6 Extraction Wells,	
	2 Dual-Purpose, 20 Total	. 3-67

FIGURES (Continued)

3-18	Scenario 1: Layout of Vertical Electrode Arrays—28 Deep Holes, 13 Shallow,	
	4 Horizontal VEAs	3-69
3-19	Scenario 2: Layout of Vertical Electrode Arrays—31 Deep Holes, 13 Shallow,	
	4 Horizontal VEAs	3-70
3-20	Scenario 3: Layout of Vertical Electrode Arrays—42 Deep Holes, 21 Shallow	
	Holes, 4 Horizontal VEAs	3-71
4-1	Surface Process Diagram	4-19
5-1	Oxygen Solubility in Water as a Function of Depth Below the Water Table and	
	Temperature	5-17
5-2	Simulated Cool-Down—Average Effluent Water Temperature	5-19

TABLES

2-1	Wood-Preserving Chemicals Used at McCormick and Baxter Site	. 2-22
2-2	Overview of Areas, Priorities, and Volumes of NAPL-Impacted Material	. 2-22
2-3	Estimation of Bulk Volume and Sand Fractions in Each Priority Treatment Zone	. 2-23
3-1	Summary of Well Counts by Layer and Scenario	. 3-72
3-2	Summary of Steam Injection Rate and Radius of Influence Calculations	. 3-73
3-3	Suggested Design Parameters for Reaching Monitoring Goals	. 3-74
3-4	Thermal Treatment Under Old Mormon Slough	. 3-74
3-5	Evaluation Matrix for Considerations Regarding Surface Cap	. 3-75
3-6	Supplemental Mobile NAPL Extraction Wells.	. 3-76
3-7	Steam Injection and Electrical Heating Rate Calculations	. 3-77
3-8	Summary of Steam, Power, and Water Demand	. 3-81
3-9	Electrical Heating Design Parameters and Power Demand	. 3-82
3-10	Total Liquid Extraction Rates and Design Capacity of Effluent Treatment System	. 3-83
4-1	Maximum Injection Pressures for Different Depth Zones	. 4-21
4-2	Effluent Treatment System—Components and Sizes	. 4-22
4-3	Average Cooling Needed During Cool-Down Period	. 4-23
4-4	Utility Estimates for Different Phases of Operation for Each Scenario	. 4-24
4-5	Estimation of Consumption of Grid Power During Operation	. 4-25
5-1	Treatment Time Estimates, Including Cool-Down Period	. 5-21
6-1	Overview of Sampling Frequency for Subsurface ERT and Temperature Monitoring	. 6-12
6-2	National Ambient Air Quality Standards	. 6-12
6-3	Performance and Compliance Monitoring and Confirmational Sampling	. 6-13
6-4	Compliance Criteria for Groundwater Re-injection	.6-17
6-5	Perimeter Monitoring Wells to Be Sampled	. 6-21
6-6	Perimeter Monitoring Frequency	. 6-21
7-1	Estimates of GAC Consumption by Liquid Stream Treatment	. 7-15
7-2	Definition of Zones, Depths, Volumes, and Energy Demand for Phasing Alternative	of
	Scenario 1	.7-16
7-3	Estimation of Bulk Volume and Sand Fractions in Priority Treatment Zones	.7-17
7-4	Summary of Design Parameters for the Four Steps	.7-18
7-5	Summary of Liquid Extraction Rate Calculations for Phasing Alternative of	
	Scenario 1	. 7-19
7-6	Sizing of Vapor Extraction System for Phasing Alternative of Scenario 1	. 7-20
7-7	Sizes and Capacities of Major Effluent Treatment System Components for Phasing	
	Alternative of Scenario 1	. 7-21
7-8	Utility Demand for Phasing Alternative of Scenario 1	. 7-22

ABBREVIATIONS AND ACRONYMS

ARAR	applicable or relevant and appropriate requirement
BTU	British thermal unit
°C	degree Celsius
Cal EPA/DTSC	California Environmental Protection Agency/Department of Toxic
	Substances Control
CAA	Clean Air Act
CDMG	California Division of Mines and Geology
CEMS	continuous emission monitoring system
CFR	Code of Federal Regulations
Cl ₂	chlorine gas
CO	carbon monoxide
CO_2	carbon dioxide
COC	chemical of concern
CPA	Cellon Process Area
DAF	dissolved air flotation
dBA	decibel A-weighted
DNAPL	dense nonaqueous-phase liquid
DO	dissolved oxygen
DTS	distributed temperature sensing
DWR	Department of Water Resources (state of California)
E-MPA	area east of Main Processing Area
EPA	U.S. Environmental Protection Agency
ERH	electrical resistance heating
ERT	electrical resistivity tomography
°F	degree Fahrenheit
FID	flame ionization detector
FS	feasibility study
ft^2	square foot
ft/day	feet per day
GAC	granular activated carbon
gpm	gallon per minute
HCl	hydrogen chloride or hydrochloric acid
HEPA	high-efficiency particulate air
HF	hydrogen fluoride
HHGIT	hybrid hydrologica-geophysical inverse technique
hp	horsepower
IAG	interagency agreement
J/kg/K	joule per kilogram per Kelvin

W:\74206\0110.035\TOC.doc

ABBREVIATIONS AND ACRONYMS (Continued)

kJ	kilojoule
km	kilometer
kPa	kilopascal
kW	kilowatt
lb	pound
lb/hr	pound per hour
LIF	laser-induced fluorescence
LNAPL	light nonaqueous-phase liquid
m	meter
MB	McCormick and Baxter
MCL	maximum contaminant level
$\mu g/m^3$	microgram per cubic meter
MG	million gallons
mg/kg	milligram per kilogram
mg/L	milligram per liter
mm	millimeter
MM	million
MPA	Main Processing Area
mph	miles per hour
msl	mean sea level
MW	megawatt
N-MPA	area under slough north of Main Processing Area
NAPL	nonaqueous-phase liquid
NO ₂	nitrogen dioxide
NO _x	nitrogen oxides
NPDES	National Pollutant Discharge Elimination System
NVD88	North American Vertical Datum of 1988
O&M	operation and maintenance
OD	outside diameter
OSHA	Occupation Safety and Health Act
OWPA	Oily Waste Pond Area
PAH	polycyclic aromatic hydrocarbon
PCDD/F	polychlorinated dioxins and furans
PCP	pentachlorophenol
PCS	power control system
PEL	permissible exposure limit
PM	particulate matter
POTW	publicly owned treatment work

W:\74206\0110.035\TOC.doc

ABBREVIATIONS AND ACRONYMS (Continued)

ppb	parts per billion
PPE	personal protective equipment
ppmv	parts per million vapor
psi	pound per square inch
psig	pound per square inch gauge
RAM	realtime aerosol monitor
RAO	remedial action objectives
RCRA	Resource Conservation and Recovery Act
RWQCB	Regional Water Quality Board (state of California)
S-CPA	area south of Cellon Process Area
S-MPA	area south of Main Processing Area
SAC	spent activated carbon
SCAPS	site characterization and analysis penetrometer system
scfm	standard cubic feet per minute
SO_2	sulfur dioxide
SO _x	sulfur oxides
STVZ	Socorro-Tech Vadose Zone
SVOC	semivolatile organic compound
TAP	toxic air pollutant
TCE	trichloroethene
TCLP	toxicity characteristics leaching procedure
TDR	time-domain reflectometer
TDS	total dissolved solids
THC	total hydrocarbon
TOC	total organic carbon
TPH	total petroleum hydrocarbon
TRS	Thermal Remediation Services, Inc.
UPRR	Union Pacific Railroad
USACE	U.S. Army Corps of Engineers
VEA	vertical electrode array
VOST	Volatile Organic Sampling Train
yd ³	cubic yard

1.0 INTRODUCTION

1.1 PURPOSE OF DOCUMENT

The purpose of this document is to provide a 10 percent conceptual design for in situ thermal remediation of the McCormick and Baxter Superfund site, a former wood treating facility in Stockton, California. This document addresses fundamental engineering design and cost issues related to the installation, operation, and post-operational stages of a combined steam and electrical heating remediation strategy.

This document will be used by the U.S. Environmental Protection Agency (EPA) and the U.S. Army Corps of Engineers (USACE) to prepare a feasibility study (FS) report that evaluates alternatives for a final groundwater remedy at the site. EPA is the lead agency for remediation of the McCormick and Baxter site, with the California EPA/Department of Toxic Substances Control (Cal EPA/DTSC) acting as the support agency.

1.2 PROJECT DESCRIPTION

The USACE has been tasked by EPA through an interagency agreement (IAG) to provide assistance with evaluation of dense nonaqueous-phase liquid (DNAPL) remediation technologies, including technical implementability and cost effectiveness as part of the FS. The USACE obtained services from URS Corporation through an indefinite delivery order contract to assist with this task. URS Corporation has contracted with SteamTech Environmental Services for expert engineering assistance on development of an in situ thermal treatment system conceptual design for the McCormick and Baxter site.

The engineering design aspects of the thermal remediation approach were produced by SteamTech Environmental Services, Inc., working in collaboration with Thermal Remediation Services, Inc. (TRS). Many design details were compared to the design for the Wyckoff-Eagle Harbor pilot test (USACE 2001b). Integration of a long-term pump-and-treat strategy with the thermal remediation was developed by the USACE. Additional technical advice was provided by URS Corporation. All cost estimates were prepared by URS Corporation, based on the engineering design and operational parameters supplied by SteamTech Environmental Services and TRS.

All site characterization data, including hydrogeology and contaminant distribution, were provided to SteamTech and TRS by the USACE.

Development of the conceptual design was further refined through the use of internal review meetings where expert consultants from research institutes provided advice regarding engineering, geophysical monitoring, and contaminant hydrology. In addition, representatives from state and federal regulatory agencies participated in these meetings to provide input on technical and regulatory compliance issues.

Production of the final conceptual design document was the responsibility of URS Corporation.

1.3 OVERALL PROJECT ASSUMPTIONS

Fundamental assumptions regarding operations and post-operational use of the McCormick and Baxter site are as follows:

- The ultimate land use at the site is expected to remain industrial.
- The selected remedy for arsenic and dioxin in surface soil after thermal remediation (a sitewide cap) will remain unchanged.
- Subsurface structures at the site, including concrete basements, pipe galleys, concrete and metal debris, and a rail car, will not be excavated during the remediation.
- Wells or other boreholes employed during thermal remediation operations at the site cannot be placed on adjacent private property. They may be placed in Old Mormon Slough, provided appropriate monitoring and emissions controls are in place.
- Old Mormon Slough cannot be permanently filled.
- Treated groundwater cannot be disposed of, at the anticipated full-scale operation volumes, to the Stockton Regional Wastewater Control Facility.

1.4 TECHNOLOGY CHOICE

The technologies chosen for this source removal are relatively innovative, but robust, thermal technologies that were shown to be successful in the 1990s. They include the following:

• Steam injection to heat the subsurface and drive contaminants toward extraction wells

- Electrical resistance heating to heat low-permeability zones internally by application of electric power to an array of electrodes
- Liquid and vapor extraction for removing contaminants and maintaining hydraulic and pneumatic control
- In situ destruction of chemicals of concern (COCs) by chemical and biological processes that are accelerated by the presence of dissolved oxygen at elevated temperatures and pressures (such as hydrous pyrolysis oxidation)
- In situ monitoring of the heating and steam flow using temperature measurements and electrical resistance tomography
- On-site cooling, separation, and treatment of effluent streams containing nonaqueous-phase liquid (NAPL), water, steam, and air

This combination of technologies can reduce NAPL source zones in a matter of years.

2.0 BACKGROUND AND OBJECTIVES

2.1 PERTINENT HISTORY AND SITE CHARACTERISTICS

The McCormick and Baxter Superfund site occupies approximately 32 acres in a predominantly industrial area near the Port of Stockton and the junction of Interstate 5 and State Highway 4 (Figure 2-1). Old Mormon Slough forms the boundary to the north and connects to the Stockton Deepwater Channel on the San Joaquin River. Site boundaries include Washington Street to the south, the Interstate 5 freeway to the east, and an industrial facility, which is located at the Port of Stockton Turning Basin, to the west. An 8-acre parcel in the southeastern portion of the site is owned by the Union Pacific Railroad (UPRR). The UPRR property boundaries shown in Figure 2-1 have been approximated from parcel maps.

The former processing areas and tank farm at the site are paved. The rest of the site surface is unpaved, with limited vegetative cover. A layer of gravel between 1 and 3 feet thick is found across most of the site. Railroad tracks are located on many areas of the site. Most of the former structures have been removed. The only remaining aboveground structures are the office building, two storage sheds, a stormwater collection system lift station, remnants of an old gas station (i.e., foundation and building, not a tank), a wooden tower, and a building near the tower. Underground sump-like basement foundations and associated piping for the former pressure treatment units remain in the central portion of the site. Entry to the site is controlled by a perimeter fence and 24-hour security service.

The site is located on the margin of the Sacramento River–San Joaquin River Delta in the Great Valley geomorphic province of California. The site terrain is relatively flat and near sea level, ranging from 8 to 15 feet above mean sea level (msl). Surface water bodies in the vicinity of the site include Old Mormon Slough, New Mormon Slough, the Stockton Deepwater Channel, and the San Joaquin River. Old Mormon Slough is approximately 2,500 feet long and 180 feet wide. Most of the slough is approximately 10 feet deep, although the western portion near its mouth has historically been dredged for barge access. Old and New Mormon Sloughs are tidally influenced, with a maximum tidal range of approximately 3 feet. Stockton Channel, the Port of Stockton Turning Basin, and Old Mormon Slough are areas of net sediment deposition, and all but the inner portion of Old Mormon Slough are periodically dredged to maintain depths appropriate for ship traffic.

The McCormick and Baxter Creosoting Company operated at 1214 West Washington Street in Stockton, California, from 1942 until 1991. Various wood preservation processes were used at the site during its operational history. The treated wood products were used primarily by power utilities, railroads, and in construction. The preservatives included creosote, pentachlorophenol (PCP), arsenic, copper, chromium, and zinc. Solvents or carriers for these preservatives included

petroleum-based fuels (such as kerosene and diesel), butane, and ether. A list of woodpreserving chemicals used at the site is shown in Table 2-1.

Most treatment processes consisted of pressure impregnation of the preservative solutions in retorts. Pressure-treated wood was removed from the retorts and allowed to dry in various wood storage areas throughout the site. The primary facility areas identified as the probable sources of contamination at the site include the Main Processing Area, Oily Waste Ponds Area, and Cellon Process Area. Figure 2-2 shows the potential source areas defined at the site.

The surface geology of the McCormick and Baxter site has been mapped as undifferentiated recent alluvium and Victor Formation (DWR 1967). These deposits are underlain by the Plio-Pleistocene Laguna Formation, which is underlain by the Mehrten Formation of Miocene to Pliocene age. The flood basin deposits contain delta equivalents of the Victor and Laguna Formations (i.e., the Victor and Laguna Formations interfinger with flood basin deposits).

More recently, surface deposits have been mapped as Modesto Formation (CDMG 1990) and as fan deposits of the Calaveras River (Atwater 1982). The Modesto Formation represents Pleistocene glacial outwash fans that are approximately 10 to 15 feet thick. For the purposes of this design, the Modesto Formation and the Calaveras Fan deposits are considered to be equivalent to the Victor Formation.

Subsurface materials at the McCormick and Baxter site at elevations higher than approximately –230 feet North American Vertical Datum of 1988 (NVD88) consist primarily of two types. The most abundant volumetrically, forming about 60 percent of the subsurface, is a uniform stiff to very stiff gray-green micaceous clayey silt with common calcite nodules and/or veins. The remainder consists of a gray to gray-green fine- to medium-grained sand and silty sand. Sedimentary structures such as laminations and cross-bedding are not observed in the soil samples of site sands and silts. Contacts between the two materials are typically abrupt but may be gradational.

Sand zones exist both as laterally continuous horizontal layers and as discontinuous layers and pods within and between the clayey silt material. These sand zones range in thickness from a few feet up to 30 feet thick. Most laterally continuous sand units are approximately 10 feet thick.

Some general trends in the occurrence of silts and sands in the deeper subsurface are apparent. The sand units are vertically well connected to -100 feet elevation beneath the former Cellon Process Area. Thus, many laterally extensive sand units at various depths are vertically connected beneath the Cellon Process Area. The proportion of silt to sand appears to be greater below an elevation of approximately -200 feet and above the gravelly sand/sandy gravel unit at a -240-foot elevation.

The observed vertical and lateral relations of the sand and silt materials at the site are consistent with features attributed to bedload channel (sand) deposits and flood basin overbank (silt and clay) deposits of a fluvial system, suggesting that the subsurface materials are flood basin deposits as described by Atwater (1982) and in various USACE reports (USACE 1999, 2000a). Since sand channel deposits represent a large percentage of the subsurface materials (approximately 40 percent), channels tend to overlap each other, allowing a locally high degree of horizontal and vertical connectivity of the sand deposits.

A sandy gravel/gravelly sand unit is encountered at an elevation of approximately –240 feet. This unit is laterally continuous and varies in thickness from 5 to more than 20 feet thick. The gravel and sand consists primarily of quartz/quartzite, volcanic, and metamorphic lithic fragments.

The bottom of the sandy gravel/gravelly sand unit has been penetrated at two locations. Sand and silts encountered below the gravelly unit are similar in grain size, color, and consistency to those encountered at higher elevations.

Old Mormon Slough sediments in the slough adjacent to the McCormick and Baxter site contain stratified clay, silt, and sand. The uppermost sediments consist of up to 6 feet of soft, dark-colored clay and silty clay. These have been interpreted as being deposited in a quiescent, stagnant water environment (comparable to the current conditions) after the channel was cut off from its upstream source in 1970 during the construction of Interstate 5. The uppermost sediments are underlain by a thin, soft, very dark or dark olive-gray silt with plant remains and occasional discontinuous, thin sand layers. The contact between the recent unconsolidated slough deposits and the older pre-Old Mormon Slough recent flood basin deposits occurs between 3 and 5 feet below the mudline (-11 to -13 feet NVD88). Silt identical in color and consistency to that observed at adjacent locations was present at 5 feet below the mudline. The change in consistency from soft to firm within the silts noted in the previous sediment core descriptions has been interpreted to be the contact between the older pre-Old Mormon Slough recent flood basin deposits (USACE 2000a).

The upper 200 feet of sediments are collectively referred to as the shallow aquifer. Groundwater in this zone occurs primarily in laterally continuous sand layers and lenses of fine- to coarse-grained greenish-gray sand. The sediments from approximately –190 feet NVD88 downward have been termed the deep aquifer.

The horizontal and vertically overlapping distribution of relatively permeable versus impermeable materials in the subsurface throughout most of the depth range examined prohibits the presence of well-defined aquifers and aquitards. Previous site investigations have divided the subsurface into five distinct sand zones designated A, B, C, D, and E (Figures 2-3 and 2-4).

Sand units at a given elevation may have some lateral continuity across the site, but silt deposits can separate sands at equal elevation (e.g., the A1-zone aquifer is discontinuous between the UPRR property and the western portion of the site.) The sandy zones beneath the former Cellon Process Area (SE-08) are well connected vertically, effectively connecting the A- and B-zones in this area. Therefore, the aquifer zone designation applies only locally to areas of the site where the silts are laterally continuous and not vertically bisected by sand channels.

Hydraulic conductivity measurements have been derived from laboratory tests of recovered samples (vertical conductivity) and from pumping tests (horizontal conductivity). Horizontal values are summarized in Table 3-2. Vertical conductivity for geologic materials is often one or more orders of magnitude lower than horizontal conductivity. Thus, the laboratory permeability data are likely to be biased in favor of low values. Detailed information on test methodology and results is contained in USACE reports and references therein (USACE 1999, 2000a).

The horizontal component of groundwater flow at the site is toward the southeast in the A-zone, turning gradually towards the east-northeast in successively deeper aquifer zones. A flow in the E-zone toward the east-southeast is consistent with historical regional groundwater flow data (DWR 1967; USEPA 1998) that show a large groundwater cone of depression in response to groundwater pumping centered over the central portion of the city of Stockton. The calculated average horizontal velocities of groundwater in the A-, B-, C-, D-, and E-zone sand units are 0.3, 0.1, 0.1, 0.05, and 0.5 foot/day respectively. The observed vertical gradient of flow has been downward between all aquifer zones. The calculated vertical groundwater flow velocities between aquifer zones A and B, B and C, C and D, and D and E are 0.0008, 0.0012, 0.0014, and 0.0011 foot/day respectively. Calculated vertical groundwater flow velocities are two to three orders of magnitude less than horizontal groundwater velocities. Therefore, the predominant direction of groundwater flow and dissolved-phase contaminant transport is horizontal within sand zones. Vertical velocities are likely to be higher in areas where sand units have a strong vertical connection.

The southeasterly flow of groundwater within the A- through D-zones suggests that groundwater recharge of the upper aquifer is from the northwest and/or local pumping of the upper aquifer is to the southeast. The Stockton Deepwater Channel is north of the site, and the main channel of the San Joaquin River is west of the site. These are likely groundwater recharge sources for the upper aquifer. Old Mormon Slough is considered to have only a poor hydraulic connection to the upper aquifer (USACE 2000a).

2.2 REMEDIAL ACTION OBJECTIVES

EPA will set separate remedial action objectives (RAOs) for the remediation of NAPL and the remediation of dissolved-phase groundwater contamination at the McCormick and Baxter site.

The RAOs for remediating NAPL will address mass removal of NAPL source areas that represent principal threat wastes; the RAOs for dissolved-phase contaminants in groundwater will address the subsequent remediation of remaining dissolved-phase contamination to achieve groundwater cleanup standards that will be established by EPA and the state.

The relevant RAOs for this conceptual design document are the NAPL RAOs, which are to (1) remove NAPL to the maximum extent technically feasible to protect the E-zone drinking water aquifer, and (2) to reduce the need for long-term pump-and-treat.

Due to the complexity of the site and the extent of NAPL contamination, thermal treatment is not being considered as a stand-alone remedy. Thermal treatment is being evaluated for source removal and may be used in conjunction with dissolved-phase pump-and-treat and/or monitored natural attenuation as part of the final groundwater remedy.

RAOs and the overall groundwater cleanup approach will be discussed in detail in the FS report to be prepared separately by EPA and the USACE.

2.3 CRITERIA FOR SELECTION OF THERMAL TREATMENT AREAS

The following criteria were provided by EPA and the USACE, with input from Cal EPA/DTSC, the California Regional Water Quality Control Board (RWQCB), and the technical advisors, as the basis for prioritizing areas of the site for thermal treatment:

- Relative amount of NAPL present
- Proximity of NAPL to drinking water risk receptors (i.e., the E-zone aquifer)
- Certainty of the data indicating the presence of mobile NAPL
- Potential for future changes in land use and/or subsurface conditions that might facilitate mobilization of currently immobile NAPL (e.g., increased pumping from E-zone downgradient of the site)
- Ease of access for followup remediation activities

2.4 DEFINITION OF TREATMENT VOLUMES AND SCENARIOS

This section provides a detailed description of the thermal treatment scenarios and the associated volumes of NAPL-impacted soil. The site was split in the areas shown in Table 2-2.

2.4.1 Treatment Scenarios

Scenario 1—Main Process Area (MPA), Cellon Process Area (CPA), and North-MPA

The MPA and CPA are the major historical sources of the NAPL that have contaminated soil and groundwater within the boundaries of the McCormick and Baxter site. Subsurface migration from the MPA and CPA has also impacted the deep subsurface zones under the slough (North-MPA). SteamTech has estimated that up to 1.54 million yd³ of NAPL-impacted soil is present in the area delineated for this scenario, representing 52 percent of the total volume of NAPL-impacted soil believed to be present at the McCormick and Baxter site.

Scenario 2—MPA, CPA, North-MPA, South-MPA, and South-CPA

This scenario includes the areas designated for Scenario 1 (MPA, CPA, North-MPA) and extends to the south (South-MPA and South-CPA). SteamTech has estimated that up to 2.10 million yd³ of NAPL-impacted soil is present in the area delineated for this scenario, representing 71 percent of the total volume of NAPL-impacted soil believed to be present at the McCormick and Baxter site. Scenario 2 consists of the same treatment processes as those indicated for Scenario 1. However, Scenario 2 would require increased steam injection, NAPL/groundwater extraction and treatment, power demand for electrical resistance (ERH), and larger-size equipment due to the larger volume of soil and groundwater to be treated.

Scenario 3 — MPA, CPA, North-MPA, South-MPA, South-CPA, East–MPA, and Oily Waste Pond Area (OWPA)

This scenario includes the areas designated for Scenario 2 and extends east to the East-MPA and west to the OWPA. The scenario includes known and suspected areas of NAPL-impact documented for the McCormick and Baxter site. SteamTech has estimated that up to 2.95 million yd³ of NAPL-impacted soil is present in the area delineated for this scenario, representing 100 percent of the total volume of NAPL-impacted soil believed to be present at the McCormick and Baxter site. Scenario 3 consists of the same treatment processes as those indicated for Scenarios 1 and 2. However, Scenario 3 would require increased steam injection, NAPL/groundwater extraction and treatment, power demand for ERH, and larger-size equipment due to the larger volume of soil and groundwater to be treated.

2.4.2 Treatment Volumes

The main treatment areas are shown in Figure 2-5. Details of the volume calculations are provided in Table 2-3, which also shows how the Main Processing Area (MPA), the area south of the MPA (S-MPA), the area under the slough north of the MPA (N-MPA), and the area south of the Cellon Process Area (S-CPA) were divided into smaller areas with different vertical treatment intervals (this is due to the complex three-dimensional distribution of the NAPL in the

subsurface and an effort to minimize the overall treatment volume). These subdivisions are shown in Figure 2-5, and their priorities for treatment are shown in Figure 2-6.

The approximate depths of NAPL-impacted material are indicated in Figure 2-7.

Volume estimates were made using simple three-dimensional geometric shapes based on a manual interpretation of all available data.

For the area east of the MPA (E-MPA), our volume calculation assumed that treatment will be from an elevation of 120 feet and down, since NAPL has not been confirmed during the site characterization and analysis penetrometer system (SCAPS) investigation to a total depth of approximately 100 to 130 feet. We have assumed treatment from 120 to 260 feet, since we understand that the lack of NAPL detection in this deeper zone may be partly due to lack of data, especially for the deeper D- and E-zones.

The N-MPA zone, which is under the slough north of the MPA, was extended to the east after we reviewed the NAPL data. The NAPL present at depth could influence the treatment efficiency in the MPA. This zone has a volume of 266,000 yd³.

The total sand volume estimated within the total NAPL-impacted areas listed above is about 720,000 yd^3 , with about 39 percent of this (253,000 yd^3) located in the MPA. This affects the estimated cost to treat each cubic yard of NAPL-impacted soil, since only between one-fifth and one-third of the sediment volume is sand, which will be readily flushed by steam.

The three scenario areas are indicated in Figure 2-8. Areal extent of the thermal treatment volume is shown for each scenario in Figures 2-9, 2-10, and 2-11. Table 2-3 indicates the associated total volumes and sand fractions for all scenarios.

Partial thermal treatment under Old Mormon Slough is included in each of the three scenarios. One treatment design for each of Scenarios 1, 2 and 3 is provided. Scenario 1 involves treatment of the C-, D-, and E-zones under the slough using angled wells, and electrical heating of the D-E aquitard. This treatment is restricted to the two easternmost subareas of the N-MPA (2 and 3). Scenarios 2 and 3 involve treatment of the N-MPA from the B-zone down. This involves wells or electrodes installed through the slough (shallowest boreholes) and by angled drilling as described for Scenario 1.

At this point there are no plans of designing thermal remediation for the shallow sediments under the slough other than those in the N-MPA deeper than -30 feet elevation. However, extraction and operation close to the slough is part of the design because adverse effects on the slough will be minimized (see Section 3.7).

All scenarios involve treatment of the A-zone aquifer in MPA. Since NAPL-containing sediments from the OWPA have been placed under the current asphalt cap, it is desirable to attempt remediation beneath the cap (see Section 3.11).

The ultimate operational goal of the thermal treatment system will be to operate until such time as free product is no longer being recovered from extraction wells within the treatment area. In parallel with this goal, the intent is to operate the system until the whole target volume has been raised to a specific target temperature for a defined period of time.

2.5 COMBINED THERMAL TREATMENT AND PUMP-AND-TREAT

A pump-and-treat system will be an integral part of the full design for all three scenarios discussed in this document. The pump-and-treat system will be operated during thermal treatment and during cool-down.

The main focus of the pump-and-treat is to extract mobile NAPL from contaminated areas and zones that are not being treated by thermal remediation and to incidentally remove dissolved-phase contaminated groundwater. The proposed extraction wells will be located outside the steam wells and electrical heating electrodes at several depth zones. The wells will be located preferentially in areas where mobile NAPL has been observed or is suspected to be present. It is anticipated that both NAPL and dissolved-phase contaminants may be recovered by the pump-and-treat system. Limited spreading of steam and heat away from the target thermal treatment zones will allow for some thermal enhancement in the pump-and-treat wells. This is a desirable effect leading to increased removal of NAPL from the volumes not directly heated; it also serves the purpose of containing small amounts of contaminants that may migrate outward from the thermal remediation target zones.

After thermal treatment, continued pumping will be used to control the flow of water during the cool-down period. A subset of the full well-field will be used for the pump-and-treat system, determined during operation as the wells that contain most contaminants, are located in optimal positions for hydraulic control, and function appropriately as extraction wells.

Well locations for both thermal treatment and pump-and-treat are indicated in Section 3.1. Hydraulic control and removal of mobile NAPL is discussed in more detail in Section 3.9.3. The full treatment period for each scenario is defined in Section 5.7.













Figure 2-5 Map of Main Treatment Zones, Showing Subdivisions and Abbreviations THERMAL TREATMENT TECHNOLOGY CONCEPTUAL DESIGN McCormick and Baxter Superfund Site Stockton, California





ĬĬŦĬĬ

Figure 2-7 Approximate Depth Intervals of NAPL-Impacted Soil and Groundwater THERMAL TREATMENT TECHNOLOG CONCEPTUAL DESIGN McCormick and Baxter Superfund Site Stockton, California









Table 2-1 Wood-Preserving Chemicals Used at McCormick and Baxter Site

Common Name	Chemical Components	Period of Use
Creosote	Creosote and fuel oil	1942-1990
Pentachlorophenol	Pentachlorophenol and oil	1946-1990
Bouliden salts	Chromium, copper and arsenic	1949-1952
CCA	Chromated copper and arsenic	1952-1970
Cellon	Pentachlorophenol, butane, and ether	1965-1988
ACA	Ammoniacal copper arsenate	1970-1986
Flamescape	Diammonium phosphate, ammonium sulfate, and boric acid	1976-1988
ACZA	Ammoniacal copper-zinc arsenate	1986-1990

Table 2-2 Overview of Areas, Priorities, and Volumes of NAPL-Impacted Material

Abbreviation	Explanation	Priority	Volume of NAPL Impact (yd ³)
MPA	Main Processing Area	1	1,152,000
CPA	Cellon Process Area	1	290,000
N-MPA	Area under slough north of MPA	1 and 2^a	266,000
S-MPA	Area south of MPA	2	200,000
S-CPA	Area south of CPA	2	196,000
E-MPA	Area east of MPA	3	597,000
OWPA	Oily Waste Pond Area	3	252,000
Total			2,952,000

^a Deepest parts of the N-MPA are priority 1; the western part of N-MPA is priority 2. Details are shown in Figures 2-9 and 2-10.

												Zone	Zone	
Zone/area	Sub	Priority	Area (ft ²)	Top (ft)	Btm (ft)	Depth (ft)	Volume (vd ³)**	Total (yd ³)	% of total	Sands*	Silts	Sands	Silts	% sand
MPA	1	1	17,695	0	-120	120	78,651			17,303	61,348			
	2	1	28,419	0	-180	180	189,475			41,685	147,791			
	3	1	74,247	0	-260	260	715,028			157,306	557,722			
	4	1	19,398	-25	-260	235	168,848	1,152,002	39.0	37,147	131,701	253,440	898,562	22
CPA		1	97,739	0	-80	80	289,620	289,620	9.8	92,678	196,942	92,678	196,942	32
N-MPA	1	2	11,097	-30	-120	90	36,993			8,138	28,855			
	2	1	9,538	-30	-180	150	52,993			11,658	41,335			
	3	1	20,628	-30	-260	230	175,734	265,720	9.0	36,904	138,830	56,701	209,019	21
S-MPA	1	2	30,977	-50	-120	70	80,317			17,670	62,647			
	2	2	49,504	-75	-140	65	119,186	199,503	6.8	26,221	92,965	43,891	155,612	22
S-CPA	1	2	18,380	-40	-80	40	27,232			5,719	21,513			
	2	2	17,965	-60	-100	40	26,617			2,662	23,955			
	3	2	30,653	-60	-100	40	45,415			4,542	40,874			
	4	2	21,853	0	-120	120	97,132	196,396	6.7	21,369	75,763	34,291	162,105	17
E-MPA		3	115,068	-120	-260	140	596,697	596,697	20.2	161,108	435,589	161,108	435,589	27
OWPA		3	85,187	0	-80	80	252,426	252,426	8.5	80,776	171,650	80,776	171,650	32
Sums								2,952,365	100	722,886	2,229,479	722,886	2,229,479	24
Scenario		Zones i	ncluded					Total (yd ³)	% of total	Sand (yd ³)	Silts (yd ³)	% sand		
1		MPA, CF	PA, N-MPA	DE zone	s only***			1,541,854	52.2	370,400	1,185,586	24.0		
2	all	MPA, CF	PA, N-MPA,	S-CPA,	S-MPA (a	all depths)		2,103,242	71.2	481,002	1,622,240	22.9		
3	all	All areas	and depths	6				2,952,365	100.0	722,886	2,229,479	24.5		
*used minin	num s	and fracti	on of 0.10 f	or any zo	one that n	eeds steam	treatment							
**volume es	stimat	es are rou	igh based o	n simple	geometri	c shapes ar	nd assuming each	subarea/vol	ume simplif	ied as a box				
***note that	scena	ario 1 invo	olves 50% o	f the soil	column f	rom 30 to 2	60 ft only, due to	D and E zon	e treatment					

Table 2-3 Estimation of Bulk Volume and Sand Fractions in Each Priority Treatment Zone
3.0 SUBSURFACE REMEDIAL DESIGN

3.1 WELL-FIELD AND VOLUMES

The boundary of the area impacted by NAPL migration has been defined on the basis of direct and indirect soil sampling. NAPL was visually observed in a number of soil borings as a brown to black liquid with a strong odor of naphthalene. Qualitative NAPL saturations observed in soil cores ranged from oozing and/or dripping product to brown stains and/or sheen. The highest saturation was most commonly observed in sandy lithology but was also observed in silt in a limited number of cases. The presence of polycyclic aromatic hydrocarbon (PAH) NAPL was also interpreted on the basis of SCAPS laser-induced fluorescence (LIF) data, indicated by a LIF count of greater than 300 (USACE 2000b). SCAPS data also indicate the presence of NAPL in both sand and silt, being found more commonly in sandy lithology. Sandy layers contaminated by NAPL tend to be effectively uniformly saturated in cores, whereas contaminated clayey silt layers more typically contain discontinuous blobs or thin, vertically oriented stringers of NAPL.

The shaded areas in Figures 2-3 and 2-4 represent subsurface volumes interpreted as containing NAPL. NAPL within these regions may occur as mobile and/or residual NAPL; as pools of DNAPL, ponded on low-permeability layers; or as thin, vertically discontinuous fingers or stringers. DNAPL may not be present everywhere within such regions. The labeled treatment zones and subzones (MPA, OWPA etc.) define the lateral extent of subsurface areas of interpreted NAPL presence based on direct soil sampling and SCAPS LIF pushes. These generally lie within the "maximum extent of interpreted NAPL" line on earlier field investigation reports (e.g., Figures 5-5 to 5-8 in USACE 2000). In certain areas, notably at the east end of the site, the defined zones extend beyond the known or interpreted NAPL presence. These boundaries represent convenient demarcation lines for well installation in areas where high dissolved-phase concentrations of COCs suggest that presently unsampled NAPL may be present nearby.

The overall thermal remediation strategy proposed for this site entails heating the volumes of soil known or suspected to contain NAPL, as defined in Section 2.4. These soil volumes will be heated by steam injection in aquifer zones A, B, C, D, and E, where permeability is sufficient for effective steam migration. In those "aquitards" where NAPL is known or suspected to be present and permeability is too low for effective steam injection, ERH will be used to heat the soil to the target temperature (this applies to zones below the C-zone aquifer). Steam injection will be focused on individual aquifer zones by screening injection wells at appropriate depths.

Extraction wells will be screened so as to straddle both the aquifer zones and parts of the adjacent aquitards. Liquid and vapor extraction will thus be achieved throughout the entire

NAPL-impacted interval in any given treatment area. The vertical distribution of injection and extraction intervals is summarized in Figures 3-1 and 3-2.

A seven-spot configuration has been used for the basic well-field cell, in which a central extraction well is surrounded by six injection wells. The interwell separation varies between aquifer zones and is dependent on the assumed bulk permeability of that zone (USEPA 1998; USACE 2000a). In the A-, B-, and D-zones, a well separation of 60 feet has been used. The higher permeability of the C- and E-zones has permitted 120- and 180-foot well separations, respectively. Multiples of 60 feet have been used as well separations for convenience of design (Section 3.3 and Appendix B). It is assumed that wells would not be collocated in reality (although they are presented as such in the accompanying plans), but clustered in groups within 10 to 20 feet of each other. Because of the large size and complex stratigraphy and range of permeability at this site, relatively conservative permeability values have been used in calculating the radius of influence of each injection well. Consequently, the well spacing presented here should be regarded as a minimum. The relatively high-density well-fields generated using these spacings are, however, the most robust configurations for the prevailing conditions at the site.

For the E-zone wells, the contaminated area was surrounded by injection wells, eliminating the need for outside extraction. However, if contaminants are detected during installation of the injection wells, the well-field will have to be adapted according to the principles outlined in Section 7.4.

3.1.1 Scenario 1

The well-field layout for Scenario 1 is presented, by aquifer zone, in Figures 3-3 to 3-7. In the A-zone, the basic 60-foot separation cell is repeated to cover the MPA and the CPA. A series of A-B-zone extraction wells forms a perimeter on the north, south and west sides where NAPL is known or suspected to be present close to the margins of the treatment area. An additional two extraction wells, screened only in the A-zone and extended to a shallower elevation of -5 feet are present in the area of the repository, at locations coincident with B-zone injection wells. This approach is intended to address contamination of imported soil in the repository, which must be surrounded by injection wells to ensure an effective steam drive toward the central extraction wells. The immediate concern of NAPL in the A- and B-zones beneath Old Mormon Slough is addressed by the presence of two rows of extraction wells, screened across the A- and B-zones in the N-MPA.

In the C- and D-zones (Figures 3-5 and 3-6), the relevant unit cell has been repeated to provide coverage across the MPA and the CPA. As in the overlying aquifer zones, a perimeter of extraction wells is proposed where potential for lateral mobilization of NAPL exists. Angled

injection and extraction wells extending beneath Old Mormon Slough in these zones are shown in plan view.

In the E-zone (Figure 3-7), a variant of the basic unit cell has been adopted. An outer ring of injection wells surrounds an inner ring of extraction wells, which in turn, surrounds a central dual-purpose well. The central dual-purpose well is intended to be used initially as an injection well and will be completed as such. However, should circumstances warrant, it may be readily converted for use as an extraction well. The outermost ring of injection wells can be justified on the basis of the absence of confirmed NAPL in the E-zone. The well-field for this zone may require modification if NAPL were to be identified during installation (as discussed in Section 7.4).

3.1.2 Scenario 2

The well-field layout for Scenario 2 is presented, by aquifer zone, in Figures 3-8 to 3-12. The well-field for the A-zone is as for Scenario 1; however that for the B-zone extends coverage into the S-MPA and the S-CPA. A perimeter of extraction wells extends along the southern boundary of the treatment area in recognition of the potential for off-site mobilization of NAPL in these areas. B-zone injection wells extend through the central part of the repository immediately adjacent to the additional A-zone extractors. In contrast to Scenario 1, EHR electrodes are collocated with A-B-zone extraction wells in the N-MPA beneath Old Mormon Slough (Figure 3-9).

Coverage of the C-zone well-field is extended further into the S-MPA and the S-CPA than that of Scenario 1 (Figure 3-10). The eastern boundary of the C-zone is marked by a line of extraction wells in order to address the risk of lateral mobilization of known or suspected NAPL into the E-MPA. A line of extraction wells forms the eastern boundary of the D-zone in response to the same risk in that aquifer zone (Figure 3-11).

The E-zone well-field for Scenario 2 is no different from that of Scenario 1 (Figure 3-12).

3.1.3 Scenario 3

The well-field layout for Scenario 3 is presented, by aquifer zone, in Figures 3-13 to 3-17. The A- and B-zone well-fields in Scenario 3 differ from that of Scenario 2 only in that coverage has been extended into the OWPA (Figure 3-13 and 3-14). The northern, southern, and western boundaries of the OWPA well-field are formed by A-B-zone extraction wells in view of the potential for off-site mobilization of NAPL in this area.

The C-zone well-field in Scenario 3 is extended into the E-MPA (Figure 3-15). The presence of NAPL has not been confirmed in the C-zone of the E-MPA; however, high dissolved-phase

naphthalene concentrations have been recorded in well DSW-1C (5,400 ppb). Consequently, it is proposed that a C-zone extraction well be located close to DSW-1C, surrounded by injection wells. The fence of C-zone extraction wells along the southern shore of Old Mormon Slough has been extended eastward to address the possibility of northward mobilization of NAPL in the C-zone by those injection wells around DSW-1C.

NAPL is not known or suspected to be present in the D-zone in the E-MPA; however, historically high dissolved-phase naphthalene concentrations in well MW-4D in the central E-MPA, coupled with recently high concentrations in the C- and E-zones at that location, suggest that NAPL may be present in the D-zone in this area. The D-zone well-field has been extended to cover this area to address this risk (Figure 3-16).

The E-zone well-field of Scenarios 1 and 2 has been extended into the E-MPA for Scenario 3, in order to address the potential presence of NAPL in this area, as suggested by the presence of high dissolved-phase naphthalene concentrations in well MW-4E (Figure 3-17).

The number of wells is indicated in Table 3-1.

3.2 COMBINED STEAM AND ELECTRICAL RESISTANCE HEATING BOREHOLES

3.2.1 Materials

The ERH electrodes consist of three elements: a down-hole electrical supply cable, a metal electrode, and a conductive backfill.

The down-hole electrode cables consist of highly flexible copper stranding that is insulated with a TeflonTM outer covering. This cable type is similar to type K and is selected based on the high temperature (165°C) and chemical aggressiveness of the subsurface. The cables will be 2-O gauge, doubled-up to provide the required amperage.

The initial current of the D-E-zone electrodes will be about 305 amperes. The N-MPA electrodes (beneath the slough) will have two discrete elements to independently control the power to each depth. Each of these electrode elements will draw about 395 amperes.

The electrode cables will be bolted to a metal plate that provides a larger surface area for electrical conduction to the electrode backfill. A number of metal types could serve the purpose in this application. Due to the great depth and resulting cost of the borehole installation, two electrode cables and metal electrodes will be installed in each borehole to provide redundancy against failure during the remediation period.

The electrode backfill is the most important part of the electrode because it is the contact with the native soil that has the highest potential resistance of the electrode. As an analogy, the sand pack is the critical element of a water well. The electrode backfill consists of a granular steel shot with a mixture of particle sizes ranging from 0.5 to 2 mm in diameter. The steel shot is remarkably dense at 270 pounds per cubic foot; this density provides high contact pressures against the borehole sides, and its particle nature provides a conformable contact to match the native soil contours.

3.2.2 Electrical Isolation

The potential for electrical shock is a concern whenever ERH is used for site remediation. In many cases, extensive voltage damping measures must be used to prevent high induced voltages at the surface, where personnel may be exposed. However, the McCormick and Baxter site does not require any special voltage damping measures due to the depth of application for ERH. This depth provides the physical separation required to reduce surface voltages to far below OSHA safe limits as described in the following paragraphs.

The D-E aquitard electrodes require no special electrical isolation measures due to their great depth. They will be separated from the D-zone injectors and extraction wells by a 10-foot interval of tremmied grout; however, the main purpose of this grout is to prevent injected steam intrusion into the electrode. Steam intrusion is not desired because the steam can push the native water away and lead to a dry out condition, where the native soils lose their electrical conductivity.

Up to 300 volts of electricity will be applied to the D-E aquitard at a 175- to 210-foot elevation. The bottom of the D-zone injection wells at a 170-foot elevation will be subjected to a voltage of about 70 volts, albeit within a sand backfill that has very high native electrical resistance. The high electrical resistance of the sand backfill will prevent it from transmitting a large electrical current to the metal well casing.

The voltage of a good conductor (such as a metal steam injection or extraction well casing) is equal to the average contact voltage over its entire length, with some corrections for the electrical resistance of the backfill materials. The average induced electrical field over the entire length of an isolated D-zone injection well will be about 12 volts. Due to the fact that the high-voltage bottom of the well will be located within a nonconductive sand pack, the actual induced voltage of an isolated well will be somewhat less, probably about 8 volts.

The above voltages refer to an electrically isolated well. In practice, the wells will be connected via conductive steel piping. The interconnection of all of the wells causes their induced voltage to regress toward the mean voltage of the entire site, which is neutral (0 volts). In practice,

inhomogeneities of the site prevent piping networks from stabilizing at exactly 0 volts; an induced voltage of 1 to 2 volts is most common.

Without special measures, the voltage of the injection wells and extraction wells will stabilize at some very low and safe value, well below the Occupational Safety and Health Act (OSHA) limit of 30 volts. However, some additional "belt and suspenders" measures are appropriate:

- The bottom of the D-zone injectors and extractors will rest on the grout interval that separates them from the electrodes. In order to increase the electrical resistance at this contact point, a second oversized TeflonTM-lined end cap will be epoxied over the bottom cap of the wells. This over-cap will increase the electrical resistance of the wells in this important region.
- Although the wells will be interconnected by piping, this piping might fail or be disconnected for maintenance. To maintain continuous electrical contact between the wells, a 1-O bare copper wire will be interconnected to each well at ground level as a redundant grounding connection. This grounding network will also incorporate any monitoring wells that are located in the heated zone.

Of note, any components that are inside the wells, such as pumps, will be at the same low voltage as the well network. No special measures will be required to protect pumps or other internal well components.

The electrical resistivity tomography (ERT) electrodes will pick up any induced AC voltage in their vicinity during ERH operation. Grounding of the ERT electrodes is NOT a preferred option because we do not want to encourage any non-uniform current flow (and resulting electrical heating) in their vicinity. The ERT electrodes require electrical insulation at the surface for proper operation. This electrical isolation can be easily accomplished by configuring the surface connections of the electrodes with the female end toward the subsurface to provide protection against the ERH voltages that are induced in them. The ERT-induced voltages will probably be in the range of 3 to 100 volts, depending on the individual ERT electrode location. For the purpose of accurate monitoring, it is not good practice to place an ERT string near an electrode or well. Locating the ERT strings at a distance from the ERH electrodes also lowers their induced voltage.

3.3 STEAM INJECTION AND MIGRATION

3.3.1 Steam Injection Intervals

The locations and layouts of the steam injection well screens are presented in Section 3.1 for the three scenarios. The intervals were chosen based on the following criteria:

- For thermal treatment of the chosen areas, steam will be injected in all of the depth intervals representing the treatment depths, except the upper A-zone in some locations where the A and B sands are sufficiently connected to allow for steam migration upward from the B-zone into the A-zone.
- Where a zone has a good definition of the outer bounds of the NAPL area, steam injection wells were placed outside the target zones, allowing for an outside-in steam migration.
- For zones where NAPL is confirmed or suspected to be present outside the thermal treatment zone, the steam injection intervals are typically surrounded with extraction wells equipped with both liquid and vapor extraction capability.
- For thick sand zones that need steam flow in the bottom section, the injection screens are designed so they preferentially inject steam in the lower half of the aquifer interval. This is done by using short injection screens, typically screened from the middle of the aquifer to several feet into the underlying silt layer/aquitard.
- Where a sand zone is overlain by a thick silt/aquitard (such as the D-E aquitard in large areas of the site), it is desirable to allow steam injection in the bottom half of the aquitard into sand lenses that may be present. Often, it is not known where such lenses are, but the likelihood of sand sections is recognized. In such cases, the deeper zone steam injection wells are completed with the top of the screened interval located in the overlying aquitard. As an example, E-zone steam wells may be screened from about –200 feet of elevation (the middle of the D-E aquitard) to the bottom of the E aquifer. This will allow steam heat-up of any sand lenses that allow steam penetration in the lower D-E aquitard, without preventing sufficient heating in the E-zone, since the permeability in the E-zone aquifer is orders of magnitude higher than that of the D-E aquitard.
- For thinner aquifers where the achievable steam injection rate may be limiting heat-up and performance, the injectors are screened across the entire aquifer, but not in the overlying aquitard, allowing a higher injection pressure (due to a greater

depth to the top of the screen), and the maximum achievable transmissivity of the aquifer interval.

3.3.2 Steam Injection Rates

The steam injection rates are critical for estimating the size of the steam generation equipment, for setting the well spacing, and for predicting the duration of thermal treatment. The estimation of achievable steam injection rates for the wells at different depths is provided in Appendix B. The procedure used to estimate the rates is as follows:

- The A through E zonation for the dominating aquifers was adapted, with an assumption of average depth for each zone as provided by Mike Bailey, USACE, in a summary table (Bailey 2001).
- The sand fraction in each zone was estimated based on Bailey (2001) and used to make a rough assumption of the steam zone thickness during the initial steam migration and heat-up. Typically, it was assumed that the steam zone filled 50 percent of the average aquifer thickness, with a condensate zone surrounding the steam zone.
- Four-inch injection well screens in 10-inch boreholes were assumed, and the maximum injection pressures were defined as 0.5 psi per foot measured from surface to the top of the injection screen.
- Permeability averages from Bailey (2000) were used for each aquifer zone. Three calculations were made for each depth interval, one where the average conductivity was used, one where one-third of the average was used, and one where three times the average was used. This allows for a simple evaluation of how local heterogeneity may affect the steam injection rates across the site, assuming that the same injection pressure will be applied.
- The simulations are simple radial, cylindrical calculations based on a numerical solution for one steam injection well, as described in Heron, Heron, and Udell (2000) and USACE (2001b).

Table 3-2 indicates the achievable steam injection rates at the maximum allowable injection pressures for the scenarios described above.

Average steam injection rates per well in each of the zones after about 180 days of injection were estimated as follows:

Zone A: 900 lb/hr Zone B: 2,400 lb/hr Zone C: 4,800 lb/hr Zone D: 2,000 lb/hr Zone E: 12,800 lb/hr

For the E- and C-zone wells, a reduced injection pressure was used, since the maximum allowable pressure would lead to excessive injection rates (more than 25,000 lb/hr per well).

For design purposes, the wells will allow for at least 150 percent of the average injection rate listed above, leading to design injection ranges as follows:

Zone A: 300 to 1,500 lb/hr Zone B: 800 to 4,000 lb/hr Zone C: 1,600 to 6,000 lb/hr Zone D: 600 to 3,000 lb/hr Zone E: 4,000 to 16,000 lb/hr

For the E-zone injection wells, the need to exceed the estimated injection rate is less important than it is for the upper zones, since the permeability is sufficiently high that all injection wells are expected to allow for injection at the design rate.

3.3.3 Radius of Influence and Choice of Well Spacing

The radii of the steam zones were calculated simultaneously with the injection rates described in Section 3.3.2. For design purposes, the most important parameter is the optimal distance between injection and extraction wells for each depth zone. For this purpose, the time needed for steam breakthrough to the nearest extraction well was estimated by making the steam zone radius equal to the well spacing.

The criteria used to choose the well spacing were the following:

- The well spacing cannot significantly exceed the predicted radius of influence after 90 days of steam injection for the simulation using the average hydraulic conductivity values for that particular depth interval.
- The well spacing will allow for steam breakthrough within 360 days in the scenario using one-third of the average hydraulic conductivity.

Based on the results provided in Table 3-2 and Appendix B, the following maximum well spacings were achieved for each zone:

Zone A: 52 feet Zone B: 83 feet Zone C: 112 feet Zone D: 69 feet Zone E: >300 feet

For practical purposes, it is desirable to group the depth zones so only two or three different well spacings are used. This allows for a logical well-field layout and minimizes the area occupied by wells and the piping from the wells to the steam and treatment systems, and it allows for access to the individual wells during operation. In addition, the E-zone injectors were predicted to allow for almost unlimited steam injection rates. For that purpose, a set of simulations was run with a lower injection pressure (840 kPa compared to the maximum allowable 928 kPa). This simulation showed that at injection rates of about 12,500 lb/hr per well, steam will break through to a well 180 feet away within 100 days of injection. Thus, the following well separations were chosen:

Zone A: 60 feet Zone B: 60 feet Zone C: 120 feet Zone D: 60 feet Zone E: 180 feet

For zone A, the well spacing is slightly higher than the 52 feet calculated above, but in this case we expect a positive effect by upward steam migration from the underlying zones. In conclusion, the 60-foot separation seems to be a good compromise.

3.3.4 Predicted Steam Breakthrough for Each Depth Interval

Based on the simulations included in Appendix B and the chosen well spacings and injection pressures, the breakthrough times were estimated for each zone as follows, using the average hydraulic conductivity values:

Zone A: 116 days (60-foot separation) Zone B: 30 days (60-foot separation) Zone C: 88 days (120-foot separation) Zone D: 44 days (60-foot separation) Zone E: 105 days (180-foot separation) These times are for constant injection at the design pressures and may, therefore, be considered reasonable predictions of the earliest steam breakthrough in areas where the hydraulic conductivity value is similar to the average value given by Bailey (2000). Due to less than 100 percent operation for the steam injection system, temporary pressure reductions, less than average hydraulic conductivity values in many areas, and the intended reduction of steam injection rates in some wells to allow for a more uniform steam distribution, more realistic steam breakthrough times are listed below:

Zone A: 100 to 180 days Zone B: 30 to 60 days Zone C: 90 to 180 days Zone D: 40 to 80 days Zone E: 100 to 180 days

During operation, the steam migration in certain zones may be reduced in order to control the direction of vertical pressure gradients, to minimize the risk of downward NAPL migration, and to achieve uniform heating with a minimal fuel demand during the heat-up phase. For design purposes and for sizing the steam supply and effluent treatment systems, the overall steam injection rate and pumping rates were fitted for a heat-up time of 180 days as an average across the site. This is a reasonable compromise among the following factors:

- The desire to heat and remediate the site rapidly (to shorten the overall operation time)
- The need to minimize the number of wells (to reduce drilling and hardware/instrumentation cost)
- The desire to have steam and treatment system sizes in practical and economic ranges (to minimize the capital cost of equipment)
- An allowance for contingencies in the actual field performance of each depth interval and the performance of each of the wells

In conclusion, the chosen well separation will allow controlled heating of the five dominant aquifer zones, with steam breakthrough to extraction wells within 180 days after initiating steam injection. Contingencies were built in for poorer performance in some areas of the site.

3.4 ELECTRICAL RESISTANCE HEATING FOCUS

The purpose of ERH in Scenario 1 is to heat and remediate critical regions of the site that have permeabilities that are too low to ensure an effective steam sweep. In scenarios 2 and 3, ERH is extended to the region under Old Mormon Slough, at an elevation of -40 to -120 feet. Angled steam wells from the slough sides would be impractical for targeting these zones.

3.4.1 Electrical Heating Intervals and Areas

Scenario 1

ERH will be used to remediate the D-E aquitard in Scenario 1 because of the low permeability of the aquitard. The D-E aquitard is adjacent to the E-zone aquifer, which is presently used as a drinking water source; this makes immediate remediation of the D-E aquitard more critical than that of the other aquitards at the site. The D-E aquitard is impacted in areas 2, 3, and 4 of the MPA and in areas 2 and 3 of the N-MPA under Old Mormon Slough. The total area of the D-E aquitard impact is 124,000 ft² or just under 3 acres.

The D-E aquitard ERH electrodes will be located at the bottom of the boreholes that are used to install the steam injection wells and extraction wells in the D-zone aquifer. The electrodes will be electrically conductive from an elevation of -175 to -210 feet. The electrical current fans out slightly as it flows between the electrodes; this fanning results in strong electrical heating over the interval from about -163 to -222 feet of elevation. Electrical heating extends a few feet up into the D-zone aquifer to improve the steam sweep of its bottom surface. The total volume of D-E aquitard heating will be 271,000 yd³.

Scenarios 2 and 3

In Scenarios 2 and 3, ERH will be extended into the region below Old Mormon Slough from -40 to -120 feet elevation. In Scenarios 2 and 3, steam will be injected in the C-zone, D-zone, and E-zone aquifers below the slough. However, steam injection shallower than the C-zone aquifer is not practical for several reasons, as detailed in Appendix C:

• Under the slough, the soil level is obviously lower than it is in the uplands. The lower soil level provides less overburden pressure at any given depth. At low levels of overburden pressure, it is possible to liquefy the soil through steam agitation. The risk of soil liquification or steam breakthrough to the surface limits the use of steam injection under the slough to the C-zone aquifer and deeper.

- Angled wells from the shore will be used to inject steam under the slough. At depths shallower than the C-zone aquifer, the drilling angle would become excessive for conventional drilling methods.
- Although vertical steam wells could be installed in the slough, such wells would require thermal insulation to prevent boiling the slough water and mud, with resulting creosote releases. Although thermal insulation could be applied, its use would complicate the installation of the steam injection wells.

Steam injection requires a certain overpressure to force the steam to leave the casing and flow away from the well. ERH generates steam everywhere and there is no significant overpressure at the electrode. However, ERH is also capable of soil liquefaction if it is applied at very shallow depths under the slough. The risk of liquefaction limits the use of ERH to depths greater than a -40 foot elevation, or about 30 feet below the slough mud line.

Electrodes under the slough will be completed with vertical boreholes and will have two separate depth elements to allow independent control of the heating of (1) the B-C aquitard, and (2) the B-zone aquifer and thin A-B aquitard. The electrodes under the slough will also incorporate collocated extraction wells that will target the B-zone aquifer. These extraction wells will have nonconductive fiberglass casings above an elevation of -52 feet to prevent any electrical current flow and resultant heating. Pumps in the electrode extraction wells will have nonconductive hoses.

The total area of ERH treatment under the slough in Scenarios 2 and 3 will be $41,000 \text{ ft}^2$, or just under 1 acre. The total volume of ERH treatment under the slough will be about 122,000 yd³.

3.4.2 Power Injection Rates

The ERH system is designed to produce a heat-up rate similar to that of the steam injection system. Coordinating the heat-up rates by steam and ERH will heat the site more uniformly and reduce condensation zones that might cause migration of NAPL into cooler regions. The ERH power control system will have sufficient capacity to heat the ERH regions to the boiling temperature of water in 180 days, if operated continuously at full power. In practice, about 365 days are scheduled for heat-up in order to account for ERH downtime for ERT shots and maintenance and to allow a margin of safety in the schedule.

ERH is commonly applied using either three or six phases of electricity. Six-phase heating is wonderfully optimized for the remediation of a small circular area. For other geometric shapes or for large regions, a disadvantage of the six-phase heating method is an inherent non-uniform heating pattern.

Three-phase heating is preferred for the remediation of large and irregularly shaped regions and has been used for all successful ERH remediations beyond the pilot scale.

In Scenario 1, the D-E aquitard ERH power control system will have a capacity of 5,100 kW (17.4 MM BTU/hr). There will be very low heat losses during the heating of the D-E aquitard because the top and bottom of the aquitard will be heated by steam. Only 6 percent of the input energy will be lost via thermal conduction through the sides of the treatment volume. In Scenarios 2 and 3, this D-E aquitard ERH system will be supplemented by a 2,300-kW (7.9 MM BTU/hr) ERH power control system. The heat losses during the treatment of the area under Old Mormon Slough will be about 29 percent of the input energy. Since the slough region is at the remediation boundary, it will be subject to greater thermal conduction losses than a more central region would be. However, it is the aggressive groundwater extraction from under the slough that accounts for the majority of the heat losses; hot water that is pumped from under the slough will be replaced by cool water from the slough itself.

3.4.3 In Situ Steam Generation and NAPL Displacement

The ERH system is very similar to the steam injection system in operation. The in situ steam production rates are equal to the steam injection rates on a per unit volume basis. In a steam injection system, all of the steam is introduced at the injection wells and it sweeps toward the extraction wells. In a classic ERH system, the steam is produced uniformly throughout the target area and then it sweeps toward the extraction wells. In the interest of overall cost savings, the spacing between the ERH electrodes has been increased by a factor of about 2.7 in this remediation (60-foot spacing versus typical 22-foot spacing). As a result, the steam production is no longer completely uniform. About 55 percent of the ERH steam will be produced within 10 feet of the electrodes. Because most of the steam will be produced near the electrodes, the ERH steam flow pattern at the McCormick and Baxter site will resemble that of a steam injection system—from the electrodes to the extraction wells.

Although much of the ERH energy will be deposited near the electrodes, almost half of the energy will be deposited in soils that are farther away. This energy will raise the subsurface temperatures to boiling and drive steam generation in the target soils, regardless of their permeability. In fact, low-permeability sediments tend to be more electrically conductive than sands and thus attract greater current for faster heat-up and stronger boiling.

Steam that is produced in low-permeability sediments will force its way out into the sandier deposits. In doing so, the steam will purge the NAPL out of fractures and thin channels that are typically the most difficult to remediate.

In each case, the power input rates are well within the practical limits of ERH, and no particular difficulty is expected in maintaining the desired heat-up and steam generation rates.

3.5 EXTRACTION APPROACH

3.5.1 Liquid Extraction

Liquid extraction during thermal remediation is crucial, for the following reasons:

- It is mandatory to maintain hydraulic control in order to minimize losses of contaminated fluids to surrounding areas.
- Steam condensate will contain high concentrations of COCs and needs to be recovered.
- NAPL may be extracted directly due to the increased mobility at the elevated temperatures and in the pressure fields induced by pumping. NAPL recovery as a liquid is desirable since the NAPL will contain nonvolatile components as well as the more volatile COCs that may be removed by steam distillation/stripping. As such, liquid NAPL recovery enhances the removal of the heavy PAHs substantially.

During the cool-down phase following thermal remediation, liquid pumping is also desirable for preventing the spread of leftover COCs, and since active cooling of the site is desired, liquid extraction provides the heat-transfer medium (cool water entering, hot water being extracted) needed to accomplish cooling in a timeframe of a few years.

Liquid extraction will be conducted for all depth intervals spanning from the vadose zone to the E-zone (an approximate elevation of 0- to -260 feet). Since the exact location of NAPL and high-concentration COC zones will never be known in sufficient detail to surgically install short extraction wells, it is important to install extraction screen intervals throughout the depth intervals, including the aquitard zones believed to have very low hydraulic conductivity (e.g., the D-E aquitard).

Each extraction well will be equipped with a minimum of one down-hole pump. The needed maximum design extraction rates for these pumps by depth zone are as follows (ranges represent all three treatment scenarios with varying numbers of injection and extraction wells):

Zone A: 2.4 to 3.4 gpm (average 0.6 to 0.8 gpm) Zone B: 6.4 to 7.2 gpm (average 1.6 to 1.7 gpm) Zone C: 6.0 to 7.7 gpm (average 1.3 to 2.0 gpm) Zone D: 8.1 to 10 gpm (average 1.9 to 2.5 gpm) Zone E: 55 to 72 gpm (average 14 to 18 gpm) For this calculation, it was assumed that all the extraction wells in a depth zone will extract 150 percent of the total liquid steam injection rate for the injection wells in that zone. For instance, if the total steam injection rate in zone A is 10,000 lbs/hr, equivalent to 20 gpm, the total extraction rate will be at least 1.5 times 20 gpm, or 30 gpm for this zone. If the well-field has a total of 40 extraction wells in the thermal remediation area, the average extraction rate will be 30 divided by 40, or 0.75 gpm.

For zones A, B, C, and D, pneumatic positive displacement pumps rated for 10 to 12 gpm will be acceptable. For the deepest E-zone, much higher pumping rates will be needed. A turbine pump (or equivalent) rated at between 50 and 70 gpm is expected. The design criteria for selecting the pumps are as follows:

- The pumps will allow pumping at the design rates listed above.
- Pumps will minimize emulsification during liquid extraction.
- All materials will be compatible with the elevated temperatures and chemicals. Temperatures in each zone will be as high as:
 - Zone A: 115°C
 - Zone B: 134°C
 - Zone C: 154°C
 - Zone D: 160°C
 - Zone E: 171°C
- The pump dimensions will allow placement in 6-inch-outside-diameter (OD) stainless steel wells.
- NAPL recovery will be optimized by design of the pump intake location. The pump intake will be adjustable depending on whether LNAPL or DNAPL is present in the extraction wells. At present, two extraction pumps per well are included in the design (this is to be tested during the pilot study at Wyckoff-Eagle Harbor Superfund site (USACE 2001b).
- Pumping rates will be adjustable in increments no larger than 10 percent of the maximum pumping rate either by direct setting of the pump speed/frequency or by automated adjustment based on the pressure in the fluid column.

The strategy for liquid extraction and the design around sensitive surface water bodies are discussed in Sections 3.7 (Old Mormon Slough Protection and Remediation), 3.9 (Hydraulic and Pneumatic Control), and 5.4 (Contaminant Extraction).

3.5.2 Vapor Extraction

Vapors will be extracted from all extraction wells located in the thermal treatment zones and in adjacent extraction wells where thermal effects are expected due to outward migration of steam and condensate. The vapor extraction will be facilitated by the connection of a vacuum extraction port on the wellheads. Extracted vapors will include atmospheric air entering the extraction wells from the vadose zone, steam that breaks through to the extraction wells, vaporized contaminants, and inorganic gases (such as nitrogen, carbon dioxide), and the organic gase methane exsolved from the groundwater during thermal treatment and vacuum extraction.

Besides controlling contaminant-laden vapor migration at the site (Section 3.9), the vapor extraction will assist in liquid recovery from wells screened in low-permeability zones by vacuum-enhanced recovery.

Vapor flow rates are difficult to predict in thermal remediation projects due to several factors:

- In the vadose zone, infiltration rates are affected by surface construction and by the design of gravel or vapor caps.
- In deeper zones, the time and magnitude of steam breakthrough are difficult to estimate accurately.
- The flow of injected air is somewhat unpredictable.
- Degradation reactions producing carbon dioxide and methane are difficult to estimate.

In conclusion, the sizing of the vapor extraction system and associated hardware is based on several rough assumptions, which are provided in Section 3.10.

As later shown in Section 5.4, the applied vacuum at each well will be used to control contaminant migration and adverse thermal effects across the site.

3.6 SUBSURFACE MONITORING

The steam front progression and the zones that become heated will be monitored by ERT and by distributed temperature sensing (DTS). ERT generates two-dimensional and three-dimensional images of the subsurface, whereas DTS generates data that can be viewed numerically, with the ERT images, or graphed on temperature distribution maps. Detailed descriptions of these two methods are provided in Appendix D.

Subsurface monitoring has four goals:

- Observing heat-up of the entire site on a large scale, with general site coverage from the surface to the bottom of the treatment zone.
- Monitoring property boundaries to prevent off-site migration of steam and hot water, using a vertical "fence" of monitoring surrounding the thermal treatment zones.
- Monitoring the subsurface beneath the south bank of Old Mormon Slough, thereby ensuring that potential lateral migration of steam is prevented, using a vertical electrode array (VEA) "fence" along the southern side of the slough for detailed monitoring of the A-, A-B-, and B-zones.
- Detecting any initial heat-up of upward steam migration from depth to the slough sediments, with a horizontal ERT plane under the slough.
- Providing data for operational decisionmaking.

Overall, the subsurface ERT and temperature monitoring are intended to prevent adverse impacts on surrounding receptors due to thermal treatment and to optimize remediation by providing guidance for operations.

3.6.1 ERT Monitoring

The VEAs needed for collecting ERT data should be placed in a configuration that maximizes the distance between any two boreholes without losing image resolution. The number of electrodes in the VEAs for each zone, A through E, is also driven by the resolution desired.

Table 3-3 indicates the desired resolution for each of the four goal areas and other suggested design parameters. This table was designed with the idea that the closer the spacing of electrodes, the greater the resolution, but also the greater the sensitivity to changes in resistivity, as affected by temperature. The total number of VEAs is indicated for each of the three scenarios. The VEA layouts proposed for each of the three scenarios are summarized in Figures 3-18 to 3-20. Those VEAs used for monitoring the Site Perimeter are part of the entire site network.

Using these configurations, VEAs would be placed around the perimeter of the steam injectors and extractors. Inside this perimeter, VEAs would be arranged in a network spaced at a set distance from each other to ensure spatial coverage of the entire site. The spaces would be smaller for the shallower boreholes, and larger for the deeper boreholes. The spaces between electrodes would vary for the four different goal areas, smaller where resolution needs to be finer and larger where resolution can be coarser.

Shallower VEAs, especially for zones A and B, would have the benefit of providing better resolution in these shallower areas and would not require drilling deeper than the areas of interest. However, shallower VEAs would require tighter spacing, which would require the emplacement of more boreholes on the site with a consequent increase in the total amount of drilling. In scenario 3 for example, using a 75-foot VEA separation in the CPA, S-CPA and OWPA, where contamination does not extend below –120 feet, would require 54 more VEAs and 3,600 more feet of drilling than the number required for the 180-foot separation layout proposed. With fewer, deeper VEAs, the whole site can be monitored with acceptable resolution and less total drilling than if the holes were shallower, closer together, and more numerous.

The slough ERT resolution is a separate issue, since we are dealing with horizontal ERT planes. The resolution is a compromise between high resolution (by having close electrode separation and many strings of ERT electrodes) and not plowing the entire slough for ERT electrode installation.

The ERT electrodes are typically constructed of 1-foot lengths of stainless steel tubing affixed with a holding screw to a fiberglass rod that provides a rigid structure from the top of the borehole to the bottom depth. The electrodes, spaced at regular intervals along the fiberglass rod, are each electrically connected to the surface with individual Teflon-coated wires that are soldered on and protected with high-temperature silicon and heat shrink tubing. The spaces between electrodes are further protected with heat shrink tubing. Each VEA is lowered into a borehole and grouted into place. From the wellhead, the wires are extended to an on-site measurement trailer for data collection.

ERT data would be collected using an automated system that is housed in the data collection trailer. This system, consisting of a transmitter, receiver, multiplexer, and power supply, would be fully computer controlled. Data collection would be automated for each data plane between two boreholes, or set of data planes. Once data are collected, the data would be transferred to the inversion code for reduction. Finally, the results would be imaged into tomographic cross sections, or three-dimensional images, for interpretation.

3.6.2 Thermal Monitoring

Thermal monitoring would be focused in the following areas:

- Temperature measured in all VEAs and ERT locations
- Temperature in injection and extraction wells

- Temperature in new pump-and-treat extraction wells placed in the A-, B-, and C-zones
- Temperature in new monitoring wells installed to verify hydraulic control and dissolved COC concentrations
- Temperature measured in dedicated boreholes placed in ERT data planes for calibration/verification of actual temperatures

It is suggested that temperature data be collected using DTS instead of arrays of individual thermocouples. DTS is a proven technology in oil field applications, where it has been used in enhanced recovery, in conditions very similar to those encountered in steam remediation (Normann, Weiss, and Krumhansl 2001). It requires less labor to collect data and generates more reliable data quality than is possible using thermocouples alone.

To outfit the site with DTS, a high-temperature plastic-coated fiber optic cable would be attached to the side of a VEA. The fiber would run from the top of the borehole to the bottom, and then it would loop back up to the top. The loop would have a minimum 2.5-inch-diameter turn to enable the optically scattered data to be collected properly. At the surface, the fibers from several wells could be spliced together to form a chain of wells, with the cumulative fiber lengths totaling 5 km, each extending to the measurement trailer. The more boreholes that can be "daisy-chained" together, the better for optimizing channel usage on the DTS data collection system, as one length of 5-km fiber occupies only one channel.

It is important to collect temperature data particularly in any injection or extraction well that lies within ERT planes. These data can be used as independent confirmation of the temperatures inferred by interpretation of the ERT data.

3.6.3 Groundwater COC Monitoring

Groundwater quality is evaluated through analysis of extracted water from hundreds of wells during operation. Therefore, the only dedicated monitoring wells are the once located outside the thermal treatment area, as described in Section 6.8.

3.7 OLD MORMON SLOUGH PROTECTION AND REMEDIATION

Since NAPL is located under Old Mormon Slough and in the adjacent sediments, thermal treatment involves facing the risk of pushing NAPL into the slough with adverse ecological effects. This section describes the options for minimizing the risk of adverse impacts, and the chosen solution.

3.7.1 Dewatering Option

The option of dewatering the slough to prevent adverse effects on the surface water during thermal treatment was rejected due to the excessive cost and difficulty involved. Appendix E provides more details of this evaluation.

3.7.2 Thermal Treatment Under Slough

All three treatment scenarios include treatment of the N-MPA area under Old Mormon Slough (Section 3.1). However, Scenario 1 includes thermal treatment for the two easternmost subareas only (N-MPA 2 and 3) and only the deeper zones below -120 feet of elevation (the C-, D-, and E-zone aquifers and the D-E aquitard). Scenarios 2 and 3 involve treatment below -30 feet of elevation, with extraction above this depth for NAPL recovery. The scenarios are summarized in Table 3-4.

The C-D aquitard is considered too thin to justify electrical heating in any of the scenarios.

The selection of angled drilling for steam injection wells and ERH electrodes for the zones below the B-C aquitard is based on limitations of the angle during drilling. Details and justification are provided in Appendix F.

All scenarios involve thermal treatment along the southern boundary of the slough. The treatment depth intervals along the west-east running boundary vary from -10 to -80 feet in the OWPA and CPA to -260 feet of elevation in the MPA. The three main tasks for protecting the slough are the following:

- Prevent northern migration of steam and NAPL from the thermal treatment zones into the slough sediments and water.
- Prevent upward migration of steam, condensate, and NAPL from the thermal treatment zones located below the slough.
- Minimize or prevent potential mobilization of NAPL resulting from the boiling of water around wells installed directly in the slough.

The design accounts for these potential adverse effects by the following:

• Thermal treatment will not occur in the A-zone aquifer under the slough. Only extraction will be performed in the A-zone aquifer, and a constant downward gradient will be maintained by ensuring that the A-zone wells are pumping during the thermal remediation.

- Steam injection will not be performed immediately south of the slough. Extraction wells will be installed as a row of guard wells as close to the slough as practical after installation of the sediment cap and the associated soil grading and placement of rip-rap, etc.
- For treatment of the shallower zones under the slough (Scenarios 2 and 3), where angled drilling is impractical due to the slope of the boreholes, only electrical heating will be used for heating. This was chosen because electrical heating can be completed through vertical boreholes installed in the slough without substantial heating of the shallow portion of the boreholes. Steam injection wells would potentially lead to heating of the sediments surrounding the wells, and boiling of the sediment pore water could lead to upward spreading of NAPL from the shallow sediments into the surface water.
- Any potential upward migration of NAPL and condensate around the ERH electrodes will be mitigated by designing the ERH boreholes as both extraction wells and electrodes. Therefore, by extracting liquids and vapors from shallow depths around the boreholes, the sediment will remain cool, and preferential migration of NAPL upward along the borehole will be eliminated.

In conclusion, it was determined that thermal treatment adjacent to, and under Old Mormon Slough, is practical with the present design.

3.7.3 Merits of Sediment Cap Installation in Slough Before or After Thermal Treatment

A brief description of the sediment cap is found in (USACE 2000c):

The selected sediment remedy consists of in situ capping of contaminated Old Mormon Slough sediments in order to isolate areas of principal-threat wastes (approximately three-fourths of the slough) by blanketing them with a minimum of 2 feet of clean fine sand. The cap materials would be armored with rip-rap and gravel filter layer where needed to prevent erosion. The portion of the slough to be capped would run from just north of the oily waste pond (OWP) area to the east end of the slough.

COCs for the sediment capping are PAHs, chiefly low-molecular weight PAHs, and polychlorinated dioxins and furans.

For our evaluation of risks associated with thermal treatment under the slough, it is obvious that the Old Mormon Slough water quality could be compromised in the following cases:

- NAPL migrates into the slough and forms either a sheen, a floating LNAPL layer, or a DNAPL cover on top of the sediment cap. This would carry PAHs and potentially dioxins and heavy metals into the surface water above the sediment cap. If the NAPL spread across the entire slough and settled on top of the cap, the entire cap would be compromised.
- Steam channeling to the slough occurs, and a substantial amount of sediment is released upward to the slough by hydrodynamic forces. This would potentially involve upward migration of sediment-associated COCs such as dioxins, PAHs, and heavy metals. If the sediment settled across the entire slough on top of the cap, the entire cap would be compromised. If the escape and settlement was localized, only a part of the sediment cap would be compromised.
- A substantial amount of water migrates upward into the surface water through the sediment cap, carrying soluble COCs, such as PCP.
- COC diffusion into the slough is encouraged by the thermal remediation operations.

Consequently, the objectives for protecting the slough during thermal treatment are relatively simple:

- Prevent NAPL migration into the slough.
- Prevent steam escape to the slough.
- Prevent heating the sediment cap to temperatures that would lead to NAPL boiling and upward escape.
- Prevent upward hydraulic gradients across the sediment cap that would lead to advection of COC-laden fluids into the slough.

The main argument for installing the sediment cap prior to thermal treatment is that the installation will prevent potential releases at least 5 years earlier than if the cap is placed after thermal treatment, with the increased benefit to the environment.

3.7.4 Monitoring and Safety Features of Design

Adverse impacts on the Old Mormon Slough will be minimized by the actions described in Section 3.7.2. In addition, the following monitoring and contingencies are included in the design:

- A row of monitoring boreholes will be installed along the slough to document the temperatures along the southern boundary of the slough. This will allow near-realtime monitoring in a vertical transect and an opportunity for advance warning if steam or hot water begins to migrate toward the slough. The monitoring will be used to guide nearby steam injection and extraction rates, so adverse effects can be minimized or eliminated (see Figures 3-18, 3-19, and 3-20 for locations).
- A horizontal ERT monitoring grid will be installed under the slough to provide high areal resolution of the monitoring under the slough.
- Boreholes completed in the slough will incorporate extraction wells in the A-zone. Groundwater extraction from the A-zone will provide three benefits:
 - Groundwater extraction will remove heat directly.
 - Groundwater extraction will exert hydraulic control and drive downward water flow from the slough
 - By monitoring the temperature of the extracted groundwater, operators will gain access to high-quality, hydraulic-conductivity-averaged subsurface temperature data. These data can be used to adjust A-zone pumping rates as needed to maintain the desired temperatures under the slough.
- A detailed mass balance will be maintained for the wells adjacent to the slough, ensuring that net extraction is maintained during thermal treatment. This mass balance is based on pumping rate measurements at each wellhead, steam injection rate monitoring, and simple calculations performed routinely as part of the daily data evaluation.
- Temperatures in the A- and B-zones under the slough will be monitored regularly during thermal treatment. Steam injection and electrical heating will be slowed if the temperature of the extracted water, or the temperature measured in the sediments, exceeds well-defined levels (e.g., 50°C in extracted water or at locations 10 feet or less below the mudline, or 30°C at locations 5 feet or less below the mudline).

• If steam or NAPL escapes to the slough, LNAPL will be contained using booms and skimming, and the spread of contaminant-laden sediments will be minimized by aggressive pumping using adjustable submersible pumps, with pump discharge into the effluent treatment system or temporary storage tanks. Any DNAPL that may settle on top of the surface cap will be sucked off the bottom, and fresh sediment cap material will be installed to replace it.

In conclusion, it is recommended that the sediment cap be placed prior to thermal remediation, and that the detailed subsurface monitoring under the slough and along the southern slough boundary be implemented.

3.8 SURFACE CAP

3.8.1 Cap Options

This section presents three options for the shallow soils at the site during thermal treatment and pump-and-treat:

- Leave the surface as is with minor leveling work (no cap option below).
- Install a low-permeability asphalt cap before treatment.
- Install a permeable gravel cap before treatment.

An evaluation matrix used for choosing the best option is provided in Table 3-5.

3.8.2 Recommendations

A surface cap is desirable due to the potential difficulty of heavy traffic during the rainy season. For this reason, as a minimum requirement, a gravel cap is recommended for most of the site prior to drilling activities. Such a cap is likely to function as a base for the final surface cap after thermal treatment and cool-down.

Due to the thick clay and silt layers under the top soils at the site, it is questionable whether a low-permeability asphalt cap will significantly affect the vertical movement of vapors, steam, and COCs. Therefore, the asphalt cap will be installed only in the portion of the CPA (Figure 3-3) that has very volatile NAPL components such as ethers (USACE 2000a). The main purpose of the cap in this area is to capture volatile contaminants during thermal treatment.

The approximate areas covered by the surface caps will be the following:

• The gravel cap will cover approximately $525,000 \text{ ft}^2$.

- The existing asphalt cap in the MPA covers $85,000 \text{ ft}^2$.
- The new low-permeability asphalt cap installed over a portion of the CPA will cover 38,000 ft².

The detailed design of the asphalt cap will be provided at a later stage in the design process. A simple asphalt cap with shallow vapor extraction has been assumed for cost estimating purposes.

3.9 HYDRAULIC AND PNEUMATIC CONTROL

A central aspect of the thermal remediation design is the prevention of escape of NAPL, steam, contaminated vapors, and hot condensate to surrounding areas. This is ensured by maintaining hydraulic control via pumping and by maintaining pneumatic control via vacuum extraction.

3.9.1 Net Liquid Extraction From Treatment Volumes

At all times, the cumulative amount of water extracted from the thermal treatment zones will exceed the cumulative amount of water injected as steam and hot water by at least 25 percent. However, since steam occupies a much larger volume than the associated water does in a liquid form, it is not sufficient to rely only on this net extraction across the site. During peak steam injection rates, more than 50 percent over-extraction will be performed, and the subsurface monitoring will be used to verify that the steam zones do not migrate off site. The treatment system will be designed to treat 50 percent more fluids than injected.

Due to the large depth and the presence of multiple aquifers at the McCormick-Baxter site, hydraulic control will be maintained on a depth-specific level. At least three distinct zones will be included in the mass balance calculations:

- The combined A- and B-zone aquifers and the A-B aquitard, where present
- The combined C- and D-zone aquifers and the C-D aquitard, where present
- The deep D-E aquitard and the underlying E-zone aquifer

Where steam or water migration is observed between the distinct depth zones, for instance by upward steam migration through the D-E aquitard into the D-zone aquifer, the mass balances will be corrected based on assumed flow rates and approximate cross-sectional areas of the vertical migration paths. This will be a highly uncertain calculation, and a substantial contingency factor will be added. For instance, if an estimated 10,000 lb/hr of steam migrates upward through the D-E aquitard, the liquid pumping rate in the D-zone aquifer will be increased by no less than the equivalent 20 gpm multiplied by a contingency factor of 2, in this case 40 gpm. The final choice of guidelines will be settled in a more detailed design.

It was assumed that the total net extraction necessary for each of the five aquifer zones can be reasonably estimated by Alternative 4 in the FS report (USEPA 1998), with an additional amount of pumping in each zone due to the potential outward push from the thermal treatment areas.

3.9.2 Vapor Extraction and Pneumatic Control

Vapor extraction will be preformed in all thermal treatment extraction wells and in those wells surrounding the thermal treatment zone for the following purposes:

- To capture contaminant-laden air and steam that can migrate to the wells under an applied vacuum
- To prevent escape of contaminant-laden gases to the atmosphere
- To pull atmospheric oxygen-rich air into the shallow soils in order to stimulate oxidation reactions in the vadose zone during remediation

Complete pneumatic control is not practical unless a surface cap is installed across the entire thermal treatment area. Local heterogeneity prevents a reliable prediction of the radius of influence of shallow vapor extraction points. However, as a measure of maintaining control with the injected air at this site, the total vapor extraction rate will exceed the total air injection rate (from co-injection with steam) at all times.

3.9.3 Mobile NAPL Extraction From Outside Treatment Volumes

The pumping from wells located outside the thermal treatment zone serves two purposes:

- To remove as much mobile NAPL as possible.
- To extract enough water to promote hydraulic control in all areas of the site, and in all depth zones where NAPL exists outside the thermal treatment zones. Hydraulic control would address potential increased mobility of NAPL during thermal remediation and cool-down in the event that heating occurs outside the thermal treatment zones.

For preliminary planning purposes, the placement, number, and pumping rates of supplemental NAPL extraction wells are based on a groundwater model developed by ICF Kaiser for a pumpand-treat scenario (USEPA 1998). Actual extraction well-field parameters (placement, number, and pumping rates of wells) will be refined by additional flow modeling to be completed during design to ensure that supplemental extraction wells do not compete with thermal extraction wells, thereby drawing NAPL outside the thermally treated zones. Operational extraction rates will be influenced locally by the location of heating wells and electrodes, and an evaluation of the potential for outward migration of fluids will be included in a later design stage.

Because it is assumed that all mobile NAPL will be removed under all scenarios in the D- and E-zones, supplemental extraction wells are not necessary for those zones. Zones A through C would have the approximate number of wells and pumping rates shown in Table 3-6 for the three scenarios.

3.10 SIZING STEAM, POWER, AND EFFLUENT TREATMENT SYSTEMS

This section describes the principles used to size the process equipment and the assumptions made in areas where predictions are difficult.

3.10.1 Treatment Volumes and Heat Capacity Calculations

A detailed calculation was made for each depth interval in each of the treatment zones, including the following:

- Heat capacity
- Total steam and power needed to heat the volume to steam temperature, with and without assumed heat losses
- Number of pore volumes of steam needed to heat the volume to steam temperature
- Total fuel demand to heat the volume and to flush a certain number of steam pore volumes or a certain amount of electrical energy through the volume
- The steam and electrical heating demand for each volume, assuming necessary heat-up periods of 90 and 180 days, respectively
- Total steam and electrical power rate demands to inject the necessary amount of steam and power, assuming total thermal treatment times of between 3 and 6 years and the amount of flushing defined as follows (details are discussed in Section 7):
 - Zone A: two pore volumes, 50 percent heat loss at fringes and to surface
 - A-B aquitard: twice the electrical energy needed for heat-up

- Zone B: two pore volumes, 15 percent heat loss at fringes
- Zone C: two pore volumes, 10 percent heat loss at fringes
- Zone D: three pore volumes, 15 percent heat loss at fringes
- D-E aquitard: twice the electrical energy needed for heat-up of the upper 85 percent of the aquitard, 6 percent heat loss
- Zone E: four pore volumes, 50 percent heat loss at fringes and downward
- Summed energy demands for steam and electrical heating for each of Scenarios 1, 2, and 3
- Minimum number of injection wells needed in each subvolume and area to meet the goal of delivering the energy during heat-up

Assumptions:

- The total sediment volume is heated. However, the aquitards will not be flushed with as many pore volumes of steam as the aquifers.
- Heat-up of one pore volume of groundwater was included in the energy calculation. In reality, some of the groundwater will be pumped out during heat-up, but this is in part compensated for by continued extraction during thermal treatment, allowing outside cool water to enter the treatment zone.
- Temperatures at depth were set equal to the saturated steam temperature (adjusted for the steam injection pressure, the depth below the surface, and the depth below the water table).
- Density, energy content, and pressure were calculated using standard tabulated values for steam.
- An average porosity of 35 percent.
- Rock heat capacity of 1,000 J/kg/K and water heat capacity of 4,186 J/kg/K.
- Heat of vaporization of 2,230 kJ/kg.
- Steam generator energy efficiency of 85 percent and heat loss in pipes of 5 percent.

• ERH power control system efficiency of 97 percent and energy loss in cables of 2 percent.

The results are provided in Table 3-7.

3.10.2 Well and Electrode Spacing

The criteria for setting the spacing of injection wells is described in Section 3.3. In summary, 60-foot spacing was chosen for the A-, B-, and D-zones; 120-foot spacing was chosen for the C-zone; and 180-foot spacing was determined to be appropriate for the E-zone.

For electrical heating electrodes, the spacing was determined in Section 3.4. It was concluded that a 60-foot separation is preferred. This will allow for collocation of wells and electrodes, reducing the drilling cost.

3.10.3 Steam Injection and Electrical Resistance Heating Rates

The final choice of heat-up period and total duration of thermal treatment is a tradeoff among several factors:

- If you want to treat a site rapidly, the size of steam generators and effluent treatment system can be cost prohibitive. For practical purposes, the largest available steam generators allowing for vapor destruction in their oxidation chambers have capacities of 50 MM BTU/hr, or 50,000 lb/hr. Other practical steam generator sizes are 25, 18, and 12 MM BTU/hr.
- The heating rate is limited by the rate at which one can extract the ambient water and steam condensate in order to maintain hydraulic control during expansion of the steam zones. The necessary pumping rate in each depth zone was calculated based on given steam injection rates and the number of extraction wells located in that zone.
- The heating rate of aquitard layers is limited by the power injection rates that may be achieved using the 60-foot electrode spacing. It was assumed that 365 days can be allowed for heating the aquitard layers electrically, and another 365 days is allowed for injecting an equivalent amount of energy into the layers for in situ steam production and flushing. This is a practical rate for ERH at a site of these dimensions.

The detailed calculations are provided in Table 3-7.

In conclusion, the following heating scenario was chosen as a compromise among all the factors:

- Site heat-up will be accomplished in less than 365 days.
- Aquitards will be steam-flushed within 730 days with an energy amount equivalent to the heat-up energy.
- Steam will be flushed through the aquifers for a period of up to 3 years after initial site heating, or as long as it takes to inject the necessary amount of steam pore volumes in each of the depth zones.

The sizing of the steam and power systems is summarized in Table 3-8. Details of the ERH design are summarized in Table 3-9.

3.10.4 Extraction Rates and Control

The minimum extraction rates necessary to maintain hydraulic control within the thermal treatment areas are estimated as 150 percent of the equivalent steam injection rate. However, as discussed in Section 3.9, this extraction rate is not sufficient across the site. For sizing the effluent treatment system, the following were assumed:

- Extract a minimum of 150 percent of the equivalent injection rates.
- Add the pumping rate necessary for maintaining hydraulic control in areas that are not being treated thermally, estimated based on the FS (USEPA 1998).
- Add a contingency pumping rate for wells located near the property boundary where NAPL presence is expected or suspected based on prior investigations (USACE 2001c).
- Assume that the thermal treatment system will handle a maximum total of 40, 55, and 70 gpm of secondary water used in the process (generator blow-down, filter backwash water, excess seal water from liquid ring pumps, etc.) for Scenarios 1, 2, and 3, respectively.

The total effluent treatment system sizes for each scenario are indicated in Table 3-10.

The total rates were calculated as the injected water rate plus 235 gpm plus the miscellaneous process water usage. This over-extraction was chosen as the best compromise in order to provide hydraulic control on a sitewide basis. It means that the effluent treatment systems will be sized to extract more than 150 percent of the equivalent steam injection rate.

3.11 TREATMENT OF SHALLOW SOILS UNDER ASPHALT CAP IN MPA

During past source removal activities at the site, NAPL-impacted soil was placed in the MPA and capped with asphalt. Under the current cap are three areas with different characteristics:

- The track pit (under the western part of the cap). This area is characterized by the presence of only a thin layer of soil that was placed on top of the old grade and is intact with rails and other metal objects.
- The central section containing basements and large amounts of debris and metal objects. The basements were filled with soil, and more soil was placed on top to form a level surface, currently under asphalt.
- The repository, which is a deeper layer of heavily impacted soil placed on top of a thin plastic membrane, which probably has been compromised by the contact with NAPL. The repository is up to 15 feet deep.

The potential for treating these soils as an integral aspect of the thermal treatment is discussed below.

3.11.1 Properties of Shallow Soils

In the track pit area, the deposited materials consist of a compacted layer 1 to 3 feet in thickness containing a mixture of sands, silts, clays, gravel, and debris. It is assumed that permeability and NAPL content are highly variable. This layer is overlain by 0.5 to 0.9 feet of aggregate, which is in turn overlain by about 0.25 foot of asphaltic concrete (USEPA 1998).

The soils placed in the basements are expected to be similar to the shallow soils.

The soils placed in the repository are heavily NAPL-impacted and believed to have a higher sand content than the average soils since they contain sandy soil excavated from the OWPA. Thermal treatment of the soils in the repository is highly desirable.

3.11.2 Underlying Layers and Potential for Steam Penetration From Depth

Examination of available drilling logs (summarized in crosssections in USACE 2001c) that penetrate the area with the asphalt cap indicates that laterally extensive sand bodies are present at shallow depth beneath the northern and southern margins of the cap. These sand bodies range in thickness up to about 10 feet, pinching out to insignificant thickness along a roughly east-west trending line along the center of the asphalt cap. The near-surface sand bodies underlying the cap are separated from the sand bodies of the main A-zone aquifer by 10 to 20 feet of lowpermeability clay or silt. Thus, upward steam migration to the shallow soils from the A-zone injection wells is unlikely to be efficient, except in the area of the deeper repository.

Heating the shallow surface soils by thermal conduction from the underlying sand zones is not likely to be an effective way of treating the shallow soils. The thickness of the clay and silt layers may exceed 25 feet under the asphalt cap, which prohibits sufficiently rapid heating by thermal conduction. Also, due to the heat losses to the surface, it is unlikely that the temperature depth profile will be such that the shallow soils will reach steam temperatures.

This suggests that the likelihood of heating and treating below the asphalt cap using steam injection wells in the A- and B-zone interval is very small, except below the repository. Should the shallow soils require thermal treatment, a special design for heating and extraction will need to be designed.

3.11.3 Design Options for Shallow Thermal Treatment

Steam injection is rejected for heating the shallow soils in the track pit area due to the low expected hydraulic conductivity of the underlying silts and clays.

Electrical heating may be used to heat the target layer to steam temperature, but the recovery of the heated NAPL and COCs will be problematic due to the low soil permeability and the need for shallow extraction wells.

It is concluded that designing a focused thermal treatment for the shallow soils under the asphalt cap is a feasible option only for the repository and the areas with buried basements. Should steam find its way to the shallow soils during injection in the A- and B-zone aquifers, the design as it stands will allow for extraction under the cap and, therefore, a beneficial treatment of those areas.

For the areas with basements, five dual-purpose wells will be used to inject steam and extract fluids. The location of these wells will be determined at a later design stage, based on improved data on the depth and shapes of existing buried basement features. Therefore, the five wells have not been included in the figures.

For the repository, steam injection wells are located along the boundary, and extraction wells are located in a row along the central west-east axis. The extraction well spacing in this area was reduced to 30 feet to optimize recovery from the NAPL-rich soils (Figure 3-3). The wells in contact with the repository soils are expected to be operated similarly to the other wells located in the upper zone. More detail will follow at a later stage in the design.







Figure 3-2 Cross Section A-A' Showing Well Layouts in Scenarios 2 and 3

THERMAL TREATMENT TECHNOLOGY CONCEPTUAL DESIGN McCormick and Baxter Superfund Site Stockton, California


































Figure 3-18 Scenario 1: Layout of Vertical Electrode Arrays–28 Deep Holes, 13 Shallow, 4 Horizontal VEAs THERMAL TREATMENT TECHNOLOGY CONCEPTUAL DESIGN McCormick and Baxter Superfund Site Stockton, California





Figure 3-19 Scenario 2: Layout of Vertical Electrode Arrays–31 Deep Holes, 13 Shallow, 4 Horizontal VEAs THERMAL TREATMENT TECHNOLOGY CONCEPTUAL DESIGN McCormick and Baxter Superfund Site Stockton, California





Figure 3-20 Scenario 3: Layout of Vertical Electrode Arrays–42 Deep Holes, 21 Shallow Holes, 4 Horizontal VEAs THERMAL TREATMENT TECHNOLOGY CONCEPTUAL DESIGN McCormick and Baxter Superfund Site Stockton, California

		ſ	(
				Collocated	Collocated				
	SES	SES	SES	Injection and	Extraction and	New Perimeter	New NAPL		1
	Injection	Extraction	Dual Purpose	Electrical	Electrical Heating	Monitoring	Extraction	Deep	Shallow
Scenario 1	Wells	Wells	Wells	Heating Wells	Wells	Wells	Wells	VEA Hole	VEA Hole
Completion Zo	one								
А	53	2	0	0	0	8	5	0	0
В	54	69	0	0	0	10	4	0	13
С	13	14	0	0	0	10	0	0	0
D	3	11	0	32	13	5	0	0	0
Е	10	4	1	0	0	6	0	25	0
				Collocated	Collocated				
	SES	SES	SES	Injection and	Extraction and	New Perimeter	New NAPL		
	Injection	Extraction	Dual Purpose	Electrical	Electrical	Monitoring	Extraction	Deep	Shallow
Scenario 2	Wells	Wells	Wells	Heating Wells	Heating Wells	Wells	Wells	VEA Hole	VEA Hole
Zone of Compl	letion								
А	57	2	0	0	0	8	5	0	0
В	73	74	0	0	9	10	1	0	13
С	22	20	0	0	0	7	0	0	0
D	3	14	0	32	13	5	0	0	0
Е	10	4	1	0	0	6	0	27	0
				Collocated	Collocated				
	SES	SES	SES	Injection and	Extraction and	New Perimeter	New NAPL		
	Injection	Extraction	Dual Purpose	Electrical	Electrical	Monitoring	Extraction	Deep	Shallow
Scenario 3	Wells	Wells	Wells	Heating Wells	Heating Wells	Wells	Wells	VEA Hole	VEA Hole
Zone of Comp	letion								
Α	69	2	0	0	0	8	0	0	0
В	85	97	0	0	9	10	0	0	13
С	28	22	0	0	0	8	0	0	0
D	23	16	0	32	13	5	0	0	0
E	12	6	2	0	0	6	0	27	0

 Table 3-1

 Summary of Well Counts by Layer and Scenario

30 230 230

30

Е

F

260

260

136

136

48.0

48.0

	~		J 01 8	·····j									
				Hydr. Cond.						Steam	K sensitivity	range (c	larcy)
	Thickness	Тор	Bottom	ft/day	darcy	TOS (ft)	Pinj (psig)	Pinj (Pa)	T(K)	zone (ft)*	Min	Avg	Max
A	30	15	45	23	8.1	25	12.5	187,439	388	15	2.7	8.1	2
A-B	5	45	50										
В	25	50	75	40	14.1	60	30	308,035	407	12.5	4.7	14	4
B-C	40	75	115										
С	20	115	135	43	15.2	125	62.5	531,997	427	10	5.1	15	4
C-D	10	135	145										
D	20	145	165	12	4.2	155	77.5	635,365	433	10	1.4	4.2	1
D-E	55	165	220										

120

107.2

928,239

840,000

448

444

15

15

16

48

48

13

144

240

240

Table 3-2 Summary of Steam Injection Rate and Radius of Influence Calculations

Injection rates,	30 days (l	bs/hr)	Injection rates	, 90 days	(lbs/hr)	Injection rates, 180 days (lbs/hr)			Injection rates, 360 days (lbs/hr)		
Min	Avg	Max	Min	Avg	Max	Min	Avg	Max	Min	Avg	Max
354	965	2,620	327	898	2,450	313	861	2,357	301	831	2,280
975	2,660	7,346	910	2,490	6,917	873	2,401	6,680	844	2,325	6,486
1,959	5,300	15,004	1,837	4,995	14,200	1,770	4,826	13,750	1,715	4,685	13,373
790	2,162	6,139	737	2,029	5,788	709	1,956	5,592	685	1,895	5,431
20,030	55,800	156,000	18,875	52,800	148,000	18,230	51,100	144,000	17,700	49,700	140,000
	14,160			13,300			12,860			12,480	

RO steam	, 30 da	/s (m)	RO stear	n, 90 day	/s (m)	RO steam,	180 days	s (m)	RO steam, 360 days (m)			
Min	Avg	Max Min Avg Max			Max	Min	Avg	Max	Min	Avg	Max	
6.9	11.4	18.7	10.1	16.7	27.5	12.8	21.2	35.0	15.9	26.3	43.5	
11.3	18.5	30.7	16.3	27.0	44.8	20.6	34.0	56.6	25.3	41.9	69.8	
15.6	25.6	42.9	22.4	36.8	62.0	28.0	46.1	77.6	34.1	56.3	95.0	
9.7	16.0	26.9	13.9	23.0	38.7	17.3	28.7	48.7	21.1	35.0	59.2	
41.0	68.2	113.9	60.7	101.3	169.5	77.3	129.2	216.0	96.0	160.7	270.0	
	34.9			51.7			65.8			81.8		

Well spacing, 90	day breakth	nru (ft)	Well spacing, 180	day breakt	hru (ft)	Well spacing, 360	day breakt	hru (ft)	Max well spacing	Chosen
Min (ft)	Avg (ft)	Max (ft)	Min (ft)	Avg (ft)	Max (ft)	Min (ft)	Avg (ft)	Max (ft)	recommended	spacing
33.1	54.8	90.2	42.0	69.5	114.8	52.1	86.2	142.6	53.4	60
53.4	88.5	146.9	67.5	111.5	185.6	83.0	137.4	228.9	85.7	60
73.4	120.7	203.3	91.8	151.1	254.4	111.8	184.6	311.5	116.2	120
45.6	75.4	126.9	56.7	94.1	159.7	69.2	114.8	194.1	72.3	60
199.0	332.1	555.7	253.4	423.6	708.2	314.8	526.9	885.2	NA	180
	169.5			215.7			268.2		169.5	

Goal Area	Desired Resolution (ft)	VEA Depth (ft)	Avg. Spacing Between VEAs (ft)	Electrode Spacing (ft)	Number of Electrodes per VEA	Total Number of VEAs
1) Entire Site	5	280	180	20	14	1. 26 2. 31 3. 43
2) Site Perimeter	2.5	280	180	10	28	1. 18 2. 19 3. 26
3) South of Slough	2.5	120	66	5	24	1. 13 2. 13 3. 21
4) Under Slough	10	50 (horizontal plane)	NA	20	66 (total)	NA

Table 3-3Suggested Design Parameters for Reaching Monitoring Goals

Table 3-4
Thermal Treatment Under Old Mormon Slough

	Α	AB	В	BC	С	CD	D	DE	Е
Scenario 1	Extraction	Extraction	Extraction	Extraction	Extraction	Extraction	Steam,	Electrical	Steam,
	only	only	only	only	only	only	angled	heating,	angled
								angled	
Scenario 2	Extraction	Extraction	Electrical	Electrical	Steam,	Extraction	Steam,	Electrical	Steam,
	only	only	heating,	heating,	angled	only	angled	heating,	angled
			vertical	vertical				angled	
Scenario 3	Extraction	Extraction	Electrical	Electrical	Steam,	Extraction	Steam,	Electrical	Steam,
	only	only	heating,	heating,	angled	only	angled	heating,	angled
			vertical	vertical				angled	

Note: All injection and electrical heating zones will have liquid and vapor extraction as well.

Option	Advantages	Disadvantages	Evaluation
No cap	Least expensive option. Allows air infiltration which will fuel degradation reactions. Allows for visual inspection of surface and direct temperature measurement by hand-held probes or IR camera.	Air infiltration will be heterogeneous and not controllable. Heavy vehicle traffic may be difficult after precipitation. No surface fugitive emissions control. No control of surface steam escape.	Acceptable with detailed near-surface monitoring of temperatures and some contingency for steam and vapor escape control, and establishment of heavy vehicle traffic routes.
Low permeability asphalt cap	Controlled air infiltration to fuel degradation reactions. Allows higher vacuum applied in vadose zone and improves shallow soil remediation. Allows heavy vehicle traffic. Can be sloped for drainage. Controls surface fugitive emissions. Some control of surface steam escape. Public perception: Safer.	High capital cost. Need air infiltration ports with controls. Need draining and vapor collection system. Design may not be identical to the final remedy cap. Importance is questionable in areas with a competent clay/silt layer already blocking horizontal vapor flow.	Desirable in areas with shallow volatile NAPL, such as a section of the CPA. Already in place in large parts of MPA, including the locations where shallow NAPL- impacted soil is present.
Gravel cap	Allows air infiltration which will fuel degradation reactions. Provides better surface for heavy vehicle traffic. A gravel cap is likely to be suited as the base for a permanent, final cap which will be the remedy for shallow soils.	Air infiltration will be heterogeneous and not controllable. No surface fugitive emissions control. No control of surface steam escape.	Desirable for ease of heavy traffic.

Table 3-5 Evaluation Matrix for Considerations Regarding Surface Cap

	A-Z	lone	B-Z	lone	C-Zone		
	No. Wells	Avg Q	No. Wells	Avg Q	No. Wells	Avg Q	
Scenario 1	6	4	5	4	1	1	
Scenario 2	6	4	1	10	0	_	
Scenario 3	0		0	—	0		

Table 3-6Supplemental Mobile NAPL Extraction Wells

Note: Q is extraction rate in gallons per minute, derived from results of Scenario 4 of USEPA (1998) groundwater model.

2 Calculation of steam density MPA A AB B BC C D D E 3 Calculation of steam density R AB B BC C D D DE E 4 Thickness of aquifer Rt 30 63505 531997 653565 65565 655765 635656 65565 2565 265 265 265 265 265 265 265 265 265 265 265<		А	В	С	D	E	F	G	Н	1	J	к
3 Calculation of stam density n A AB B BC C CD D DE E 4 Thickness of aquifer ft 30 5 25 40 20 10 20 65 5 in p P Pa 187439 187440 308035 531997 635365 665365 684 6 in T C 118.2 118.2 134.2 134.2 153.4 160.0 170.0 160.0 170.0 170.0 170.0 170.0 170.0 170.0 170.0 170.0 170.0 170.0 100.0 100.0 100.0	2			MPA							-	
4 Thickness of aquifer ft 30 5 25 40 20 10 20 65 5 In P Pa 187439 187440 308035 531907 531907 633565 683565 684 6 In T K 391 407 407 427 427 433 433 7 C 118.2 114.2 134.2 153.4 153.4 160.0 120.0 120.0 120.0 130.759 130.759 130.759 130.759 130.759 130.759 120.061 120.061 2.407.220 6.086.925 2.600 12 Volume 14" 130.374 137.40 134 153 153 160 160 171.7 140 118	3	Calculation of steam density		A	AB	В	BC	С	CD	D	DE	E
5 Inj P Pa 187490 308035 531997 531997 633865 643356 6 Inj T K 331 301 407 407 427 433 433 7 C 118.2 118.2 134.2 134.2 153.4 160.0 160.0 8 F 244.7 247.7 273.5 308.0 308.0 32.53 4 9 Steam density kg/m ² 10.62 1.677 1.677 2.766 6169 7254 7254 11 Area ft ² 3.610.30 689.755 3.49.975 5.50.60 2.795.160 120.361 23.845 92 12 Volume ft ² 3.610.30 689.755 5.50.60 2.795.160 120.361 24.02.84 100 13 Cleanup volume yd ³ 133.734 22.881 129.400 2.005 135.25 4.4578 89.156 22.442 100 14 Tranbient	4	Thickness of aquifer	ft	30	5	25	40	20	10	20	65	30
6 Inj T K 391 391 407 407 427 427 433 433 7 C 118.2 118.2 134.2 154.4 153.4 160.0 160.0 7 8 F 244.7 224.7 237.5 277.5 308.0 308.0 320.0	5	Inj P	Pa	187439	187440	308035	308035	531997	531997	635365	635365	840000
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	6	Inj T	К	391	391	407	407	427	427	433	433	444
8 F 244.7 273.5 273.5 308.0 320.0 </th <th>7</th> <th></th> <th>С</th> <th>118.2</th> <th>118.2</th> <th>134.2</th> <th>134.2</th> <th>153.4</th> <th>153.4</th> <th>160.0</th> <th>160.0</th> <th>170.8</th>	7		С	118.2	118.2	134.2	134.2	153.4	153.4	160.0	160.0	170.8
9 Steam density kg/m ³ 1.062 1.077 1.677 2.766 2.766 3.253 3.253 10 Steam energy content kJ/m ³ 2369 2369 3740 3740 6169 7254 7254 11 Area ft ² 120,361 139,759 139,759 139,759 130,750 120,361 22,402.20 2608 2.407,220 6,086,925 2.800 13 Cleanup volume yd ³ 133,734 25,881 129,406 207,050 103,525 44,578 89,156 225,442 100 15 Tambient °C 22	8		F	244.7	244.7	273.5	273.5	308.0	308.0	320.0	320.0	339.5
10 Steam energy content kJ/m³ 2369 2366 3740 3740 6169 6169 7254 7254 11 Area ft² 120,361 139,759 139,759 139,759 139,759 120,361 120,361 93,645 93 12 Volume ft² 3,610,830 698,795 3,493,975 5,590,360 2,795,180 1,203,61 2,407,220 6,608,622 2,600 13 Cleanup volume yd³ 133,734 25,881 129,406 207,050 103,525 44,578 89,156 225,442 100 15 T ambient °C 118 118 134 134 153 150 160 160 16 T final °C 11,000 1	9	Steam density	kg/m ³	1.062	1.062	1.677	1.677	2.766	2.766	3.253	3.253	4.196
11 Area ft ² 120,361 139,759 139,759 139,759 120,361 120,361 93,645 93,645 12 Volume ft ² 3,610,830 698,795 3,493,975 5,590,360 2,795,180 1,203,610 2,407,220 6,086,925 2,800 13 Cleanup volume yd ³ 133,734 22,5841 129,406 207,050 133,274 44,578 89,156 225,442 100 15 T ambient °C 22	10	Steam energy content	kJ/m³	2369	2369	3740	3740	6169	6169	7254	7254	9356
12 Volume If* 3,610,830 698,795 3,493,975 5,590,360 2,795,180 1,203,610 2,407,220 6,086,925 2,800 13 Cleanup volume yd* 133,734 25,881 129,406 207,050 103,525 44,578 89,156 225,442 100 15 T ambient °C 22<	11	Area	ft ²	120,361	139,759	139,759	139,759	139,759	120,361	120,361	93,645	93,645
13 Cleanup volume yd ³ 133,734 25,881 129,406 207,050 103,525 44,576 89,156 225,442 104 15 T ambient °C 22	12	Volume	ft ³	3,610,830	698,795	3,493,975	5,590,360	2,795,180	1,203,610	2,407,220	6,086,925	2,809,350
15 T ambient °C 22	13	Cleanup volume	yd ³	133,734	25,881	129,406	207,050	103,525	44,578	89,156	225,442	104,050
16 T final °C 118 118 134 134 153 153 160 160 17 Rock heat capacity J/(kg K) 1,000 <th>15</th> <th>T ambient</th> <th>°C</th> <th>22</th> <th>22</th> <th>22</th> <th>22</th> <th>22</th> <th>22</th> <th>22</th> <th>22</th> <th>22</th>	15	T ambient	°C	22	22	22	22	22	22	22	22	22
17 Rock heat capacity J/(kg K) 1,000 </th <th>16</th> <th>T final</th> <th>°C</th> <th>118</th> <th>118</th> <th>134</th> <th>134</th> <th>153</th> <th>153</th> <th>160</th> <th>160</th> <th>171</th>	16	T final	°C	118	118	134	134	153	153	160	160	171
18 Water heat capacity J/(kg K) 4,186<	17	Rock heat capacity	J/(kg K)	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
19 Porosity (-) 0.35	18	Water heat capacity	J/(kg K)	4,186	4,186	4,186	4,186	4,186	4,186	4,186	4,186	4,186
20 Mineral density kg/L 2.65	19	Porosity	(-)	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
31 Calculation of steam need for heatup Image: colored steam need for heatup Image	20	Mineral density	kg/L	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65
32 Calculation of steam need for heatup kg/m³ 137 137 160 160 188 188 197 197 33 per m3 kg/m³ 137 137 160 160 188 188 197 197 34 pv water pv water 0.39 0.46 0.46 0.54 0.56 0.56 0.56 35 pv steam pv as steam 370 273 273 194 194 173 173 36 Whole site kg steam 1.4E+07 2.8E+06 1.6E+07 2.6E+07 1.5E+07 6.5E+106 1.3E+07 3.4E+07 1.7 37 Whole site kJ 3.1E+10 6.1E+09 3.5E+10 5.7E+10 3.3E+10 1.4E+10 7.7E+10 3.8 38 Whole site BTU 2.98E+10 5.82E+09 3.36E+10 5.43E+10 1.37E+10 1.37E+10 7.7E+10 3.58 39	31											
33 per m3 kg/m³ 137 137 160 160 188 188 197 197 34 pv water pv water 0.39 0.38 0.46 0.46 0.54 0.56 0.56 0.56 35 pv steam pv as steam 370 273 273 194 194 173 173 36 Whole site kg steam 1.4E+07 2.8E+06 1.6E+07 1.5E+07 3.5E+107 3.5E+10 7.7E+10 3.8E 38 Whole site kJ 3.1E+10 6.1E+09 3.5E+10 5.7E+10 3.3E+10 1.4E+07 7.7E+10 3.8E 39 7.7E+10 3.5E 39 5.82E+09 3.36E+10 5.67E+10 3.32E+10 7.60E+10 7.60E+10 3.78 45 Energy demand, no loss kJ 3.14E+10 6.08E+09 <td< th=""><th>32</th><th>Calculation of steam need for heatup</th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th></td<>	32	Calculation of steam need for heatup										
34 pv water pv water 0.39 0.39 0.46 0.46 0.54 0.54 0.56 0.56 35 pv as steam pv as steam 370 273 273 194 194 173 173 36 Whole site kg steam 1.4E+07 2.8E+06 1.6E+07 2.6E+07 1.5E+07 6.5E+06 1.3E+07 3.4E+07 1.7 37 Whole site kJ 3.1E+10 6.1E+09 3.5E+10 5.7E+10 3.3E+10 1.4E+10 3.0E+10 7.7F+10 3.8E 38 Whole site BTU 2.98E+10 5.82E+09 3.36E+10 5.43E+10 1.37E+10 1.37E+10 7.7F+10 3.58 39	33	per m3	kg/m ³	137	137	160	160	188	188	197	197	213
35 pv as steam pv as steam 370 370 273 273 194 194 173 173 36 Whole site kg steam 1.4E+07 2.8E+06 1.6E+07 2.6E+07 1.5E+07 6.5E+06 1.3E+07 3.4E+07 1.7 37 Whole site kJ 3.1E+10 6.1E+09 3.5E+10 5.7E+10 3.3E+10 1.4E+10 3.0E+10 7.7E+10 3.8 38 Whole site BTU 2.98E+10 5.82E+09 3.36E+10 3.15E+10 1.37E+10 2.85E+10 7.7E+10 3.58 39 Image: State and heatup rates / steam demand Image: State and he	34	pv water	pv water	0.39	0.39	0.46	0.46	0.54	0.54	0.56	0.56	0.61
36 Whole site kg steam 1.4E+07 2.8E+06 1.6E+07 2.6E+07 1.5E+07 6.5E+06 1.3E+07 3.4E+07 1.7 37 Whole site kJ 3.1E+10 6.1E+09 3.5E+10 5.7E+10 3.3E+10 1.4E+10 3.0E+10 7.7E+10 3.8 38 Whole site BTU 2.98E+10 5.82E+09 3.36E+10 5.43E+10 1.37E+10 2.85E+10 7.27E+10 3.8 39	35	pv steam	pv as steam	370	370	273	273	194	194	173	173	145
37 Whole site kJ 3.1E+10 6.1E+09 3.5E+10 5.7E+10 3.3E+10 1.4E+10 3.0E+10 7.7E+10 3.8 38 Whole site BTU 2.98E+10 5.82E+09 3.36E+10 5.43E+10 1.37E+10 1.37E+10 2.85E+10 7.7E+10 3.58 40 Site size and heatup rates / steam demand	36	Whole site	kg steam	1.4E+07	2.8E+06	1.6E+07	2.6E+07	1.5E+07	6.5E+06	1.3E+07	3.4E+07	1.7E+07
38 Whole site BTU 2.98E+10 5.82E+09 3.36E+10 5.43E+10 3.15E+10 1.37E+10 2.85E+10 7.27E+10 3.58 39 Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraint of the size and heatup rates / steam demand Image: Constraintof heatup	37	Whole site	kJ	3.1E+10	6.1E+09	3.5E+10	5.7E+10	3.3E+10	1.4E+10	3.0E+10	7.7E+10	3.8E+10
39	38	Whole site	BTU	2.98E+10	5.82E+09	3.36E+10	5.43E+10	3.15E+10	1.37E+10	2.85E+10	7.27E+10	3.58E+10
40 Site size and heatup rates / steam demand	39											
45 Energy demand, no loss kJ 3.14E+10 6.08E+09 3.54E+10 5.67E+10 3.32E+10 1.43E+10 3.00E+10 7.60E+10 3.78 46 Energy demand, no loss BTU 2.98E+10 5.76E+09 3.36E+10 5.37E+10 3.15E+10 1.43E+10 3.00E+10 7.60E+10 3.78 47 Estimated heat loss % % 50 0 15 0 10 0 15 0 47 Estimated heat loss % % 50 0 15 0 10 0 15 0 48 Heatup pr needed pv as water 0.39 0.36 0.46 0.54 0.55 0.56 49 Total pv designed pv as water 2.00 0.79 2.00 0.92 2.00 1.07 3.00 0.56 50 Boiler efficiency 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.85	40	Site size and heatup rates / steam dem	and									
46 Energy demand, no loss BTU 2.98E+10 5.76E+09 3.36E+10 5.37E+10 3.15E+10 1.36E+10 7.20E+10 7.20E+10 3.58 47 Estimated heat loss % % 50 0 15 0 10 0 15 0 45 48 Heatup pv needed pv as water 0.39 0.46 0.46 0.54 0.54 0.56 0.56 49 Total pv designed pv as water 2.00 0.79 2.00 0.92 2.00 1.07 3.00 0.56 50 Boiler efficiency 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.95	45	Energy demand, no loss	kJ	3.14E+10	6.08E+09	3.54E+10	5.67E+10	3.32E+10	1.43E+10	3.00E+10	7.60E+10	3.78E+10
47 Estimated heat loss % % 50 0 15 0 10 0 15 0 48 Heatup pv needed pv as water 0.39 0.39 0.46 0.46 0.54 0.56 0.56 49 Total pv designed pv as water 2.00 0.77 2.00 0.92 2.00 1.07 3.00 0.56 50 Boiler efficiency 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.95	46	Energy demand, no loss	BTU	2.98E+10	5.76E+09	3.36E+10	5.37E+10	3.15E+10	1.36E+10	2.85E+10	7.20E+10	3.58E+10
48 Heatup pv needed pv as water 0.39 0.39 0.46 0.46 0.54 0.54 0.56 49 Total pv designed pv as water 2.00 0.79 2.00 0.92 2.00 1.07 3.00 0.56 50 Bolier efficiency 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.85 0.9	47	Estimated heat loss %	%	50	0	15	0	10	0	15	0	50
49 Total pv designed pv as water 2.00 0.79 2.00 0.92 2.00 1.07 3.00 0.56 50 Boiler efficiency 0.85	48	Heatup pv needed	pv as water	0.39	0.39	0.46	0.46	0.54	0.54	0.56	0.56	0.61
50 Bolier efficiency 0.85<	49	Total pv designed	pv as water	2.00	0.79	2.00	0.92	2.00	1.07	3.00	0.56	4.00
51 Delivery efficiency 0.95 0.95 0.95 0.95 0.95 0.95	50	Boiler efficiency		0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
	51	Delivery efficiency		0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
22 Iotal BI U need wilosses BI U 37.05±11 1.43±10 2.14±11 1.61±11 3.36±10 2.21±11 8.92±10 5.84	52	Total BTU need w/losses	BIU	3.75E+11	1.43E+10	2.14E+11	1.33E+11	1.61E+11	3.36E+10	2.21E+11	8.92E+10	5.84E+11
35 lotal BLU need, riusning, no loss BLU 1.52E+11 1.15E+10 1.47E+11 1.07E+11 2.71E+10 1.52E+11 7.20E+10 2.36	23	I otal B I U need, flushing, no loss	BIU	1.52E+11	1.15E+10	1.4/E+11	1.0/E+11	1.1/E+11	2./1E+10	1.52E+11	7.20E+10	2.36E+11
34 protai bi u need no iosses BLU 2.98E+10 5.76E+09 3.30E+10 5.37E+10 3.35E+10 1.36E+10 2.85E+10 7.20E+10 3.58	54	I OTAL B I U NEED NO IOSSES	віО	2.98E+10	5.76E+09	3.36E+10	5.37E+10	3.15E+10	1.36E+10	2.85E+10	7.20E+10	3.58E+10
33	20	Steam domand 190 d boot in					40			-	47	0
	70	Steam demand, 180 d neatup		/	1	8	12	1	3	7	17	8

Table 3-7 Steam Injection and Electrical Heating Rate Calculations

 $W:\74206\0110.035\Section\ 3.doc$

	L	М	N	0	Р	Q	R	S	Т	U	V	W
2	CPA			N-MPA								
3	A	AB	В	A	AB	В	BC	С	CD	D	DE	E
4	35	5	35	30	5	25	40	20	10	20	65	30
5	187439	187440	308035	187439	187440	308035	308035	531997	531997	635365	635365	840000
6	391	391	407	391	391	407	407	427	427	433	433	444
7	118.2	118.2	134.2	118.2	118.2	134.2	134.2	153.4	153.4	160.0	160.0	170.8
8	244.7	244.7	273.5	244.7	244.7	273.5	273.5	308.0	308.0	320.0	320.0	339.5
9	1.062	1.062	1.677	1.062	1.062	1.677	1.677	2.766	2.766	3.253	3.253	4.196
10	2369	2369	3740	2369	2369	3740	3740	6169	6169	7254	7254	9356
11	97,739	97,739	97,739	41,263	41,263	41,263	41,263	41,263	30,166	30,166	20,628	20,628
12	3,420,865	488,695	3,420,865	1,237,890	206,315	1,031,575	1,650,520	825,260	301,660	603,320	1,340,820	618,840
13	126,699	18,100	126,699	45,848	7,641	38,206	61,130	30,565	11,173	22,345	49,660	22,920
15	22	22	22	22	22	22	22	22	22	22	22	22
16	118	118	134	118	118	134	134	153	153	160	160	171
17	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
18	4,186	4,186	4,186	4,186	4,186	4,186	4,186	4,186	4,186	4,186	4,186	4,186
19	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
20	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65
31												
32												
33	137	137	160	137	137	160	160	188	188	197	197	213
34	0.39	0.39	0.46	0.39	0.39	0.46	0.46	0.54	0.54	0.56	0.56	0.61
35	370	370	273	370	370	273	273	194	194	173	173	145
36	1.3E+07	1.9E+06	1.6E+07	4.8E+06	8.1E+05	4.7E+06	7.6E+06	4.4E+06	1.6E+06	3.4E+06	7.6E+06	3.7E+06
37	3.0E+10	4.3E+09	3.5E+10	1.1E+10	1.8E+09	1.0E+10	1.7E+10	9.8E+09	3.6E+09	7.5E+09	1.7E+10	8.3E+09
38	2.82E+10	4.07E+09	3.29E+10	1.02E+10	1.72E+09	9.92E+09	1.60E+10	9.29E+09	3.43E+09	7.14E+09	1.60E+10	7.90E+09
39												
40												
45	2.97E+10	4.25E+09	3.47E+10	1.08E+10	1.79E+09	1.05E+10	1.67E+10	9.80E+09	3.58E+09	7.53E+09	1.67E+10	8.33E+09
46	2.82E+10	4.03E+09	3.29E+10	1.02E+10	1.70E+09	9.92E+09	1.59E+10	9.29E+09	3.40E+09	7.14E+09	1.59E+10	7.90E+09
47	50	0	15	50	0	15	0	10	0	15	0	50
48	0.39	0.39	0.46	0.39	0.39	0.46	0.46	0.54	0.54	0.56	0.56	0.61
49	2.00	0.79	2.00	2.00	0.79	2.00	0.92	2.00	1.07	3.00	0.56	4.00
50	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
51	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
52	3.56E+11	9.98E+09	2.09E+11	1.29E+11	4.21E+09	6.31E+10	3.93E+10	4.77E+10	8.41E+09	5.53E+10	1.96E+10	1.29E+11
53	1.44E+11	8.06E+09	1.44E+11	5.20E+10	3.40E+09	4.33E+10	3.17E+10	3.46E+10	6.79E+09	3.80E+10	1.59E+10	5.20E+10
54	2.82E+10	4.03E+09	3.29E+10	1.02E+10	1.70E+09	9.92E+09	1.59E+10	9.29E+09	3.40E+09	7.14E+09	1.59E+10	7.90E+09
55												
70	7	1	8	2	0	2	4	2	1	2	4	2
71	10	0	6	4	0	2	1	1	0	2	1	4

Table 3-7 (Continued) Steam Injection and Electrical Heating Rate Calculation

W:\74206\0110.035\Section 3.doc

	Х	Y	Z	AA	AB	AC	AD	AE	AF	AG	AH	AI	AJ
2	S-MPA			S-CPA			=		E-MPA				
3	В	BC	С	A	AB	В	BC	С	C	CD	D	DE	E
4	25	40	20	30	5	25	40	20	20	10	20	65	30
5	308035	308035	531997	187439	187440	308035	308035	531997	531997	531997	635365	635365	840000
6	407	407	427	391	391	407	407	427	427	427	433	433	444
7	134.2	134.2	153.4	118.2	118.2	134.2	134.2	153.4	153.4	153.4	160.0	160.0	170.8
8	273.5	273.5	308.0	244.7	244.7	273.5	273.5	308.0	308.0	308.0	320.0	320.0	339.5
9	1.677	1.677	2.766	1.062	1.062	1.677	1.677	2.766	2.766	2.766	3.253	3.253	4.196
10	3740	3740	6169	2369	2369	3740	3740	6169	6169	6169	7254	7254	9356
11	80,481	80,481	80,481	21,853	40,233	88,851	70,471	70,471	115,068	115,068	115,068	115,068	115,068
12	2,012,025	3,219,240	1,609,620	655,590	201,165	2,221,275	2,818,840	1,409,420	2,301,360	1,150,680	2,301,360	7,479,420	3,452,040
13	74,519	119,231	59,616	24,281	7,451	82,269	104,401	52,201	85,236	42,618	85,236	277,016	127,853
15	22	22	22	22	22	22	22	22	22	22	22	22	22
16	134	134	153	118	118	134	134	153	153	153	160	160	171
17	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
18	4,186	4,186	4,186	4,186	4,186	4,186	4,186	4,186	4,186	4,186	4,186	4,186	4,186
19	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
20	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65	2.65
31													
32													
33	160	160	188	137	137	160	160	188	188	188	197	197	213
34	0.46	0.46	0.54	0.39	0.39	0.46	0.46	0.54	0.54	0.54	0.56	0.56	0.61
35	273	273	194	370	370	273	273	194	194	194	173	173	145
36	9.2E+06	1.5E+07	8.6E+06	2.6E+06	7.9E+05	1.0E+07	1.3E+07	7.5E+06	1.2E+07	6.2E+06	1.3E+07	4.2E+07	2.1E+07
37	2.0E+10	3.3E+10	1.9E+10	5.7E+09	1.8E+09	2.3E+10	2.9E+10	1.7E+10	2.7E+10	1.4E+10	2.9E+10	9.4E+10	4.6E+10
38	1.93E+10	3.13E+10	1.81E+10	5.40E+09	1.67E+09	2.14E+10	2.74E+10	1.59E+10	2.59E+10	1.31E+10	2.72E+10	8.94E+10	4.40E+10
39													
40													
45	2.04E+10	3.27E+10	1.91E+10	5.70E+09	1.75E+09	2.25E+10	2.86E+10	1.67E+10	2.73E+10	1.37E+10	2.87E+10	9.33E+10	4.65E+10
46	1.93E+10	3.10E+10	1.81E+10	5.40E+09	1.66E+09	2.14E+10	2.71E+10	1.59E+10	2.59E+10	1.30E+10	2.72E+10	8.85E+10	4.40E+10
47	15	0	10	50	0	15	0	10	10	0	15	0	50
48	0.46	0.46	0.54	0.39	0.39	0.46	0.46	0.54	0.54	0.54	0.56	0.56	0.61
49	2.00	0.92	2.00	2.00	0.79	2.00	0.92	2.00	2.00	1.07	3.00	0.56	4.00
50	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
51	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
52	1.23E+11	7.67E+10	9.30E+10	6.82E+10	4.11E+09	1.36E+11	6.71E+10	8.14E+10	1.33E+11	3.21E+10	2.11E+11	1.10E+11	7.18E+11
53	8.45E+10	6.19E+10	6.76E+10	2.75E+10	3.32E+09	9.32E+10	5.42E+10	5.92E+10	9.66E+10	2.59E+10	1.45E+11	8.85E+10	2.90E+11
54	1.93E+10	3.10E+10	1.81E+10	5.40E+09	1.66E+09	2.14E+10	2./1E+10	1.59E+10	2.59E+10	1.30E+10	2./2E+10	8.85E+10	4.40E+10
55		-				_	-						10
70	4	/	4	1	0	5	6	4	6	3	6	20	10
11	4	2	3	2	0	4	2	2	4	1	6	3	20

Table 3-7 (Continued) Steam Injection and Electrical Heating Rate Calculations

	AK	AL	AM	AN	AO	AP	AQ
2	OWPA			Totals			
3	А	AB	В	Scenario 1	Scenario 2	Scenario 3	
4	30	5	25				
5	187439	187440	308035				
6	391	391	407				
7	118.2	118.2	134.2				
8	244.7	244.7	273.5				
9	1.062	1.062	1.677				
10	2369	2369	3740				
11	85,187	85,187	85,187				
12	2,555,610	425,935	2,129,675	38,891,310	57,990,045	79,786,125	ft ³
13	94,652	15,775	78,877	1,440,419	2,147,779	2,955,042	yd ³
15	22	22	22				
16	118	118	134				
17	1,000	1,000	1,000				
18	4,186	4,186	4,186				
19	0.35	0.35	0.35				
20	2.65	2.65	2.65				
31							
32							
33	137	137	160				
34	0.39	0.39	0.46				
35	370	370	273				
36	1.0E+07	1.7E+06	9.7E+06	1.9E+08	2.8E+08	4.0E+08	kg steam
37	2.2E+10	3.7E+09	2.2E+10	4.3E+11	6.3E+11	8.8E+11	kĴ
38	2.11E+10	3.55E+09	2.05E+10	4.05E+11	5.93E+11	8.38E+11	BTU
39							
40							
45	2.22E+10	3.70E+09	2.16E+10	4.26E+11	6.23E+11	8.80E+11	kJ
46	2.11E+10	3.51E+09	2.05E+10	4.04E+11	5.90E+11	8.34E+11	BTU
47	50	0	15				
48	0.39	0.39	0.46	pv heatup			
49	2.00	0.39	2.00	pv design			
50	0.85	0.85	0.85				
51	0.95	0.95	0.95				
52	2.66E+11	4.35E+09	1.30E+11	2.61E+12	3.55E+12	5.15E+12	BTU
53	1.07E+11	3.51E+09	8.94E+10	1.43E+12	2.05E+12	2.89E+12	BTU
54	2.11E+10	3.51E+09	2.05E+10	4.04E+11	5.90E+11	8.34E+11	вти
55							
70	5	1	5	93	137	193	MM BTU/hr
11	8	0	4	75	101	147	IVIIVI BIU/hr 4 yrs

Table 3-7 (Continued) Steam Injection and Electrical Heating Rate Calculations

W:\74206\0110.035\Section 3.doc

Table 3-8					
Summary of Steam, Power, and Water Demand					

	Scenario 1	Scenario 2	Scenario 3
Steam demand during 360	93,000 lbs/hr	137,000 lbs/hr	193,000 lbs/hr
day heat-up	(100 MM BTU/hr gas)	(150 MM BTU/hr gas)	(200 MM BTU/hr gas)
Steam demand during	75,000 lbs/hr	101,000 lbs/hr	147,000 lbs/hr
years 2-4			
Power demand for	5,100 kW max	7,400 kW max	7,400 kW max
electrical heating during	2,550 kW avg	3,680 kW avg	3,680 kW avg
year 1 and 2			
Maximum water supply	205 gpm	310 gpm	440 gpm
for steam generation			

Table 3-9Electrical Heating Design Parameters and Power Demand

	MPA & N-MPA DE	N-MPA B/BC
Electrical Resistance Heating (ERH) Area:	123,811 sq. ft.	41,263 sq. ft.
Shallow Extent of ERH:	173 ft bgs	40 ft MLLW
Deep Extent of ERH:	232 ft bgs	120 ft MLLW
Typical Depth to Groundwater:	10 ft bgs	0 ft MLLW
Treatment Volume:	270,500 cu yds	122,300 cu yds
Estimated Number of Electrodes:	46	13
Electrode Boring Size:	10-inch o.d.	10-inch o.d.
Electrode Depth Interval Drill Cuttings:	50 tons	35 tons
Estimated Distance Between Electrodes:	60 ft	60 ft
Total Depth of Electrodes:	220 ft bgs	108 ft MLLW
Depth to Top of Electrodes:	185 ft bgs	52 ft MLLW
Electrode Steel Shot Required:	119 tons	54 tons
Liquid Groundwater Pumping Rate:	0 gpm	12.5 gpm
Controlling Contaminant:	NAPL	NAPL
Maximum Expected Temperature:	177°C	150°C
Rating of Power Control Units:	5,100 kW	2,300 kW
Average Electrical Heating Power Input:	2,550 kW	1,130 kW
Electrical Heating Steam Production Rate:	2,900 scfm	830 scfm
Time to Heat-up Treatment Volume:	365 days	365 days
Time to Boil Treatment Volume:	365 days	365 days
Total Heating Treatment Time:	730 days	730 days
Design Remediation Energy:	44,718,000 kW-hr	19,765,000 kW-hr

Table 3-10 Total Liquid Extraction Rates and Design Capacity of Effluent Treatment System

	Scenario 1	Scenario 2	Scenario 3	
Equivalent steam injection rate	186 gpm	274 gpm	386 gpm	
Design extraction rate	280 gpm 410 gpm		580 gpm	
Miscellaneous process water	40 gpm 55 gpm		70 gpm	
Over-extraction need from pump &	235 gpm	235 gpm	235 gpm	
treat estimate				
Total effluent treatment system	475 gpm	575 gpm	700 gpm	
capacity				

4.0 SURFACE PROCESS DESIGN

The design criteria for the surface treatment process are based on proven technology and many years of practical experience leading to a simple robust design. The design has been developed with consideration of daily operation and maintenance factors. The design has built-in flexibility for addressing the wide range of anticipated process conditions and unforeseen difficulties, and it consists of a modular approach, which provides the ability to scale changes in actual process conditions.

4.1 STEAM GENERATION

Steam will be needed for the thermal remediation process. Subsurface heating by the application of steam enhances the recovery of site contaminants via several thermodynamic mechanisms. Steam generation will be a constant operation, although the steam generation rate may vary depending on operational needs during the 4 years of steam injection.

4.1.1 Steam Purchased From Outside Source or Service Provider

A quote for purchasing steam was obtained from a boiler vendor close to the site. The vendor indicated that the charge for providing steam as a commodity to McCormick and Baxter at the design requirements would be \$7.00/1,000 lb of steam produced. The cost includes everything necessary to make 100,000 lb/hr of steam including the fuel (Scenario 1). Even though the quote for off-site steam generation compares favorably with the cost of on-site generation (see Section 4.1.2), the on-site steam generation option was chosen for inclusion in the scenarios because of the need for vapor destruction and the beneficial cogeneration option. A more detailed analysis and comparison will be made at a later design stage.

Currently, a steam line runs south of the McCormick and Baxter property line. The capacity and pressure of this line was investigated, with the following conclusions:

- The maximum amount of steam that could be delivered to the thermal remediation system is 20,000 lb/hr.
- The steam pressure would be in the range of 100 to 150 psig.
- The service is expected to be interrupted.

In conclusion, the off-site steam supply was not included in the design for any of the three scenarios.

However, the off-site supplier expressed interest and offered to increase the supply to accommodate the expected demand and should not be completely ruled out in the final design phase.

4.1.2 Steam Generation Using Purchased or Rented Boiler

Renting a boiler for up to 1.5 years would cost the same as purchasing the same boiler. This does not include the salvage value of the purchased boiler, which may decrease the rental time necessary for the rental cost to be equivalent to the purchase cost. For a project with 4 years of steam generation and the need for using the steam generator combustion chamber for destruction of COC-laden vapors, the optimal solution is to purchase the steam generators.

The cost estimate for purchasing steam production equipment and operating the steam plant (e.g., not purchasing steam as a commodity) indicated that the unit cost for steam would be \$8.31/1,000 lb of steam produced, excluding the cost for pretreating the feed water. However, it is assumed that the pretreatment would cost no more than \$1.00/1,000 lb of steam produced, based on 1999 fuel oil costs.

4.1.3 Boiler Type Selection

With a total anticipated maximum steam injection rate of 100,000 lbs/hr (Scenario 1), 150,000 lb/hr (Scenario 2), and 200,000 lb/hr (Scenario 3), it is recommended that the boiler plant consist of packaged 1,500-horsepower (hp) water tube boilers (each rated at about 50 million BTU/hr). Water tube boilers are, in general, less efficient and require more maintenance than fire tube boilers. Water tube boilers are traditionally used in high-pressure operations, such as deep oil exploration (thousands of psig steam pressure). The steam generation for the remediation will be a low-pressure process (maximum of 150 psig steam pressure). However, a water tube design is best suited for vapor recycling, as described in Section 4.5, and the high-pressure steam that is produced can be used to generate electricity, as described in Section 4.1.7.

Vapor recycling requires a 1 second or better residence time in the combustion chamber to fully reduce latent hydrocarbons in the vapor stream to nonharmful components before release to the atmosphere along with the boiler flue gas stream. This is best done in oil-field-style water tube boilers due to their large combustion chambers. Overall, oil-field-style water tube steam generators offer the best value for a 4-year thermal remediation project with the need to destroy vapors, and the benefit of using the power produced during the pressure reduction step.

4.1.4 Fuel Source

Boilers require a hydrocarbon fuel source to generate the high temperatures necessary to convert water to steam. The fuel source would most likely be natural gas. While propane has been

employed as a backup fuel source for natural-gas-fired boilers in this size range, propane as the primary fuel source is not planned. It is anticipated that the local utility company will provide the fuel. Bulk storage of propane would be more costly because the vessels must be pressure rated, and the cost of propane necessary for any given thermal capacity would exceed the cost of natural gas.

Should fuel prices change such that diesel would be a cheaper overall fuel source, this is a viable option as well. The final decision will be made based on current fuel prices at the time of the final design.

4.1.5 Fresh Water Source

Water is necessary for steam generation at the site. A sustained rate of between 200 and 400 gpm will be needed to make steam for up to 1 year, after which the requirement will decrease to between 60 and 75 percent of these amounts (see Section 4.6). The actual steam generation rate will vary depending on operational needs. It will also be necessary to have additional water available for boiler blow-down, wash down, and water softener regeneration. This water will be introduced into heat exchange equipment; therefore it must be as free of minerals and other chemicals as possible, to reduce the level of pretreatment required for the feed water. Pretreatment would be needed to limit the fouling of boiler heat exchange surfaces. Fouling makes heat exchange less efficient and thus increases the cost of making steam. More importantly, scaling may cause overheating of the boiler tube metal and subsequent tube failures resulting in boiler downtime and costly repairs. Since it is assumed that steam generation will be a nonstop operation during the heating phase of the remediation, the water supply must be uninterrupted during that entire time.

The fresh water supply will be city water. Other options were ruled out:

- Surface water from Old Mormon Slough was rejected because pumping of water would increase the need for boiler water pretreatment compared to using fresh water.
- Water from a deep well was rejected because it would not offer any advantage over treated effluent water.

The total water need for the different phases of operation is discussed in Section 4.6. The principles used to generate the numbers are as follows:

• Water will be recirculated to the extent practical, thereby minimizing both the water supply and the treated water disposal rate.

• The fresh water supply will be regulated so the potential scaling problems discussed above may be kept tolerable, while the disposal of treated water will be minimized.

4.1.6 Feed Water Treatment Systems

Feed water for the boiler will require pretreatment due to natural occurring minerals. A sample of the untreated feed water should be analyzed during the treatment design to provide data that will allow proper design of a pretreatment system. Depending upon the analysis, various pretreatment methods may be used to prepare make-up water for the boiler feed water system. The objectives of boiler water treatment are as follows:

- Prevent hard scale deposits or soft sludge-type deposits that would impair the rate of heat transfer and possibly result in overheating and damage to the pressure vessel.
- Prevent general corrosion or pitting to ensure maximum life of the boiler at the lowest maintenance cost.
- Prevent intercrystalline cracking or caustic embrittlement of boiler metal.
- Prevent carryover to ensure high-quality steam at an economical rate of continuous and intermittent blow-down.

The accomplishment of these objectives will involve several forms of treatment including the following:

- Pretreatment of the make-up water before it enters the boiler feed water system (filtering and softening using an ion-exchange resin).
- Chemical treatment of the boiler water internally in the boiler (addition of oxygen scavenger and scale inhibitor chemicals).
- Preheating of the water prior to steam generation.

It is probable that recirculating a portion of the treated water will gradually increase the total dissolved solids, if in situ degradation reactions produce substantial amounts of inorganic carbon. The extra solids will reduce the life of the water softening resin, which in turn will increase the amount of salt needed to regenerate the units.

4.1.7 Electrical Cogeneration Option

The use of off-the-shelf, low-technology steam turbines deserves attention. Since the steam directly leaving the steam generation chamber will be at a high pressure and a pressure reduction step is needed, using a turbine for this purpose can save fuel overall and reduce the power cost for electrical heating.

- The steam turbine to be used for this application is a basic single-wheel impulsetype turbine. These machines are rugged in design and will operate with little or no attendance. They are available as new machines with 6 to 8-month lead time on order; there is also an abundance of used equipment.
- Steam requirements for the turbine are flexible: inlet pressure ranging from 150 to 1,000 psig, with no super heat component required. Outlet pressure can range from 15 to 200 psig.
- Typically the available generators will be in a size range of 100 to 500 kW. A larger generator would be more complicated to install and operate. The generators can be connected together and will operate as a small "grid" with frequency control.
- The power produced is approximately 1,100 kW for a 50 MM BTU/hr steam generator, leading to the following maximum power generation rates for the three scenarios:
 - Scenario 1: 2,200 kW
 - Scenario 2: 3,300 kW
 - Scenario 3: 4,400 kW
- The cost to generate the power is typically assumed to be 4 percent of the energy that is passed through the turbine. However if there is currently pressure reduction required, this number could be improved to as little as 1.5 percent.

The cost estimate for the turbine generator sets based on a reconditioned turbine with a new generator and voltage control is \$200/kW.

We suggest that the power generated from the turbines be directed to the electrical heating arrays. It turns out that the potential power generated by the turbines closely matches the average power proposed for the ERH in each case (see Section 4.6.1).

4.2 ABOVEGROUND STEAM AND AIR CONVEYANCE

4.2.1 Steam Injection Pressures

The estimated injection pressures for the five main injection intervals are summarized in Table 4-1. More detail regarding the assumptions and calculations is provided in Section 3.10.

4.2.2 Steam Quality

Steam will be injected below the surface as approximately 75 percent saturated steam vapor and 25 percent condensed liquid. The water-tube-style steam generator is preferred for this application due to the larger combustion chamber. An oil-field-style steam generator with a water tube design is even better due to the extra large combustion chamber required to properly combust crude oil for the primary fuel. The steam quality of an oil-field-style steam generator is 80 percent or less. If a steam turbine is used, the steam quality ratio will remain unaffected. The use of a steam separator may be helpful for better turbine performance for the proposed cogeneration unit. If used the steam and condensate will be rejoined with the dry steam used for electrical power generation before final injection into the wells. Condensate traps will be installed in the steam lines in order to remove water and prevent water hammer during startup and system shutdown.

If the concentration of total dissolved solids (TDS) in the condensate is higher than that of the native groundwater, it can be injected at the electrodes, increasing the electrical conductivity in the vicinity of the electrode.

4.2.3 Air Injection

Steam and compressed air will be piped across the site from the treatment plants' air compressors in pipe mains, with several branches off to sections of the well-field. The main lines will be maintained at relatively high pressures, due to the need for no less than 107 psig for injection in the E-zone aquifer. It is anticipated that the header pressure will be between 125 and 200 psig.

The air injection rates are expected to be in the range of 1 percent by mass of the steam injection range, which equals a total of 240, 360, and 480 standard cubic feet per minute (scfm) for Scenarios 1, 2 and 3, respectively. This estimate is based on experience at the Visalia Pole Yard during full-scale remediation (Southern California Edison 2000). Each injection well will have a pressure reducer and a simple flow meter.

4.2.4 Piping Systems and Control

Steam and air will be distributed to injection wells using Grade A 53 black steel piping. In addition, valves in the pipe system will be manually controlled. Details of the design, including pressure regulators near the injection wells and control for valves, will be developed after the 10 percent conceptual design is completed.

The design of the piping layout should take into consideration ease of access to all wells for operation and maintenance. The piping layout will include a steam "main" from the boiler routed around to the remediation area. Steam supply to each injection well will be branched off from the steam main. This type of layout would not only minimize piping, it would also allow light transport access to all injection and extraction wells by placing ramps at specific locations to avoid aboveground pipes. Carbon steel pipe should be used for steam conveyance during the remediation. The carbon steel pipes will be wrapped with insulation to prevent unnecessary heat loss and provide protection for personnel.

ERH cables will be laid on the ground underneath the piping racks to keep as much of the site as possible open to routine vehicle traffic.

4.3 ELECTRICAL HEATING POWER SUPPLY AND DELIVERY TO ELECTRODES

The ERH system consists of the following components: a power control system, electrical distribution cables, and the electrodes. The electrodes were described previously in Section 3.2.

The ERH power control system (PCS) consists of the various components that reduce the supplied voltage to a level that is appropriate for passage through the subsurface. The PCS also includes all of the automatic safety shutdowns and monitoring equipment for the various electrical parameters. Due to the configuration of the heating zone at the McCormick and Baxter site, the use of three-phase electrical current is most appropriate.

ERH remediation of the D-E aquitard requires greater voltage and less amperage than that required for remediation under the slough. For this reason, one PCS would be required for Scenario 1 and an additional PCS would be required for Scenarios 2 or 3. The D-E aquitard PCS will have a capacity of 5,100 kW and be designed for an output voltage range from about 180 to 320 volts. The slough PCS will have a capacity of 2,300 kW and be designed for an output voltage range from about 120 to 220 volts.
It is premature to provide extensive details about the PCS in the 10 percent design. However, the major components of the PCS will include the following:

- An input power disconnect switch
- Input electrical usage metering
- A contractor to allow local, remote, and automatic shutdown of ERH power
- A step-down transformer to reduce the utility supply voltage (typically 13,800 volts) to an appropriate range for ERH use
- A motorized tap switch to allow local, remote, and automatic adjustment of ERH voltage in response to soil resistance changes
- Output instrumentation for voltage and current flow to the electrode field

For the McCormick and Baxter site, autotransformers would likely be used to provide alternate electrode voltages that are 20 percent above and below the standard voltage. The ERH operator would have the choice of connecting any particular electrode to the standard voltage, to a higher voltage if greater electrode power and current was desired, or to a lower voltage if lesser electrode power and current was desired.

The PCS will be centrally located in a simple pad-mounted building, about 600 to 900 ft².

Power to the individual electrodes will be distributed using type W (extra-hard usage) cable. These cables are suitable for routine foot traffic, periodic vehicle traffic, and wet locations.

Electrodes in the slough will be installed inside a pile-driven casing. A barge will carry the drill rig. The electrode cables and flexible extraction hoses from the electrode casing will be supported by a slack-removing pulley and borne to the shore on floats.

4.4 CONTAMINATED LIQUIDS AND VAPOR EXTRACTION AND CONVEYANCE

4.4.1 Wellhead Design and Down-Hole Pumps

Wellheads will be configured from standard 6-inch 150-pound raised-face flanged tees. Each of the injection wellheads will have the following:

• A steam injection port equipped with an on/off valve and a flow meter

- An air injection port with on/off valve and flow meter
- A pressure gauge and/or transducer
- A temperature gauge and/or transmitter
- A cooling port for cooling the well prior to opening it for service
- A pressure relief port

Each extraction well will have the following:

- A vacuum extraction port connected to the main vacuum line
- Connections and piping/tubing for down-hole pumps
- A pressure gauge and/or transducer
- A temperature gauge and/or transmitter
- A cooling port for cooling the well prior to opening it for service
- A pressure relief port
- Appropriate flow meters

Design drawings and more detail will be provided at a later design stage.

4.4.2 Liquid Conveyance

With the higher concentrations of contaminants and higher temperatures of extracted materials during the remediation, pipe material will be selected on the basis of temperature and chemical compatibility, as well as its resistance to corrosion due to the extracted groundwater and the environment. Steel pipe is recommended for use as the primary pipe material due to its cost as well as its ability to be used at the expected temperatures. The rate of corrosion of steel pipe will be evaluated during the remediation. Other piping material such as stainless steel and fiberglass can handle the higher temperatures and contaminants expected during the remediation. Results from related pilot tests such at the one performed at Wyckoff-Eagle Harbor (USACE 2000b) should be used to determine the appropriate lifetime of the selected material. Stainless steel and fiberglass are on the order of two to three times more expensive than comparably sized steel pipe. At this time, steel pipe has been chosen as the most appropriate pipe material for liquid conveyance.

4.4.3 Vapor Conveyance

Vapor extracted from the extraction wells and the vapor collection system under the vapor cap will be conveyed from field locations to the treatment plant. Construction materials for this conveyance system should be similar to those used for the liquid contaminant conveyance system, since the temperatures and constituents will be similar.

A final piping layout will be provided at a later design stage.

4.5 EFFLUENT TREATMENT SYSTEM

An on-site treatment plant is needed to process the contaminant-laden liquid and vapor extracted from the ground. For the conceptual design, it was assumed that effluent water would have to be treated to meet the compliance criteria discussed in Section 6.6, or the background levels, as appropriate, and that treated air would have to meet applicable air quality standards.

4.5.1 Effluent Streams and Composition

Estimates of contaminant concentrations in groundwater cover a large range due to temperature variations during thermal treatment. Although PAH solubilities can be expected to increase as much as three orders of magnitude during heating from ambient to steam temperatures, dissolved concentrations in the field will be limited by mixing with steam condensate and groundwater recharge.

Actual COC concentrations in the extracted water were not calculated because of the degree of associated uncertainty. Therefore, the treatment system was based on previous work (i.e., Visalia Pole Yard full-scale operation data [Southern California Edison 2000] and the Wyckoff-Eagle Harbor pilot test [USACE 2001b]). The Visalia COC concentrations are valuable especially because that site used a similar effluent treatment system; therefore, the waste streams are believed to have a similar composition. One example is the use of other sites to estimate the liquid-phase granular activated carbon (GAC) consumption (see Section 7.1).

4.5.2 Treatment System Components

The treatment system is designed to accommodate all phases of liquids and vapors extracted from the site. Robust proven technology with strong vendor support should be utilized. The primary components of the system are shown in Figure 4-1. Table 4-2 indicates the individual units and their sizes and capacity for each of the three scenarios.

4.5.3 Cooling and Condensation

Cooling Fluids

A shell and tube heat exchanger will be included in the treatment train to recover energy (heat) from the liquid and cool the effluent prior to discharge. If further cooling is needed, cool water from the cooling tower will be circulated on the outside jacket of the exchanger.

Condensing Vapors

The primary condenser for extracted vapors will be an air-to-air fin-fan-style heat exchanger sized to anticipate potential hot condensate loading due to steam breakthrough from multiple extractors at once. A separator vessel equipped with a demister screen and evacuation pump will follow the condenser. Condensed fluids will be directed to the liquid treatment system.

The estimated cooling needs during cool-down are indicated in Table 4-3.

4.5.4 Liquid-Vapor Separation

There will be a small amount of condensate collected in the vapor conveyance that will be captured before the fin fan. A liquid-vapor separator after the air exchanger will separate the bulk of the condensate. The liquid-vapor separators are vertical vessels with multilevel trays and demisting screens that will contact the gas to promote condensation within the vessel. Discrete level controllers will initiate and control the transfer of condensate to the liquid treatment system.

4.5.5 NAPL Removal From Water

Selection of the appropriate treatment processes for treatment of an oily waste is dependent on the oil classification. Under proper quiescent conditions, free oil can be removed by gravity separation. Emulsified oil cannot be removed by gravity separation unless it can first be converted to free oil by breaking the emulsion. Emulsified oil may be removed by air flotation, although the emulsion may first have to be broken for this process to be effective.

Destabilization

Treatment of oil emulsions is usually directed toward destabilizing the dispersed oil droplets, causing them to coalesce and form free oil. The process typically consists of rapidly mixing coagulant chemicals with the wastewater, followed by gentle mixing (flocculation). The agglomerated oil droplets may then be removed by gravity or flotation.

Chemical Processes

Alternative processes for breaking chemical emulsions include either the addition of acid (acid cracking), iron, or aluminum salts (coagulation), or the use of chemical emulsion breakers. In acid cracking, the pH is reduced to approximately 3 to 4, so the wastewater must be neutralized after oil-water separation. The use of iron or aluminum salts with or without polyelectrolytes may be less costly, but it produces additional solids from the chemical precipitates. Proprietary chemical emulsion breakers are very effective, but they are more costly than iron or aluminum salts.

Air Flotation

In the air flotation process, separation of both oil and solid particles is brought about by introducing fine air bubbles into the liquid waste stream. The bubbles attach to the particulate matter and oil droplets, and the buoyant force of the air bubbles causes both particles and small oil droplets to rise to the surface. The oil/solids/air bubble mixture forms a froth layer at the surface, which is skimmed away. The removal efficiency of air flotation separators for free oil is similar to that of gravity separators. However, air flotation units can also remove dispersed oil droplets and more readily accommodate heavier oil loading.

The recommended NAPL-water separation approach is the following:

- Allow long retention time in incoming water surge tanks to allow for phase separation where the NAPL is not present as an emulsion.
- Equip the large holding tanks with oil skimmer pumps so they function as oilwater separators.
- Run the water through induced air flotation units to remove NAPL and particles.
- Encourage emulsion breaking by chemical addition only if deemed necessary, with the necessary pH neutralization after the induced air flotation unit.

At this point, it is anticipated that emulsion breaking will not be necessary.

4.5.6 Dissolved-Phase Treatment Alternatives

The only alternative considered for removal of the organic COCs from the water is adsorption.

Adsorption is a natural process in which molecules of a liquid or gas are physically attracted to and held at the surface of a solid. Treating waste streams by adsorption involves transferring and concentrating contaminants (the adsorbate) from one medium (liquid) to another (the adsorbent). The most commonly used adsorbent is GAC. In liquid-phase carbon adsorption, the contaminated liquid comes in contact with the carbon by flowing through one or more packed-bed units. The activated carbon selectively adsorbs organic hazardous constituents that are attracted to and held in the internal micropores of the carbon granules.

Limitations:

• Cost is high if used as primary treatment of fluids with high contaminant concentrations.

- Spent GAC may require hazardous waste handling and transport before being disposed of.
- Influent streams with high concentrations of suspended solids (>50 mg/L) or oil and grease may cause GAC fouling.
- Metals can foul GAC systems.
- Carbon may require backwashing if suspended solids are present.

It is desirable to regenerate the GAC on site using steam. Details are provided in Section 7.1.

Biological treatment was not included for dissolved COC polishing. If the pilot test at Wyckoff-Eagle Harbor proves successful, biological treatment may be added to the treatment system at a later design stage.

4.5.7 Vapor Treatment Alternatives

Contaminated vapors that are collected from the extraction wells, the vapor cap, and the treatment plant unit operations as off-gas will require treatment prior to discharge to the atmosphere. The alternative treatment options are discussed below.

Oxidation

Vapor emission oxidation is a process in which oxygen and organics react under high temperatures to produce carbon dioxide, water vapor, and in some cases acidic gases (such as hydrochloric acid). Oxidation systems are relatively simple devices capable of achieving destruction efficiencies of 98 percent or greater.

Oxidation by Flares. Flares are typically used for waste streams that have large volumes and high organic concentrations. Basically, a large flame burns the contaminated vapors as they leave a stack or chimney. Since organic concentrations in remediation applications are relatively dilute, flaring is not an applicable technology.

Thermal Oxidation. In thermal oxidation, gas is heated to a sufficient temperature to oxidize organic compounds to carbon dioxide and water. This technology does not offer any advantages over vapor recycling and, therefore, will not be considered further.

Catalytic Oxidation. In catalytic oxidation, a catalyst is used to alter the oxidation/reduction rate causing it to occur faster and/or at a lower temperature. This technology does not offer any advantages over vapor recycling and, therefore, will not be considered further.

Gas-Phase Adsorption

Gas-phase adsorption is a natural process in which molecules of a gas are physically attracted to and held at the surface of a solid. Treating waste streams by adsorption involves transferring and concentrating contaminants (the adsorbate) from one medium (gas) to another (the adsorbent). The most commonly used adsorbent is GAC. In gas-phase carbon adsorption, the contaminated gas comes in contact with the carbon by passing through one or more GAC units, which are usually the fixed-bed type. The activated carbon selectively adsorbs organic molecules, which are held in the internal micropores of the carbon granules.

GAC units may be single- or multistage. Multistage systems make optimal use of carbon stock but increase operation and maintenance (O&M) costs. On-site regeneration using steam is recommended for the primary vapor-phase GAC vessels. A polishing vessel will consist of sacrificial GAC.

Vapor Recycling

Recycling the contaminated vapors in the boiler is a treatment alternative. In this common industrial practice, the vapor stream is blended with excess air and fed to the combustion chamber of the boiler, where it combines with heat and the boiler's fuel (natural gas) to generate more heat, which boils the feed water and makes steam. Proper recycling of the contaminated vapors would require a suitable combustion chamber temperature and retention time in the combustion chamber. It has been reported that the boilers at the Visalia site recycled the contaminated vapors with a retention time of 4.6 seconds. Further evaluation of the proper temperature and retention time requirements will be necessary to complete the design of this system.

Recommended Alternative

Vapor recycling is recommended as the primary vapor treatment alternative. Use of the steam generator for the destruction of vapors collected from the field would significantly decrease the cost and overall operational complexity of the project. The burden on the steam generator and its operator would be minimal. Once the treatment area has been heated, it would be necessary to extract and treat contaminated vapors most of the time to control potential fugitive emissions, regardless of whether the rest of the thermal remediation system is being operated. Thus, a backup treatment system would be necessary for times when the boiler is not operating (e.g., during maintenance and unexpected shutdowns) or not recycling vapors effectively. This backup system could become the primary vapor treatment system once steam generation is no longer needed. For these reasons, it is recommended that a gas-phase adsorption system be used as a secondary vapor treatment system for the remediation. On-site regeneration of the primary vessels using steam is recommended.

The capacities of the chosen units are provided in Table 4-2.

4.6 UTILITY REQUIREMENTS

The processes requiring power, water, and fuel include the following:

- Steam generators and water pretreatment units
- ERH power control units
- Powered valves and down-hole pumps
- Effluent treatment system

The utility requirements will vary considerably over the duration of field operations, depending on the phases. For instance, steam generation and power demand will be at their maximum during the first and second years, when the main focus will be on heating the major sand layers and the D-E aquitard. During subsequent years of thermal treatment, ERH will be discontinued, and the steam injection will be performed in a cyclic manner, where parts of the site will be receiving steam while other parts will be in a depressurization mode. Thus, the average fuel and water demand will be less than the maximum during the last half of thermal treatment.

The cooling capacity will vary considerably over time as well. Since the cooling will be accomplished using water circulated through large cooling towers, the associated utilities will be water (make-up for the water that is evaporated in the towers) and power (for the motors turning the fans that blow atmospheric air across the towers). During the first year of operation, it is expected that the cooling demand will be relatively low, since the site will be heating up during this period. At a later stage, when most of the site has been heated, steam will be extracted by allowing steam to flush through to the extraction wells, and the cooling need will increase as vapors are being condensed for treatment. However, even under the most demanding cooling requirements, the electricity required for cooling will be a small fraction of the total electricity used.

The predicted utility requirements are indicated in Table 4-4. These values were calculated from an overall mass and energy balance for the site and include several assumptions and estimates.

The following subsections describe the individual utilities.

4.6.1 **Power**

The total power demands are indicated in Table 4-4. The total service need ranges from 6.6 to 9.9 MW for the three scenarios.

On-site electrical cogeneration using steam turbines was discussed in Section 4.1.7. Since the steam directly leaving the steam generation chamber will be at a high pressure, and a pressure reduction step is needed, using a turbine for this purpose can reduce the power cost for electrical heating.

We suggest that the power generated from the turbines be directed to the ERH during the first 2 years of operation. It turns out that the potential power generated by the turbines fairly closely matches the average power proposed for ERH in each case (Table 4-4). During the third and fourth year, the cogeneration power will reduce the need for grid power.

Grid power will be used to ensure that power is available at all times and when the steam generators are inactive. Table 4-5 indicates the estimated grid power that will be needed and the anticipated cogeneration power that will be used during the first 4 years of operation.

4.6.2 Fuel

Boilers require a hydrocarbon fuel source to generate the high temperatures necessary to convert water to steam. The fuel source will most likely be natural gas. It is anticipated that the local utility company will provide the fuel. Natural gas will be delivered to the site using dedicated pressurized pipe. The actual source of the gas and the cost of installing the line will be defined at a later design stage.

Should fuel prices change such that diesel would be a cheaper overall fuel source, that would be a viable option as well. The final decision will be made based on current fuel prices at the time of the final design.

The potential use of recovered NAPL as a supplemental fuel source is discussed in Section 7.3. This option has not been included in the design at this stage, but should be considered carefully later.

4.6.3 Water

Water will be necessary for steam generation at the site. A sustained rate of between 160 and 320 gpm will be needed to make steam for up to 1 year, after which the need will decline as the steam injection rates are reduced (Table 4-4). The actual steam generation rate will vary depending on the remediation operational needs. It will also be necessary to have additional water available for boiler blow-down, wash-down, and water softener regeneration.

The fresh water supply will be city water. The total water needed for different phases of operation is indicated in Table 4-4. The principles used to generate the numbers are as follows:

- Water will be recirculated to the extent practical, thereby minimizing both the water supply and the treated water disposal rate.
- The fresh water supply will be regulated so the potential scaling problems discussed above may be kept tolerable, while the disposal of treated water will be minimized.

Other utilities such as telephone lines will be detailed at a later design stage.

4.7 WASTE GENERATION AND DISPOSAL

4.7.1 Solids From Subsurface

Spent filter cartridges and sludge from the separating tanks will be collected in roll-off bins. The contents will be characterized and sent to appropriate waste disposal facilities. For planning purposes, it was assumed these wastes will not be considered a listed waste.

4.7.2 Nonaqueous-Phase Liquids

The NAPL recovered by the vapor condensation and oil-water separator will be collected in a holding tank on site. As necessary, the wastes will be loaded onto a 5,000-gallon tanker truck and transported to an incinerator for destruction. For planning purposes, SteamTech has assumed that the Clean Harbors facility in Kimball, Nebraska, will be used. The possibility of recycling the NAPL product should be explored further. Also, the possibility of using the NAPL as fuel in the steam generators should be investigated.

4.7.3 Spent Activated Charcoal

Carbon (GAC) will be utilized (see Section 7.1). Liquid-phase carbon that has become fouled with heavier compounds and can no longer be steam regenerated will be slurried from the vessels and loaded onto lined shipping containers. The frequency of replacement was based on scaling from Visalia Pole Yard's full-scale thermal remediation project (Southern California Edison 2000). SteamTech has assumed that this carbon will be incinerated at the Clean Harbors facility in Kimball, Nebraska.

4.7.4 Personal Protective Equipment

Personal protective equipment (PPE) will be stored in drums and periodically disposed of at an appropriate facility, as required.

4.7.5 Other Solid Waste

SteamTech has assumed that any other miscellaneous hazardous solid waste would be disposed of at the Clean Harbors facility in Kimball, Nebraska. Miscellaneous nonhazardous waste will be disposed of at a local Resource Conservation and Recovery Act (RCRA) landfill.

4.8 SITE INFRASTRUCTURE AND LAYOUT

The entire site is currently surrounded with adequate fencing and secured with a 24-hour guard, which will remain throughout the scheduled 7-year remediation and beyond. Close to the site entrance, there is an existing office structure, which could be refurbished for administrative functions and conference space. A small vapor cap is proposed for the CPA area, and there is an existing cap over excavated soils in the center of the site (MPA). A likely location for the installation of the steam generation and treatment system is just west of the existing soil cap in the MPA. The portion of the system that handles contaminated liquids will be placed on a 1-foot-thick 100- by 200-foot concrete containment/decontamination pad and covered with a temporary structure. Holding tanks will remain outside the structure but on the containment pad. Steam generators, the cooling tower, the air-to-air heat exchangers, and the operations office trailers will be positioned close by but off the pad. Footings and other details concerning the containment/decontamination pad will be saved for a later design. Piping for conveyance of steam, liquids, and vapors will dominate the site and pose access problems if not carefully designed. The surface should be covered with gravel or decomposed granite to facilitate vehicle access.



	Α	В	С	D	Е
Depth to top of injection screen (ft)	25	60	125	155	240
Injection pressure (psig)	12.5	30	63	78	107

Table 4-1 Maximum Injection Pressures for Different Depth Zones

Component	Parameter	Unit	Scenario 1	Scenario 2	Scenario 3	Comment/assumption
Vapor phase liquid separator 1	Air flow rate	scfm air	4,733	7,100	9,467	Non-condensable gas plus 10% of injected steam as condensable vapor
	Liquid flow rate	gpm condensate	22	27	32	Assuming max 5% of liquid extracted with vapor stream
Liquid phase heat exchanger	Liquid flow rate	gpm water	435	535	635	Injected plus 235 gpm
	Cooling capacity	MM BTU/hr	22	27	32	Assumed 100 F cooling
Vapor phase heat exchanger/condensor	Cooling capacity	MM BTU/hr	12	17	22	Assumed 10% of injected energy as steam and energy in 200F condensate
Vapor phase liquid separator 2	Air flow rate	scfm air	1,000	1,500	2,000	
	Liquid flow rate	gpm condensate	23	34	45	Based on maximum vapor phase cooling capacity
Vacuum pumps	Air flow rate	scfm	1,000	1,500	2,000	
	Vacuum	psig	-10	-10	-10	Need to apply vacuum
Vapor treatment units	Air flow rate	scfm	1,000	1,500	2,000	
Water surge tank	Retention time	hours	1	1	1	Need settling and separation time for solids and NAPL
	Minimum capacity	gallons	26,100	32,100	38,100	
Dissolved Air Floatation	Water flow rate	gpm	435	535	635	Recovered fluid and condensate not to exceed maximum pumping rate
	NAPL flow rate	gpm	11	13	16	Assumed max NAPL ratio of 2.5% of total water flow
Multimedia filters	Water flow rate	gpm	475	575	700	DAF flow rates plus process water
Water treatment unit (GAC)	Water flow rate	gpm	475	575	700	DAF flow rates plus process water
NAPL holding tanks	Capacity	gallons	21,000	21,000	21,000	Tanks added and emptied for disposal as needed
	Minimum number	#	2	2	2	Empty backup tank on-site

Table 4-2 Effluent Treatment System—Components and Sizes

Cooling (MM BTU/Hour)								
Year	Scenario 1	Scenario 2	Scenario 3					
1	21	32	45					
2	14	19	28					
3	6	11	16					

Table 4-3Average Cooling Needed During Cool-Down Period

Note:

MM BTU/hr - million British thermal units per hour

Erech water weeks estimates		Cooperio 4	Seemerie 2	Cooperio 2	1	Commont	1		1	1	
Fresh water usage estimates	Veerd	Scenario 1	Scenario 2	Scenario 3		Comment					
Heatup	Year 1	200	300	400	gpm	100% fresh w	ater				
Continued thermal treatment	Year 2	133	200	267	gpm	67% fresh wa	ter				
Continued thermal treatment	Year 3	100	150	200	gpm	50% fresh wa	ter				
Continued thermal treatment	Year 4	100	150	200	gpm	50% fresh wa	ter				
Cool-down	Year 5	10	15	20	gpm	100% fresh w	ater				
Cool-down	Year 6	10	15	20	gpm	100% fresh w	ater				
Cool-down	Year 7	10	15	20	gpm	100% fresh w	100% fresh water				
Treatment system pumping capacity	Maximum	475	575	700	gpm						
Steam generation water demand	Maximum	200	300	400	gpm						
						General proc	General process power		Electrical hea	ating demand	
Power consumption estimates		Scenario 1	Scenario 2	Scenario 3		Scenario 1	Scenario 2	Scenario 3	Scenario 1	Scenario 2	Scenario 3
Heatup	Year 1	3,550	5,080	5,480	kW	1,000	1,400	1,800	2,550	3,680	3,680
Continued thermal treatment	Year 2	3,551	5,080	5,480	kW	1,000	1,400	1,800	2,551	3,680	3,680
Continued thermal treatment	Year 3	800	1,000	1,200	kW	800	1,000	1,200	0	0	0
Continued thermal treatment	Year 4	800	1,000	1,200	kW	800	1,000	1,200	0	0	0
Cool-down	Year 5	500	650	800	kW	500	650	800	0	0	0
Cool-down	Year 6	500	650	800	kW	500	650	800	0	0	0
Cool-down	Year 7	500	650	800	kW	500	650	800	0	0	0
Max power usage for equipment sizing	Maximum	6,600	9,400	9,900	kW	1,500	2,000	2,500	5,100	7,400	7,400
Natural gas consumption estimates		Scenario 1	Scenario 2	Scenario 3		Comment					
Heatup	Year 1	152,381	228,571	304,762	scfh	80% firing rate	e in average				
Continued thermal treatment	Year 2	133,333	200,000	266,667	scfh	70% firing rate	70% firing rate				
Continued thermal treatment	Year 3	114,286	171,429	228,571	scfh	60% firing rate					
Continued thermal treatment	Year 4	95,238	142,857	190,476	scfh	50% firing rate					
Cool-down	Year 5	0	0	0	scfh	No steam gen	eration				
Cool-down	Year 6	0	0	0	scfh	No steam gen	eration				
Cool-down	Year 7	0	0	0	scfh	No steam gen	eration				

Table 4-4 Utility Estimates for Different Phases of Operation for Each Scenario

Total power consumption		Scenario 1	Scenario 2	Scenario 3					
Heatup	Year 1	3550	5080	5480	kW				
Continued thermal treatment	Year 2	3551	5080	5480	kW				
Continued thermal treatment	Year 3	800	1000	1200	kW				
Continued thermal treatment	Year 4	800	1000	1200	kW				
Cool-down	Year 5	500	650	800	kW				
Cool-down	Year 6	500	650	800	kW				
Cool-down	Year 7	500	650	800	kW				
Co-generation power available ¹		Scenario 1	Scenario 2	Scenario 3					
Heatup	Year 1	2200	3300	4400	kW				
Continued thermal treatment	Year 2	1760	2640	3520	kW				
Continued thermal treatment	Year 3	1540	2310	3080	kW				
Continued thermal treatment	Year 4	1320	1980	2640	kW				
Cool-down	Year 5	0	0	0	kW				
Cool-down	Year 6	0	0	0	kW				
Cool-down	Year 7	0	0	0	kW				
Grid power comsumption ²		Scenario 1	Scenario 2	Scenario 3					
Heatup	Year 1	1350	1780	1080	kW				
Continued thermal treatment	Year 2	1791	2440	1960	kW				
Continued thermal treatment	Year 3	250	350	500	kW				
Continued thermal treatment	Year 4	250	350	500	kW				
Cool-down	Year 5	500	650	800	kW				
Cool-down	Year 6	500	650	800	kW				
Cool-down	Year 7	500	650	800	kW				
Recommended minimum service	project	2000	2500	2500	kW				
1) Co-generation power rates reduced with scaled down steam injection rates									
2) A minimum of 50% of the general process power is assumed to be provided by grid power									

Table 4-5Estimation of Consumption of Grid Power During Operation

5.0 PROCESS CONTROL, OPERATIONS, AND MAINTENANCE

5.1 **OPERATIONAL STRATEGY**

5.1.1 Overall Operational Goals

The goals for operating the thermal treatment system include the following:

- Heat target volume to steam temperature within the first 365 days of operation using steam injection and ERH in a manner that optimizes steam sweep from the outside in where possible. Sweep and apply the heat aiming at target pore volume sweeps with steam or as directed by the observed NAPL and dissolved-phase COC recoveries.
- At locations where NAPL has been found outside the target volume (or is suspected to be present), operate extraction wells screened over the intervals where NAPL is expected and where steam flow is predicted (both).
- Oxygenate the treated extracted water and re-inject it in locations where there is a benefit of increased biological activity.
- Maximize liquid-phase NAPL recovery by minimizing vaporization and alteration during the first phase of operation. This will optimize recovery of high-boiling-point components of the NAPL, including dioxins/furans and metals dissolved in the NAPL.
- Where liquid NAPL recovery rates diminish in a zone or area, encourage vaporization and degradation by allowing steam breakthrough to wells, induce pressure cycling, and inject oxygen to fuel degradation reactions.
- Operate the thermal treatment system for a period of up to 4 years with stimulation of in situ degradation reactions such as hydrous pyrolysis/oxidation and biodegradation, by injecting of air containing oxygen and by inducing pressure cycles that help mix the injected air with the heated water.
- After active heating has been discontinued, continue with a pump-and-treat system that re-injects treated water to encourage aerobic natural attenuation reactions. Consider modest injection of air into previous steam wells to supply oxygen (biosparging) during cool-down.

5.1.2 Phases of Operation

The operational phases consist of the following:

- Thermal treatment (total duration estimated at 4 years)
 - Heat-up (180 days to 1 year)
 - Continued heating and NAPL recovery with ERH of the aquitards (for as long as 1 year after heat-up)
 - Flushing and pressure cycling (up to 2 years after cessation of ERH)
- Cool-down with continued extraction (3 years)

The phases will overlap both in time and space, as one area heats up more rapidly than others, and one phase may be completed in a depth interval, but not in another.

5.1.3 Prevention of NAPL Spread

The main issues and challenges in preventing the spread of NAPL include the following:

- Prevent NAPL spread to greater depth by a general downward-up steam and power injection strategy, leading to steam rise and formation of hot floors under the mobile NAPL areas before they are heated to steam temperature.
- Prevent NAPL migration northward under Old Mormon Slough by:
 - Aggressive extraction along the boundary between the slough and the main site to its south, or
 - Heating from north of the NAPL (steam or ERH), combined with extraction from wells installed under the slough.
- Prevent NAPL migration southward off the property boundary in S-CPA and S-MPA by aggressive extraction from wells screened in the relevant zones.
- Prevent NAPL migration eastward from MPA into E-MPA in the D- and E-zones.
- Prevent NAPL migration through the mudline into the slough water. Such migration can be caused by diagonal upward migration of steam and condensate from treatment of the A- and B-zones close to the slough, or by the heating of

soils adjacent to well bores if wells or electrodes are installed in the slough without dewatering. Methods for preventing the NAPL migration are discussed in Section 3.7.

5.1.4 Injection of Air and Oxygenated Water

Considerations regarding air and water injection are as follows:

- Air injection will not be initiated during the initial heat-up phase because of the following factors:
 - Potential blocking of sand layers by air, leading to reduced steam injection rates.
 - Risk of COC-laden air escaping to surrounding zones.
 - Potential for the air injection to reduce the stability of the steam front, with the risk of reducing the efficacy of NAPL displacement.
 - Air injection encourages NAPL alteration by vaporization of most volatile COCs and degradation reactions, which could hurt the recovery of the less volatile COCs.
- Air injection will be initiated after peak NAPL recovery in each zone or area to minimize alteration of NAPL and optimize recovery of nonvolatile COCs. Low air-steam ratios will be used, so the air flow does not lead to increased pipe or pump sizes in the vapor recovery system.
- Co-injection with steam is favorable since it encourages mixing.
- Air injection during pressure cycling is favorable since it optimizes mixing.
- Measurement of the dissolved oxygen (DO) content of the extracted water may be used to adjust the rate of air injection. Water in wells where the dissolved oxygen level is less than 1 mg/L is considered anaerobic, and the injection rate of air in neighboring wells can be increased.

Continued aeration after termination of steam and power injection is favorable. During cool-off, when the extraction system is still being operated, the treated oxygenated water will be flushed through the zones where natural attenuation will be supported. Air injection should be evaluated and may be used to supply oxygen to the degradation reactions.

More detail will be provided at a later design stage.

5.2 STEAM AND AIR DELIVERY

Steam and compressed air will be distributed across the site from the steam generators and air compressors (via high-pressure pipelines). Branch lines will extent to different sections of the well-field. The main pipelines will be maintained at relatively high pressures because of the need for no less than 810 kPa (107 psig) for injection in the E-zone aquifer. It is anticipated that the header pressure will be between 125 and 175 psig.

Condensate traps will be installed in the steam lines to remove water and prevent water hammer during system start-up and shut-down.

5.2.1 Temperature and Pressure Regulation

The maximum injection pressure will be applied to the steam header line across the site. Each injection well will be equipped with a secondary pressure regulating valve and temperature and pressure gauges. The steam injection pressure will be controlled to an accuracy of 1 psi using air-powered pressure regulators. The secondary steam pressure (which is the pressure of the steam going down-hole) will be monitored continuously using pressure transducers with data output to a central location. Black iron pipe rated at least for 150 psig steam will be used to deliver the steam to the wellheads.

5.2.2 Injection Rate Control and Measurement

The total steam injection rate for the site will be monitored continuously at the steam generation plant, as the total water consumption minus blow-down. Steam injection rates will be monitored at several locations across the site using orifice plates and differential pressure measuring equipment.

The steam injection rate for each well will be measured using standard steam flow meters such as orifice plates or the equivalent, with automatic data collection and integration.

The injection rates will be scaled by the operators based on observed heating rates and the overall strategy for heating the site. Injection rates will be changed by changing the injection pressure at the steam pressure regulator at each wellhead.

5.2.3 Safety Measures

Each injection well will be equipped with a pressure relief valve set at a predetermined pressure, which will prevent the steam injection pressure from exceeding the maximum allowable pressure at any given location (see Section 3.3).

5.3 ERH PROCESS CONTROL, OPERATIONS, AND MAINTENANCE

5.3.1 ERH Process Control

The ERH system will consist of two independent sections:

- The D-E aquitard heating system used in all scenarios
- The B- and B-C zones heating system under Old Mormon Slough in Scenarios 2 and 3

Ohm's law governs the flow of electrical current through the subsurface in that the current flow is proportional to the applied electrode voltage and the power applied to the heated zone is proportional to the square of the applied electrode voltage. Thus, the voltage applied to the electrodes is varied by the power control system (PCS) in order to control electrical heating and the subsurface steam generation rate. In general, the most cost-effective remediation results from maintaining the highest sustainable ERH power within the capacity constraints of the PCS.

The electrical resistance of the subsurface varies considerably during ERH operation. It decreases as it is heated. A decrease to 30 to 50 percent of initial cold resistance is expected as the ERH zone is heated to boiling. When in situ steam generation begins, steam bubbles displace a portion of the groundwater, and the subsurface electrical resistance increases again, typically to within 70 to 90 percent of the initial cold resistance. The most rapid change in resistance occurs whenever the site is under boiling conditions and then the electrical power is shut down for a few hours. Almost all the steam bubbles rise out of the ERH zone due to buoyancy during the brief shutdown. Upon restart, the electrical resistance is at a minimum due to warm conditions with no steam bubbles; the resistance rises rapidly over the next hour or two as in situ steam bubbles are formed and reach equilibrium steaming conditions.

These changes in resistance require the ability to vary the sitewide electrode voltage with relative ease. The PCS can independently and rapidly vary the voltage of the three electrical phases through local operator action, through remote operator action, or automatically in order to maintain a desired voltage, a desired current, or any ERH desired power. The ERH system can be started and adjusted to any desired power in less than 1 minute. The ERH system can be shut down as simply as flipping a light switch; the subsurface voltage is instantly removed. Upon

shutdown, in situ steam generation stops instantly; however, the existing steam bubbles remain in the subsurface and continue to move upward due to buoyancy until they condense or reach an impermeable layer or until they are removed via an extraction well.

In addition to the automatic sitewide PCS control, local operators are able to adjust the operation of individual electrodes relative to the rest of the site. As presently envisioned, each electrical phase will be distributed at three levels: standard voltage, 20 percent high voltage, and 20 percent low voltage. If the operator wants to boost the relative voltage, current, and power of an individual electrode, it is simply disconnected from the standard voltage cable and connected to the 20 percent high voltage cable, an operation that requires about 10 minutes of labor. Similarly, connecting an electrode to the 20 percent low voltage cable will reduce its relative voltage, current, and power. The concentration of conductive ions varies across the McCormick and Baxter site, requiring the operator to determine the appropriate relative voltage for each electrode upon start-up. However, although sitewide resistance varies considerably during operation, the resistance of individual electrodes relative to each other is quite constant. Thus, the selection of the appropriate relative voltage for each electrode is pretty much a "set and forget" operation and we anticipate changing the relative voltage of only two or three electrodes per week following start-up. The preceding discussion has been simplified by treating each electrode as completely independent. In practice, each electrode interacts with all of those in its vicinity, and the relative adjustment of one affects all of its neighbors slightly.

The PCS computer continuously monitors the output voltage and current to the electrodes, sitewide ERH power, and utility demand. In addition to this sitewide monitoring, the ERH operator will verify the current draw of each electrode twice each week during routine operation. The data will be used to determine the need for changes in electrode voltages. The ERH power distribution over the site can be reported at any desired time interval.

5.3.2 ERH Operations

The ERH system is likely to be operated at close to its full capacity by adjusting the voltage as described in Section 5.3.1. However, the ERH system will be shut down under certain conditions:

- The ERH system will be shut down in the event of a significant failure of the vapor extraction, condensing, or vapor treatment systems. Although a short duration upset (less than 1 hour) does not pose any particular problems, an automatic timer will shut down the ERH system in the event of extended vapor extraction shutdowns to prevent ERH generated steam from moving off site.
- The ERH system will be shut down by a sitewide power failure.

- The ERH system will be shut down for several hours in order to complete each ERT shot. The frequency of ERT monitoring varies with the phase of operation (Section 6.1).
- The ERH system will be shut down briefly to allow changing the relative voltage of individual electrodes, as described in Section 5.3.1.
- The ERH system will be shut down for any subsurface maintenance (e.g., extraction pump change-out) to ensure that workers are not exposed to any hazardous voltages.
- The ERH system will be shut down for any major maintenance in the well field (e.g., piping manifold replacement) to reduce the risk to workers if heavy equipment damages an ERH cable. Hard usage cable will be installed to minimize the possibility of damage caused by these events.

In addition to these required shutdowns, the ERH system may also be shut down during periods of high electrical demand, such as hot summer afternoons. Such voluntary shutdowns are a mark of good citizenship; however, self interest also motivates these voluntary shutdowns because the local utility will provide electrical service at far lower rates to customers who modulate usage in this manner. Operating at higher power overnight can easily make up any loss of heating during voluntary and required shutdowns.

5.3.3 ERH Maintenance

Like all electrical components, the ERH system will require relatively little maintenance. The principle maintenance activities and approximate maintenance intervals will be as follows:

- Adjustment of the individual electrode relative voltages as described in Section 5.3.1 (weekly)
- Calibration of the PCS voltmeters and ammeters (monthly)
- Greasing of cooling fan motors (monthly)
- PCS cleaning and infrared inspection (at start-up and semiannually)
- Replacement of ERH distribution cables that become damaged during well-field maintenance (rarely or never)
- Replacement of failed PCS components (rarely or never).

The ERH system should readily last for the planned treatment period. Therefore no major component replacement is foreseen.

5.4 CONTAMINANT EXTRACTION

This section describes how the extraction of water, NAPL and vapors, will be controlled and monitored.

5.4.1 Controlling Liquid Levels in Wells

The levels in the pumping wells will be controlled directly by the placement of the extraction pump intakes and by the applied extraction rate. For pumps designed to optimize LNAPL recovery, the intake will be set at the desired level for water draw-down during extraction, which will optimize the NAPL-water ratio. For pumps designed to optimize DNAPL recovery, the intake will be placed at the well sump and the pumping rate will be adjusted to maintain the desired liquid level in the well.

Water levels will be measured in the wells using standard techniques (details will be provided at a later design stage).

5.4.2 Metering Extraction Rates and Fluid Properties

Liquid discharge rates from the down-hole extraction pumps will be measured using in-line flow meters of a type selected to work optimally with the chosen pump type.

Vapor extraction rates will not be measured for every wellhead due to the difficulty of measuring flow in a stream of air, contaminants, steam, and liquid droplets. Total flow rate per zone will be measured in several locations in order to ensure that vapor extraction rates in each depth zone are sufficient.

5.4.3 Hydraulic Control

The goals of hydraulic control are outlined in Section 3.9. The performance will be measured based on the following:

- Depth-specific water balances, from recorded steam injection rates (from steam flow meters) and liquid extraction rates (from flow meters)
- Overall site water balance based on the volume of water treated and re-injected

• Measurement of water levels and hydraulic gradients at multiple locations close to the property boundary

5.4.4 Pneumatic Control

Pneumatic control is described in Section 3.9.2. The goal of pneumatic control is the extraction of vapors in the form of noncondensable gas (air, not steam). Verification of pressure gradients in the vadose zone is not practical, since this would require burial of thousands of pressure transducers and near-realtime monitoring during thermal treatment. Therefore, pneumatic control strategies are intended not to document that no vapors escape at the site, but to ensure that the selected approach allows for a degree of certainty regarding vapor capture. The monitoring principles are follow:

- Measure the applied vacuum at each extraction well and verify that the desired vacuum is achieved at the well. At a minimum, this should be confirmed by daily reading of vacuum gauges attached to the wellheads.
- Calculate total air injection rates (sum of individual well air injection rates) and total extraction rate for noncondensable gas (measured downstream of the vacuum pump). By ensuring that more vapor is extracted than injected, the net loss of vapor is prevented.

A feature that is likely to assist in maintaining pneumatic control is the presence of a laterally extensive silt and clay layer in the upper 20 feet of the site, minimizing the vertical vapor permeability. Since a vacuum will be applied in the sandy zones below this clay/silt layer, it is anticipated that the radius of vacuum influence will be reasonably large and that contaminant-laden vapors migrating upward into the A-zone aquifer will be captured.

5.5 TREATMENT PLANT OPERATIONS

The effluent treatment plant will be operated continuously after the onset of initial cold water extraction until the end of the cool-down period, with only minor shutdowns for routine maintenance. All the control functions will be governed by a master programmable logic system monitored and adjusted by the plant operators.

5.5.1 Cooling Efficiency and Control

Cooling will be controlled by temperature regulators at each of the individual heat exchangers. Cooling towers are relatively inexpensive and their efficiency increases when operated at 50 to 75 percent of capacity. The controllers will monitor the process stream temperatures and regulate the flow of cool water to maintain the set point temperature.

5.5.2 Phase Separation Efficiency and Control

Phase separation efficiency will be measured by the purity of the phases extracted from the separator. Most separators are controlled by level or interface probes. Separated components are pumped to holding areas for disposal or reuse, as appropriate. The control functions, levels, and flow rates for this system will be governed by a master programmable logic system monitored and adjusted by the plant operators.

5.5.3 Water Treatment Efficiency and Control

The process water will pass through two separation phases. Contaminants and dissolved solids will be separated along the way. Efficiency will be determined through analytical checks. Sampling and analysis of total petroleum hydrocarbons (TPH) and COCs will be used to determine the efficiency of the components.

5.5.4 Vapor Treatment Efficiency and Control

Vapors will be separated from moisture before being combusted in the steam generators or bypassing through GAC units. An on-line flame ionization detector (FID) sensor will track hydrocarbons at the inlet to either unit and at the outlet of the vapor-phase carbon to determine the efficiency and monitor the clean air released to the atmosphere. When vapors are combusted in the steam generator, the efficiency will be determined by stack testing for COCs. Liquids desorbed from the carbon system will be metered as they are transferred to the NAPL holding tank. The control functions for this system will be governed by a master programmable logic system monitored and adjusted by the plant operators.

5.5.5 Adding or Removing Treatment Units During Operation

A modular design of the treatment system will include multiplexed systems in most cases. For example two 1,000-scfm vacuum pumps may be used to pull a 2,000-scfm load (Scenario 3). All piping and utilities will be designed to accommodate an addition or subtraction of components and systems during the remedial effort. This methodology also provides backup during routine maintenance without full interruption of the process. Off-line systems could be demobilized or salvaged as the remediation nears completion. In the same manner, additional systems could be added if the remediation requires more.

The need for heavy metals treatment could potentially arise during operation, if the concentrations of heavy metals in the effluent substantially exceed the E-zone levels (see Appendix A). The treatment system should be designed in a way that allows ready addition of such a unit, should it become necessary.

5.6 **BASIC MAINTENANCE REQUIREMENTS**

Since the duration of the remediation is planned for a total of 7 years, there could be major maintenance issues for most equipment. It is good practice that a record-keeping system be developed to help manage information and activities related to the following:

- Prevent maintenance schedule and completion
- Repair and maintenance performed
- Master equipment list
- Repair parts list
- Equipment vendor data index
- Operation records

The following equipment will likely require maintenance and repair:

- Boilers
- Pumps (potentially require significant maintenance)
- Compressors
- Piping
- Valves and meters
- Injection and extraction wells (potentially require significant maintenance)

Routine maintenance for pumps consists of lubrication and cleaning every 6 months and checking the oil level every month. Pump seals should be periodically checked, and pumps should be inspected for leaks, cracks, wear, or damage in the piping, especially from the extraction well to the treatment plant. Additionally, valves and flow meters may need to be repaired or replaced.

5.7 DURATION OF THERMAL TREATMENT

This section presents the assumptions made for setting the duration of the thermal treatment and briefly discusses the operational adjustments made in order to optimize the COC removal rates and minimize the use of time and resources.

5.7.1 Criteria for Discontinuing Steam and Power Injection in Areas/Zones

All three scenarios assume a total duration of 4 years for thermal treatment, as defined in Section 5.1.2. However, thermal treatment may be discontinued in certain depth zones or sections of the site if the operational monitoring shows that continued operation will lead to diminishing returns for the invested time and effort.

Guidelines for discontinued steam injection and ERH in areas include the following:

- When the water extracted from the E-zone wells has no more NAPL or sheen.
- No more NAPL is recovered during aggressive extraction in the D-zone and the interval between D and E.
- Practical constraints indicate that recovery of more NAPL from the A-, B-, and C-zones and the silt layers between them is no longer worth the time and resources (diminishing returns).
- The cumulative data indicate that mobile NAPL will not be able to migrate downward after heating is discontinued. (Specific criteria will be established to determine NAPL mobility).
- The whole target volume has been raised to a specific target temperature for a defined period of time.

5.7.2 NAPL Removal Rates

The total NAPL removal rate will be determined on the basis of daily measurements of the NAPL levels in the holding tanks used for on-site storage of recovered product. If the total NAPL removal rate diminishes for the site, the focus will be on the distribution of dissolved COCs across the extraction well-field.

In addition to monitoring the total NAPL recovery rate, the NAPL recovery from individual well clusters and separate wells will be monitored manually by visual observation of the NAPL fraction in the discharge lines from dedicated pumps. This will be performed either by installation of fraction collectors at the desired locations (for an accurate NAPL fraction estimate), or alternatively by visual observation through transparent TeflonTM tubing (for absence or presence of NAPL).

If the NAPL recovery rates from wells in the A-, B-, or C-zones diminish, the thermal treatment may be scaled back in order to save resources. However, several pressure cycles will be conducted in those zones before the decision is made to scale back.

If NAPL recovery rates diminish in the D- and E-zone extraction wells, operations will be continued with the main focus of removing leftover dissolved and adsorbed COCs by in situ destruction and vaporization during pressure cycling. The monitoring then will focus more on the dissolved COC levels in the extracted water

5.7.3 Dissolved Total Organic Carbon and COC Removal Rates

Diminishing NAPL recovery is expected for the D- and E-zones during thermal treatment, since these zones have much less mass of contamination, and will be flushed with more steam pore volumes than the upper zones. When small amounts of NAPL are recovered from certain wells, the dissolved concentration of total organic carbon (TOC) becomes the next relevant parameter. TOC can be measured inexpensively on site. Periodic screening of the well-field for TOC contents can provide a detailed picture of where the remaining extracted mass is coming from, and it will allow the operators to focus thermal treatment in those areas and depth intervals.

The concentrations of individual COCs indicate how far the steam distillation and destruction reactions have proceeded near the well being sampled. For discontinuing thermal treatment in the E-zone, where the targets for groundwater are MCLs (for organic COCs) or background (for inorganic COCs), dissolved COC concentrations will be monitored in point of compliance wells.

5.7.4 In Situ Destruction Rate Evaluation

After most of the NAPL has been removed from the site and the COC concentrations are approaching the cleanup levels, an evaluation of the in situ destruction rates may provide a basis for discontinuing operation prior to the achievement of all the groundwater goals. Such an evaluation will be based on the following:

- Measurement of recovered carbon dioxide in the extracted vapors and comparison to atmospheric levels as well as levels obtained during the proceeding operational phases.
- If practical, measurement of the isotopic composition of the extracted inorganic carbon, allowing for an estimate of the fraction of carbon derived from the oxidation of creosote-related organic carbon, which has a different isotopic signature than naturally occurring organics such as humic and fulvic acids.

If it can be established that the in situ destruction rates are sufficient to degrade the leftover COCs within the cool-down period, active thermal treatment can be discontinued.

5.7.5 Redox Level Measurements and Air Injection Strategy

An important prerequisite for relying on aerobic in situ destruction reactions is that sufficient oxygen is present in the groundwater at the appropriate locations. Oxygen will be injected along with the steam into an initially anaerobic aquifer. Anaerobic conditions were observed as low redox levels, absence of oxygen and nitrate, and the presence of reduced species such as ferrous iron, ammonia, and methane in groundwater (USACE 2001a).

The maximum solubility of oxygen in water at different depths below the water table and at different temperatures is shown in Figure 5-1. At depth, dissolved oxygen concentrations of 10 mg/L or higher can be accomplished, which will encourage hydrous pyrolysis oxidation reactions at depth.

Measurement of dissolved oxygen in the extracted water will be used to evaluate the optimal air injection rates. If possible, the air injection rate will be adjusted to allow for more than 1 mg/L dissolved oxygen in the extracted water.

5.7.6 General Considerations Regarding Steam Pore Volumes

The selection of the necessary number of pore volumes of steam and the needed amount of electrical energy input were based on several sources of information.

Laboratory treatability studies were inconclusive with respect to the number of pore volumes needed to remove the mobile NAPL and to reduce the water concentrations to MCLs. Two pore volumes were sufficient to remove the mobile NAPL. However, this does not scale up directly in the field due to flow regime issues.

Field experience from Visalia (Southern California Edison 2000) indicates the following:

- Mobile NAPL can be removed with an average injection of less than two pore volumes of steam (the Visalia site was more permeable, so it cannot be directly scaled).
- The MCL for PCP can be approached in the best wells with an average steam injection of less than two pore volumes.

The following assumptions were made:

- Mobile NAPL layers will receive at least two pore volumes of steam or an equivalent amount of energy will be injected using ERH.
- Silt and clay layers will be heated to steam temperature, and an additional 100 percent of the energy needed to heat them will be injected to encourage steam production and flushing of mobile NAPL out of these layers. This affects the upper 85 percent of the D-E aquitard in all scenarios and the B- and B-C-zones under the slough in Scenarios 2 and 3.

- The D-zone aquifer will receive at least three pore volumes of steam to ensure the removal of mobile NAPL and continued steam stripping and degradation reactions.
- The E-zone aquifer will receive at least four pore volumes of steam to ensure the removal of mobile NAPL and continued steam stripping and degradation reactions.
- Steam that sweeps upward through aquitard layers will be encouraged, thereby increasing the efficiency of the steam for NAPL removal.

5.7.7 Cool-Down Options and Stimulation of In Situ Destruction Reactions

After approximately 4 years of thermal treatment, a cool-down period is designed for all three scenarios. This period consists of continued pumping and treatment of water from the site, with the following main purposes:

- Preventing the escape of steam and COC-laden vapors from the site to the atmosphere, surface water, or groundwater
- Preventing the heating of Old Mormon Slough by upward migration of hot water and by thermal conduction from nearby heated soils
- Preventing the migration of hot groundwater off site

The main cooling mechanism will be the extraction of hot water from the aquifer zones and the replacement of the water by cool water from the outside. The extracted water will be cooled, treated, and re-injected to assist in cooling at desired locations.

During cool-down, degradation reactions such as hydrous pyrolysis and biological oxidation can play a major role in reducing the groundwater concentrations at the site. These processes can be encouraged by the following:

- Aeration of all re-injected water during the treatment so that the dissolved oxygen levels are in the 5 to 50 mg/L range (depth and pressure dependent)
- Injection of air into zones where supplemental oxygen is needed for degradation of leftover COCs

Other potential enhancements include ozone injection, addition of nutrients to the re-injection water, and the use of pressure vessels to aerate the water, allowing for a dissolved oxygen level higher than 10 mg/L for re-injection. If thermal treatment is selected for source removal,

additional methods of enhancing natural degradation processes would still be required to achieve final groundwater cleanup standards at the site.

The duration of the cool-down period for each scenario was calculated using a simple heat removal simulation. It was assumed that the effluent treatment system size would be kept the same as that for thermal treatment and that the extracted water mass would constantly be replaced by inward flow of cool water (from adjacent aquifers or from re-injection wells). A target extracted water temperature of 40°C was chosen. This means that the temperatures at the site boundaries will be well below this temperature, probably much closer to ambient temperatures. As such, the risk of adverse thermal impacts at the cessation of pumping would be minimal.

In Scenarios 1 and 2, some of the extraction wells will be located in unheated regions outside the target volume for thermal treatment, and their pumping will remove essentially no heat. It was assumed that the portion of the pumping occurring in the heated zone is in proportion to the fraction of the total heated region.

The water from the wells will be slightly cooler than the average temperature of the site because the water pulled from the wells will be influenced by the most permeable pathways from outside the site. These permeable pathways will cool more rapidly and we will have to rely on an element of thermal conduction to move heat into the water from the less permeable zones.

The simulated cool-down is shown in Figure 5-2. Resulting cooling times are shown in Table 5-1.

5.7.8 Estimates of Total Operation Time

Figure 5-2 indicates that the desired cooling can be accomplished in a period of 2.5 to 3 years. The total estimated operation time (thermal treatments and cool-down) is to about 7 years for all three scenarios.





Oxygen Solubility in Water as a Function of Depth Below the Water Table and Temperature

McCormick and Baxter Superfund Site Stockton, California



Thermal Treatment Zone Cooldown After Remediation

	Pump-and-treat	Scenario 1	Scenario 2	Scenario 3	Unit
Volume in Thermal Treatment (TT) zone	2,952,365	1,541,854	2,103,242	2,952,365	yd ³
% of total treated thermally	0	52	71	100	%
Heated volume ¹	NA	1,927,318	2,418,728	3,247,601	yd ³
Total amount of energy stored	NA	442	674	960	10 ⁹ BTU
Total water extraction rate	NA	475	575	700	gpm
Cooling water extraction rate ²	NA	282	428	700	gpm
Thermal treatment duration	0	4.0	4.0	4.0	vears
Cool-down to 40 °C	0	2.9	2.9	2.5	years
Total duration of activities, 40 °C final	>100	6.9	6.9	6.5	years
1) Heated volume is corrected for steam a	Ind heat losses to the	surrounding formation) Dn		
2) Assuming that not all water can be pum	ped from the heated	areas during cool-do	wn (scaled by volum	e fraction of full s	ite)

Table 5-1 Treatment Time Estimates, Including Cool-Down Period
6.0 PERFORMANCE AND COMPLIANCE MONITORING

6.1 TEMPERATURE AND STEAM DISTRIBUTION MONITORING

This section provides a suggested data collection schedule for acceptable coverage of subsurface monitoring over time.

A complete set of ERT data will be collected, processed, and interpreted, prior to the injection of steam. These data would be collected no sooner than 2 weeks after the last VEA hole has been grouted, to allow time for the grout to dry completely and eliminate any radical resistivity fluctuations that occur during the drying process. This would serve as a background measurement set against which to compare all data collected after thermal treatment begins. A background data set would be collected twice, several days apart. The purpose of the two data sets would be to determine the expected sensitivity of the measurement equipment. In other words, any changes in the resistivity between the two data sets that should otherwise be identical would be indicative of measurement error effects, like noise. Then, when data are collected during a process to observe, it would be understood which resistivity changes are true anomalies and which are just noise effects.

Table 6-1 provides an overview of the estimated data collection schedule during thermal operation, cool-down, and long-term monitoring. This schedule may be refined at a later design stage.

6.2 SUBSURFACE CONTAMINANT REMOVAL RATE MONITORING

It is not absolutely crucial, nor is it possible, to document the mass of contaminants removed or destroyed during full-scale thermal remediation as a means to evaluate remedy effectiveness. The initial NAPL volume at the site was estimated to be in the range of 1 million gallons, but with a high degree of uncertainty. Therefore, it is not possible to determine the amount of NAPL left or the degree of cleanup based on the removed mass. For the contaminant removal rate monitoring design, it has been assumed that the following data objectives apply:

- The cumulative volume of recovered NAPL will be documented for waste disposal and cost tracking.
- The amounts of individual COCs removed in the vapor phase will not be determined accurately, rather they will be estimated based on concentrations detected in grab samples and rough estimates of the amounts of COCs recovered from the GAC during steam regeneration of the vapor-phase GAC vessels.

- The amounts of individual COCs removed in the dissolved phase will not be determined accurately, rather they will be estimated based on concentrations detected in grab samples, and rough estimates of the amounts of COCs recovered from the GAC during steam regeneration of the liquid-phase GAC vessels.
- The amounts of individual COCs in the disposed GAC will not need to be quantified, since the amounts will be small and the data will not be needed.
- The amounts of individual COCs degraded in situ will not be quantified. However, the geochemical and thermodynamic conditions will be optimized to allow for in situ destruction (see Section 5.1), and the water quality monitoring will be used to adjust the operational strategy in order to optimize the in situ destruction.

Since a total COC mass balance is not crucial, the sampling and analysis performed for the effluent treatment system and at the wellhead level will be restricted to the minimum necessary to document the treatment efficiency and to troubleshoot individual system components. The monitoring program will include the following:

- Weekly gauging of the amount of NAPL recovered in the holding tanks (to support the flow rate data, which will be collected automatically).
- Monthly sampling of the major liquid and vapor streams for the major COC levels (semivolatile organic compounds [SVOCs] and metals).
- In-line measurements of bulk parameters in separate waste streams using automated equipment: FID and infrared sensors on dry vapors for estimation of carbon mass and carbon dioxide levels, and TOC analyzer sampling and analysis of key water streams for bulk organic carbon.
- Miscellaneous automated flow rate and cumulative flow measurements on NAPL, water, and vapor streams. The data are collected mainly to ensure that the treatment system is functioning correctly and to optimize individual components. The NAPL flows from holding tanks, dissolved air floatation units, and condensate streams from regenerating carbon will be quantified and used to document the efficiency of each component.

Overall, the flow rate monitoring, grab sampling for COC concentrations, and the measurement of bulk vapor and water parameters will be used to estimate the removed mass and, more importantly, to optimize the treatment system efficiency. Finally, the analysis of the phase distribution of the recovered mass (vapor, dissolved, and NAPL) will be used to optimize the operational strategy.

6.3 REMEDY EFFECTIVENESS MONITORING

A key component of the thermal treatment is the monitoring conducted to document the remedy effectiveness. For a complex site with multiple injection and extraction zones, the remedy effectiveness can be evaluated by the following data:

- Recovery from the combined well-field (treatment system level)
 - NAPL recovery rate for the combined well-field operated under ideal conditions
 - Bulk TOC recovery rates in the extracted water (most relevant when NAPL removal diminishes)
 - Bulk TOC recovery rates in the vapors extracted
 - COC concentrations in the combined effluent streams of liquids and vapor
- Recovery from individual depth zones and areas
 - NAPL recovery
 - TOC in water and vapor
 - Individual COC levels in water and vapor
- Individual well performance
 - NAPL production
 - TOC in water and vapor
- Individual COC levels in water and vapor
 - Monitoring parameters from surrounding wells
 - Groundwater COC concentrations
- Interim drill-back activities
 - Soil TPH analysis to estimate leaching potential

- Soil COC concentrations
- Final drill-back and site characterization
 - LIF survey
 - NAPL mobility leach tests
 - Soil COC concentrations
 - Groundwater COC concentrations

Recovery data will be collected during heat-up, continued thermal treatment, and cool-down. The data will be collected as an integral part of the overall sampling and monitoring program.

The interim drill-back activities may be used to determine if individual zones have reached the desired cleanup levels and to identify areas that need additional treatment. One drill-back per year is recommended during thermal treatment (three total). Each activity will involve an estimated 10 boreholes drilled to the target treatment depth at each location, with an estimated average drilling depth of 150 feet per borehole.

The final site characterization will be conducted after cool-down is complete. It will consist of drill-back (approximately 10 boreholes) and groundwater sampling and analysis. The final scope and extent of this effort will be designed at a later stage.

6.4 BOILER AIR EMISSIONS MONITORING

The objective of monitoring boiler emissions is to demonstrate substantive compliance with local, state, and federal regulations and demonstrate that the thermal remediation system would not affect the ambient air quality in the surrounding community.

The Clean Air Act (CAA), which was last amended in 1990, requires the EPA to set National Ambient Air Quality Standards for pollutants considered harmful to public health and the environment. The CAA established primary and secondary air quality standards. Primary standards set limits to protect public health, including the health of "sensitive" populations such as children, the elderly, and individuals with asthma. Secondary standards set limits to protect public welfare, including protection against decreased visibility, and damage to animals, crops, vegetation, and buildings. National Ambient Air Quality Standards for six principal pollutants, which are called "criteria" pollutants, are provided in Table 6-2.

In general, emissions from manufacturing operations are regulated under the CAA. Similarly, emission-control devices or units themselves (e.g., gas-phase activated carbon units) are also subject to CAA control. Therefore, boiler emissions are regulated under the CAA and are

subject to compliance with national ambient air quality requirements. It is anticipated that the following criteria pollutants may be analyzed: carbon monoxide (CO), nitrogen dioxide (NO₂), sulfur dioxide (SO₂) or sulfur oxides (SO_x), and particulate mater (PM).

RCRA requirements for boiler emissions are applicable or relevant and appropriate requirements (ARARs). Thermal treatment after a material (solid or liquid) becomes a hazardous waste (i.e., heating it to a gaseous state) is fully regulated under RCRA (54 FR 50973; December 11, 1989). Emissions from boilers and industrial furnaces that burn hazardous waste are regulated under 40 CFR, Part 266, Subpart H. Emission screening limits have been set for various metals (including arsenic and chromium, which are COCs at the site), chlorine gas (Cl₂), hydrogen chloride (HCl), and various volatile organic compounds (VOCs) and SVOCs.

Although it is possible to estimate the composition and volume of noncondensable vapors entering the boiler, it is difficult to estimate the quantity or variability of the constituents over time. Therefore, it is not possible to conclusively predetermine the type or amount of emissions generated by the treatment of the waste stream. Therefore, initial testing of the boiler for toxic air pollutant (TAP) emissions as provided in Table 6-3 would be conducted to determine the type and quantity of emissions produced by the treatment system. EPA guidance allows for sampling to be conducted on one representative unit if the boilers are similar (e.g., same size, operation, influent streams, etc.), and, therefore, emissions would be tested from only one of the four boilers.

Based on the chemicals of potential concern at the site and regulatory requirements, it is anticipated that the boiler exhaust stream would be sampled for dioxins/furans, PAHs, SVOCs, VOCs, HCl, Cl₂, particulate matter (PM), and metals. The boiler would be tested under a single operating condition, and three exhaust gas samples would be collected for each parameter. In addition, a field blank would be collected for each sampling train, and three field blanks would be collected for VOCs (i.e., one pair of resin traps would be collected during each day of sampling). The dioxin/furan, PAH, and SVOC samples would be collected using a Modified Method 5 sampling train. Dioxins/furans would be collected on a separate sampling train according to EPA Method 0023, whereas PAHs and SVOCs would be collected simultaneously on a single sampling train according to EPA Method 0010 procedures. Both sampling trains would be run for approximately 3 hours to collect the appropriate sample volume. The Volatile Organic Sampling Train (VOST) would be used to collect gas samples for VOCs. Six pairs of resin traps would be collected over each sampling run at various sampling volumes; three pairs of traps would be analyzed for each run. One of these three pairs would be analyzed separately to assess potential breakthrough of target analytes. Total hydrocarbon concentrations would be measured using a continuous emission monitor according to EPA Method 25. EPA Method 0050 would be used to collect HCl/Cl₂ and PM samples over an approximate 2-hour period. Metals would be collected over a 1-hour sampling period according to EPA Method 0060

procedures. The boiler exhaust gas would also be sampled for total hydrocarbons (THCs), CO, SO_x , and nitrogen oxides (NO_x) using a continuous emission monitoring system (CEMS).

It has been assumed that at least three sampling ports would be available to conduct the boiler emissions testing. Based on this assumption, it would take approximately 12 hours of steady state operation of the system to collect one run of the required samples. Therefore, boiler exhaust samples would be collected over a 3-day period. One day would be required to set up all equipment and the mobile laboratory and 1 day would be needed for demobilization (i.e., it would take a total of 5 days on site to conduct the boiler testing). It is anticipated that after the initial sampling, the analyte list could be reduced to opacity, CO, SO_x, and NO_x using CEMS.

Emissions testing for the gas-phase activated carbon unit during the steam phase of treatment would likely not be required since it is configured as a bypass (i.e., backup) unit.

Air monitoring of the gas-phase activated carbon is anticipated during the cool down phase. The boilers, which are the system's primary vapor treatment units, are not operating during cooldown.

6.5 SITE FUGITIVE EMISSIONS MONITORING

The objectives of fugitive emission monitoring are (1) to ensure that organic vapors emitted by operations of the thermal remediation system would not affect the surrounding community or on-site personnel, and (2) to demonstrate substantive compliance with local, state, and federal clean air regulations. Conceptually, there are three potential sources of fugitive air emissions: the conveyance system from the wellheads to the treatment plant, the treatment plant, and the soil area surrounding the treatment areas. Since the conveyance system would operate under negative pressure, it is unlikely that the pipe runs from the wellheads to the treatment plant would be a source of organic vapors. COCs include PAHs and PCP.

Potentially the largest source of fugitive emissions during the steam-phase treatment operations is the contaminated soils surrounding the treatment areas. In addition, a substantial volume of NAPL exists in the subsurface (USACE 2000a, 2000b, 2000c). Steam injection in the treatment areas may result in conductive heating of subsurface soils surrounding these areas. Consequently, volatilization of organic vapors may occur outside the perimeter of the planned vapor cap and vapor collection. Fugitive emissions can also occur at wells, pumping systems, and other portions of the treatment plant. Similar to the treatment plant, COCs that may be emitted from heated soils surrounding the treatment areas are PAHs and PCP.

Fugitive emissions would be assessed at two levels. Site worker exposure would be monitored with personal air monitoring. Substantive compliance with local, state, and federal clean air

regulations would be assessed with the use of canister air samples collected at the perimeter of the treatment area. The compliance monitoring program for the steam phase (active phase) and cool-down phase of treatment are provided in Table 6-3.

For the steam phase, up to three monitoring stations would be established around the perimeter of the treatment area. The monitoring stations would depend on seasonal changes that may effect the predominant wind direction. At least one station would serve as a background station. Initial sampling frequency would be determined after consultation with the local air quality agency. However it is anticipated that PAHs, PCP, and PM would be analyzed monthly, and CO, SO_x, and NO_x would be analyzed by CEMS. If no regulatory action levels are exceeded, monitoring may be decreased. Monitoring may be re-initiated for each 25°C increase in perimeter soil temperature.

During the cool-down phase, it is anticipated that sampling would be reduced to PAHs, PCP, and PM on a quarterly basis at three monitoring stations.

6.6 TREATMENT PLANT DISCHARGE MONITORING

Effluent from the treatment plant would be re-injected into groundwater at the site. The COCs that would be monitored include TPH, PAHs, PCP, metals (arsenic, chromium, copper, and zinc), and dioxins/furans. Effluent from the treatment system would be sampled and analyzed on a monthly basis, except for dioxins/furans, which would be sampled and analyzed on a quarterly basis. Sampling would likely be required at a higher frequency during the initial treatment phase; however, the frequency may be reduced later in the steam phase and cool-down phase of the treatment program. Cleanup levels (i.e., effluent standards for groundwater re-injection) would likely be set based on background levels in the E-zone aquifer.

A summary of compliance criteria for groundwater re-injection is provided in Table 6-4. The criteria are compared to the quantitation limits and the background contaminant levels. The criteria selected and included in this table are based on the following beneficial uses as presented in the document *The Water Quality Control Plan (Basin Plan) for the California Regional Water Quality Control Board, Central Valley Region, The Sacramento River Basin and the San Joaquin River Basin* (California Regional Water Quality Control Board, Central Valley Region 1998):

- Municipal and domestic water
- Agricultural supply
- Industrial service supply
- Industrial process supply

6.7 WASTE DISPOSAL CHARACTERIZATION

Waste material generated from the thermal treatment at the site would include both liquids and solids. NAPL recovered from the thermal treatment process would require disposal. Additionally, solids collected from the extraction wells during treatment, spent activated carbon (SAC), sludge, and spent filter media would also require disposal. Sampling would be conducted to verify compliance with all applicable state and federal hazardous waste regulations. The reasons for selective sampling are described in the following subsections. The wastes generated from the process would not be handled as RCRA-listed hazardous waste; however, they may be handled as a RCRA-characteristic waste for off-site disposal.

6.7.1 NAPL Disposal Characterization

NAPL requiring disposal would include all recovered product from the treatment plant and extraction at the wells. NAPL would be recovered by the on-site recovery system (i.e., thermal treatment train). The NAPL would likely require off-site disposal at a RCRA-permitted incinerator or permitted boiler. Characterization of the NAPL would include TPH, PAHs, PCP, metals (arsenic, chromium, copper, and zinc), and dioxins/furans. It is anticipated that characterization would be required quarterly during the steam phase of treatment and annually during the cool-down phase.

6.7.2 Solid Waste Characterization

Solid wastes generated during the treatment process would include solids that accumulate in extraction wells during treatment; sludge bottoms from treatment system components (e.g., oil-water separator units); SAC from the GAC unit, which consists of waste material that can no longer be thermally regenerated; and spent filter media from the sand filters. These materials would need to be characterized using the toxicity characteristics leaching procedure (TCLP) to determine disposal requirements. COCs at the site that have established regulatory limits on the TCLP extract for the toxicity characteristics include arsenic, chromium, and SVOCs (e.g., PCP). It is anticipated that solids characterization would be required quarterly during the steam phase of treatment and annually during the cool-down phase.

6.8 SITE PERIMETER ENVIRONMENTAL MONITORING

The objectives of site perimeter monitoring are to evaluate potential impacts of the thermal treatment system operations on the surrounding community and to demonstrate substantive compliance with local, state, and federal environmental regulations. Perimeter monitoring will be focused on measuring and evaluating impacts beyond the perimeter of the site.

6.8.1 Noise Monitoring

The objectives of noise level monitoring are to evaluate the impacts on the surrounding community and to ensure that the exposure of workers to on-site noise complies with the Occupation Safety and Health Act (OSHA) during all operational phases of the thermal treatment system. The federal role in regulating noise is predominately limited to transportation, workplace activities, and certain types of machinery. State and local governments determine the extent to which all other sources of noise are controlled, and regulations for such sources can vary widely among localities. The state of California has not established any standards to restrict noise but does provide model ordinances to assist municipal governments in developing noise-control programs. Sources of noise commonly regulated at the state and local level include commercial, industrial, and residential activities. Regulations for such sources typically control the public's exposure to irritating or potentially harmful noise levels by limiting the activity concerned to specific times of the day.

Prior to actual treatment operations, a background study would be conducted to measure ambient noise conditions at the three or four community monitoring locations. Once ambient noise levels have been established, several monitoring events would be conducted at the site during active construction activities to maintain compliance with worker-related (OSHA) and local ordinances. Background and monitoring events may use Type I or Type II sound level meters with a demonstrated accuracy of ± 1 dBA for Type I meters and ± 2 dBA for Type II meters. At the time of testing, wind speed should not exceed 12 mph, and no testing should occur when precipitation is falling at a rate that would affect measurement readings. Typically, a randomincidence microphone is used and oriented vertically to assess ambient noise levels from all potential noise sources. A 25-hour noise-monitoring period is recommended, with hourly noise statistics.

Technically, noise generated by construction activity is exempt from community noise regulations. At the discretion of the EPA, noise monitoring can be instituted during construction to address specific concerns of stakeholders or the surrounding community.

6.8.2 Air Quality

Construction on the site has the potential to create dust that could impact the ambient air quality of the surrounding community. While engineering controls are likely to control the generation of dust during construction, ambient air quality monitoring may be conducted to verify that air quality during construction activities substantively complies with local, state, and federal ambient air standards for airborne particulates. Initial air monitoring is described below.

Air monitoring during construction may include the use of an MIE DataRAM, a realtime aerosol monitor (RAM). The instrument would be placed downwind of construction activities (based on

meteorological data). The MIE DataRAM would monitor dust generated on site by construction and operation/maintenance activities, as well as ambient dust levels unrelated to site activities. The monitors can be operated continuously and have the ability to collect and log the data. Analytes for construction monitoring include PM_{10} (particulate matter with a mass median aerodynamic diameter less than 10 micrometers), PCP, and PAHs. Samples would be collected every 8 hours for the first 7 days during construction activities.

During operations, air monitoring instruments would remain at stationary locations (to be determined) surrounding the treatment areas. Analytes for operation monitoring include PM_{10} , CO, SO_x, NO₂, PCP, and PAHs. Particulate matter would be monitored at three stations using a RAM every 8 hours for the 7 days of operation. If any 8-hour measurement exceeds 50 µg/m³ in 8 hours, then a RAM would be collocated with a Hi-Vol Sampler or equivalent to determine if the concentration exceeds 150 µg/m³ over 24 hours. If this value is exceeded, institutional controls (such as a temporary buffer zone) would be implemented until concentrations decrease below exposure limits. PCP and PAHs would be monitored with a canister sampler, and samples would be sent for laboratory analysis. CO, SO_x, and NO₂ would be monitored with a CEMS. Samples for all of the organics would be taken every 8 hours for the first 7 days of operation. If no permissible exposure limits (PELs) are exceeded, sampling intensity could be decreased.

6.9 GROUNDWATER MONITORING

Compliance monitoring for groundwater would be conducted at the perimeter during treatment (steam phase and cool-down phase) and at the perimeter and within the treatment zone for long-term monitoring (after treatment is complete).

Perimeter monitoring would be conducted to ensure compliance with relevant RAOs. Select perimeter monitoring wells would be analyzed for groundwater contaminant concentrations, and all monitoring wells located outside of the thermal treatment zones would be used to determine hydraulic gradients (horizontal and vertical). Analysis of groundwater samples would be for COCs identified in the Record of Decision (USEPA 1999), including PAHs, PCP, dioxins/furans, and metals (arsenic, chromium, copper, and zinc). Information obtained from the perimeter monitoring well network would be used to evaluate contaminant concentration trends (increasing, decreasing, or stable), horizontal gradients (toward the site, away from the site, or flat), and vertical gradients (downward, upward, or neutral). If that analysis indicates that off-site migration of contaminants is occurring, then remedy modifications would be considered.

To accommodate requirements for determining horizontal hydraulic gradients, new perimeter monitoring wells would be installed as pairs, located less than 100 feet apart and screened across the same stratigraphic interval. Wherever possible, existing monitoring wells would be used, and

a new perimeter monitoring well would be installed nearby. For each monitoring well pair, only one would be sampled periodically for contaminant concentrations (Table 6-5).

Monitoring frequency and duration will depend on the amount of NAPL removal achieved under the three remediation scenarios. Additionally, the monitoring frequency for the 7-year period of active remediation (including cool-down period) will be different from that of the postremediation period. Because mobility of contaminants is enhanced during thermal treatment, monitoring for all scenarios and in all zones would take place semi-annually for the period through cool-down. For all scenarios after cool-down, complete restoration of the D- and E-zones is assumed. For cost estimating purposes, monitoring in the D- and E-zones would occur every 5 years as part of the 5-year review process. More frequent sampling may be required based on remedy effectiveness monitoring and data quality objectives that have yet to be developed by EPA, the state of California, and other stakeholders. Monitoring is assumed to continue for 30 years or until the site is delisted. In zones A through C, a significant amount of NAPL may be left in place, since fewer pore volumes of steam are flushed through the soils. Monitoring would, therefore, take place annually for 30 years and the analyte list would be reduced to PAHs and PCP. The monitoring program is outlined in Tables 6-3, 6-5, and 6-6.

6.10 SOIL CONFIRMATION SAMPLING

Soil borings would be collected at the end of the cool-down phase. An LIF survey would be conducted to select soil samples for analysis. It is anticipated that 10 samples would be collected and tested for TPH, PAHs, and PCP. In addition, an estimated 10 NAPL mobility leach tests would be conducted to evaluate the effectiveness of the thermal treatment at reducing contaminant mobility.

Table 6-1 Overview of Sampling Frequency for Subsurface ERT and Temperature Monitoring

	Gener Cove	al Site erage	Vertica South of	l Plane f Slough	Horizontal Plane Under Slough	New P&T Extraction Wells	New P&T Monitoring Wells	
Phase	ERT	DTS	ERT	DTS	ERT	DTS	DTS	
Heat-up	Weekly	Daily	Weekly	Daily	Daily	Daily	Daily	
Flushing/cycling	Monthly	Weekly	Monthly	Weekly	Daily	Weekly	Weekly	
Cool-down	Quarterly	Monthly	Quarterly	Monthly	Weekly	Monthly	Monthly	
Long-term	NA	NA	NA	NA	NA	NA	Quarterly	
monitoring								

Table 6-2
National Ambient Air Quality Standards

	Standard Value	
Constituent	$(\mu g/m^3)$	Standard Type
СО		
1- Hour average	40 mg/m^3	Primary
8-Hour average	10 mg/m^3	Primary
NO ₂		
Annual arithmetic mean	100	Primary and secondary
O ₃		
1-Hour average	235	Primary and secondary
8-Hour average	157	Primary and secondary
Pb		
Quarterly average	1.5	Primary and secondary
PM ₁₀		
Annual arithmetic mean	50	Primary and secondary
24-Hour average	150	Primary and secondary
SO ₂		
Annual arithmetic mean	80	Primary
24-Hour average	365	Primary and secondary
3-Hour average	1,300	Primary and secondary

Note: Units in microgram per cubic meter of air, unless otherwise noted.

				No. of	
			_	Sampling	Total No.
Characteristic	Media	Analyte	Frequency	Points	per Year ^a
Performance Monitoring	g—Steam Phase				
ERT data	Subsurface	Temperature	Daily/weekly/	41 to 63 vertical	NA
			monthly (see	4 horizontal	
			Table 6-1)		
Electrical	System	Amps and power	Weekly	59 ERH electrodes	NA
Temperature/DTS	Wellhead	Temperature	Daily/weekly	301 to 416 wells	NA
distribution			(see Table 6-1)		
Steam distribution	Wellhead	Liquid flow	Continuous	169 to 249	NA
	Wellhead, boiler plant,	Pressure	Continuous	200 to 300	NA
	treatment plant				
Contaminant removal	NAPL (at wellhead)	Volume	Weekly	132 to 167	NA
rate	Contaminated water	Flow	Continuous	132 to 167	NA
		Volume	Weekly	4 to 6	NA
		TOC	Weekly	4 to 6	NA
	Vapor	Flow	Continuous	4 to 6	NA
	(up or	CO	Continuous	4 to 6	NA
		PAHs	Continuous	4 to 6	NA
		PCP	Continuous	4 to 6	NA
Well installation soil	Soil	Visual	Initial	20	20
horings ^b	5011	TPH	Confirmation	20	20
bornigs		PAHs	Confirmation	20	20
		PCP	Confirmation	20	20
Interim drill-back soil	Soil	ТРН	Δnnual ^c	50 ^d	50 ^d
horings	5011	PAHe	Annual ^c	50 ^d	50 ^d
bornigs		PCP	$\Delta nnualc$	50 ^d	50 ^d
Groundwater monitoring ^b	Water	TOC	Initial	60	60
Groundwater monitoring	vv ater	DAHe	Initial	60 60	60
		PARS DCD	Initial	00 60	60
		I CI Motala ^e	Initial	60	60
		Dioxing/furanc	Initial	10	10
Croundwater treatment	Watar	TOC	Wookly	10	52
plant maniforing	vv ater	DALL	Monthly	1	12
plant monitoring		РАПЯ	Monthly	1	12
		PCP	Monthly	1	12
Cool Down Phase	0.1.0	T		201 / 41 / 11	214
Temperature/DTS	Subsurface	Temperature	Monthly (see T_{1})	301 to 416 wells	NA
distribution		T 7 1	Table 6-1)	100 - 177	27.4
Contaminant removal	NAPL (at well-head)	Volume	Monthly	132 to 167	NA
rate	Contaminated water	Flow	Continuous	132 to 167	NA
		Volume	Monthly	4 to 6	NA
		TOC	Monthly	4 to 6	NA
	Vapor	Flow	Continuous	4 to 6	NA
		CO_2	Continuous	4 to 6	NA
		PAHs	Continuous	4 to 6	NA
		PCP	Continuous	4 to 6	NA

Table 6-3 Performance and Compliance Monitoring and Confirmational Sampling

Table 6-3 (Continued) Performance and Compliance Monitoring and Confirmational Sampling

				No. of Sampling	Total No
Characteristic	Media	Analyte	Frequency	Points	ner Year ^a
Performance Monitoring	Cool Down Phase (Co	ontinued)		1 0 1110	
Groundwater treatment	Water	TOC	Weekly	1	52
plant monitoring		PAHs	Monthly	1	12
r		PCP	Monthly	1	12
Compliance Monitoring-	—Steam Phase	-			
Boiler air emissions/	Gas	VOCs	Initial	1	4 ^f
stack monitoring		SVOCs ^g	Initial	1	4^{f}
e		Dioxins/furans	Initial	1	4^{f}
		Metals ^h	Initial	1	4^{f}
		PM	Initial	1	4 ^f
		HCl	Initial	1	4 ^f
		Cl_2	Initial	1	4 ^f
		Total hydrocarbons	CEMS	1	NA
		CO	CEMS	1	NA
		SO_x	CEMS	1	NA
		NO _x	CEMS	1	NA
		Opacity	CEMS	1	NA
		CO	CEMS	1	NA
		SO_x	CEMS	1	NA
		NO _x	CEMS	1	NA
Site fugitive	Air	PAHs	Monthly	3	36
emissions/air quality		PCP	Monthly	3	36
perimeter monitoring		PM_{10}	Monthly	3	36
·		Metals	Monthly	3	36
		СО	CEMS	3	NA
		SO_x	CEMS	3	NA
		NO ₂	CEMS	3	NA
Treatment plant	Water	TPH	Monthly	1	12
discharge (groundwater		PAHs	Monthly	1	12
re-injection)		PCP	Monthly	1	12
		Metals ^e	Monthly	1	12
		Dioxins/furans	Quarterly	1	4
Waste disposal	NAPL	TPH	Quarterly	1	4
characterization		PAHs	Quarterly	1	4
		PCP	Quarterly	1	4
		Metals ^e	Quarterly	1	4
		Dioxins/furans	Quarterly	1	4
	Solids (well)	TCLP ⁱ	Quarterly	1	4
	Solids (SAC)	TCLP ⁱ	Quarterly	1	4
	Solids (sludge)	TCLP ⁱ	Quarterly	1	4
	Solids (spent filter	TCLP ⁱ	Quarterly	1	4
	media)				
Perimeter groundwater	Water	PAHs	Semi-annual	25	50
monitoring		PCP	Semi-annual	25	50
		Metals ^e	Semi-annual	25	50
		Dioxins/furans	Semi-annual	25	50

				No. of Sompling	Total No
Characteristic	Media	Analyte	Frequency	Points	per Year ^a
Cool-Down Phase		<u> </u>	<u> </u>		
Site fugitive	Air	PAHs	Quarterly	3	12
emissions/air quality		PCP	Quarterly	3	12
perimeter monitoring		PM	Quarterly	3	12
Treatment plant	Water	ТРН	Monthly	1	12
discharge (groundwater		PAHs	Monthly	1	12
re-injection)		PCP	Monthly	1	12
		Metals ^e	Monthly	1	12
		Dioxins/furans	Quarterly	1	4
Waste disposal	NAPL	TPH	Annually	1	1
characterization		PAHs	Annually	1	1
		PCP	Annually	1	1
		Metals ^e	Annually	1	1
		Dioxins/furans	Annually	1	1
	Solids (well)	TCLP ⁱ	Annually	1	1
	Solids (SAC)	TCLP ⁱ	Annually	1	1
	Solids (sludge)	TCLP ⁱ	Annually	1	1
	Solids (spent filter	TCLP ⁱ	Annually	1	1
D 1 4	media)	TDU	0 1	25	50
Perimeter groundwater	Water	TPH	Semi-annual	25	50
monitoring		PCP Matala ^e	Semi-annual	25	50
		Disaving/formers	Semi-annual	25	50
C C C C C C C C C C		Dioxins/lurans	Semi-annual	25	50
Confirmational Sampling	Soil	LIE currier	Final	4 waales	4 maalra
Son borings	5011	NAPI mobility	Final	4 weeks	4 weeks
		InAFL III00IIIty	rillai	10	10
		TDH	Final	10	10
		PAHe	Final	10	10
		PCP	Final	10	10
Long-Term Monitoring	(30 years)	101	1 mai	10	10
Groundwater monitoring	Water	PAHs	Annually	25	25
(perimeter and treatment		PCP	Annually	25	25
zones)					

Table 6-3 (Continued) Performance and Compliance Monitoring and Confirmational Sampling

Table 6-3 (Continued) Performance and Compliance Monitoring and Confirmational Sampling

^aTotal number of samples does not include QA samples. QA samples are 10% per batch.

^bIncludes baseline sampling and analysis during system (e.g., wells) installation.

^cWill be conducted for years 2, 3, and 4 (i.e., 3 times over the course of the steam treatment period).

^dAssumes 5 samples from each of 10 borings.

^eMetals include As, Cr, Cu, and Zn.

^fIncludes 3 samples taken from 1 boiler stack and 1 blank.

^gSVOCs include PAHs and PCP.

^h40 CFR 266.106 provides standards to control metals emissions. Subpart I regulated metals include Ag, As, Ba, Be, Cd, Cr, Hg, Pb, Sb, and Th. Of these metals, As and Cr are of concern at the site.

ⁱLeachate from TCLP analyzed for As, Cr, and SVOCs

ⁱNAPL Mobility Product Leaching Study, methodology leach test to be determined.

Notes:

Performance monitoring: Monitor system performance during remediation. Includes steam phase and cool-down period. Compliance monitoring: Monitor compliance with disposal/discharge requirements. Includes steam phase and cool down period. Confirmational sampling: To confirm "long-term effectiveness" of the remediation performed.

Ag: silver As: arsenic Ba: barium Be: beryllium CEMS: continuous emissions monitoring system Cd: cadmium CO: carbon monoxide CO₂: carbon dioxide COCs: chemicals of concern (PAHs, PCP, dioxins/furans, As, Cr, Cu, and Zn) Cl₂: chlorine gas Cr: chromium Cu: copper HCl: hydrogen chloride Hg: mercury LIF: laser-induced fluorescence NO_x: nitrogen oxides O₃: ozone PAHs: polynuclear aromatic hydrocarbons Pb: lead PCP: pentachlorophenol PM: particulate matter SAC: spent activated carbon Sb: antimony SO_x: sulfur oxides SVOCs: semivolatile organic compounds TAPs: toxic air pollutants TCLP: toxicity characteristics leaching procedure Th: thallium TOC: total organic carbon TPH: total petroleum hydrocarbons VOCs: volatile organic compounds Zn: zinc

	FPA	FPA	FPA IRIS	EPA IRIS 1 x 10 ⁻⁶ Cancer Risk Estimates	EPA Drinking Water Health Advisory	NAS Drinking Water Health Advisory	EPA Drinking Water Health Advisory	CA	CA	CA Public	CAL/EPA Cancer	CA Proposition 65	CA State	CA State Action Other Taste	Agricultural Water	gricultural Quan Water Tap Methe Ouality Water Par	Quantitation Limits or Method 1613B	Background Levels		
Compounds	Primary MCL	Secondary MCLs	Reference Dose	1 x 10 ⁻⁶ Cancer Risk Estimates	(SNARLs) for Toxicity	(SNARLs) for Toxicity	(SNARLs) for Cancer	Primary MCLs	Secondary MCLs	Health Goals	Potency Factor	Regulatory Level	Level - Toxicity	and Odor Thresholds	Quality Goals ⁴	Water PRGs	Reporting Limits	Well OFS-5A ¹	¹ Well OFS-5C	Well OFS-5E
PAHs µg/L (Method 8270))		_					-			-	_	_	-			-	_	-	
Acenaphthene	NA	NA	420	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	370 - nc	0.1 - 10	ND	ND	ND
Anthracene	NA	NA	2100	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1800 - nc	0.1 - 10	ND	ND	ND
Benzo(a)anthracene	0.15	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.029	0.02 ¹	NA	NA	NA	0.092 - ca	0.1 - 10	ND	ND	ND
Benzo(a)pyrene	0.2	NA	NA	NA	NA	NA	NA	0.2	NA	0.004	0.029	0.02 ¹	NA	NA	NA	$0.0092 - ca^9$	0.1 - 10	ND	ND	ND
Benzo(b)fluoranthene	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.029	0.02 ¹	NA	NA	NA	0.092 - ca	0.1 - 10	ND	ND	ND
Benzo(g,h,i)perylene ¹²	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.1 - 10	ND	ND	ND
Benzo(k)fluoranthene	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.029	NA	NA	NA	NA	0.92 - ca	0.1 - 10	ND	ND	ND
Carbazole	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.4 - ca	10	ND	ND	ND
Chrysene	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.29	0.11	NA	NA	NA	9.2 - ca	0.1 - 10	ND	ND	ND
Dibenzo(a,h)anthracene	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.085	0.11	NA	NA	NA	0.0092 - ca	0.1 - 10	ND	ND	ND
Dibenzofuran	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	24 - nc	10	ND	ND	ND
Indeno(1,2,3-cd)pyrene	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.029	NA	NA	NA	NA	0.092 - ca	0.1 - 10	ND	ND	ND
Fluorene	NA	NA	280	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	240 - nc	0.1 - 10	ND	ND	ND
Fluoranthene	NA	NA	280	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1500 - nc	0.1 - 10	ND	ND	ND
1-Methylnaphthalene ¹²	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NS	NS	NS
2-Methylnaphthalene ¹²	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.1 - 10	ND	ND	ND
Naphthalene	NA	NA	14	NA	100	NA	NA	NA	NA	NA	NA	NA	170	217	NA	6.2 - nc	0.1 - 10	ND	ND	ND
Phenanthrene	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.1 - 10	ND	ND	ND
Pyrene	NA	NA	210	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	180 – nc	0.1 - 10	ND	ND	ND
Pentachlorophenol (PCP)	1	NA	NA	0.3	300 (10-day)	6/216	0.3	1	NA	0.4	0.43	20	NA	30 ⁸	NA	0.56 - ca	0.0074 - 25	ND	ND	ND
Metals µg/L (Method 601	0)																			
Zinc	NA	5000	2100	NA	2000	NA	NA	NA	5000	NA	NA	NA	NA	NA	2000	11000 - nc	20	569.0	24.0	22.8
Arsenic	50 ¹⁰	NA	2.1	0.02	NA	NA	0.02 ¹	50	NA	NA	0.023	5	NA	NA	100	0.045 - ca	10	18.1	30.0	43.3
Chromium	100	NA	21 ³	NA	1000 (10-day)	NA	NA	50	NA	2.5 ²	0.18 ³	NA	NA	NA	100 ³	$110 - nc^{3}$	10	2.2	1.7	1.5
Copper	1300	1000	NA	NA	NA	NA	NA	1300	1000	170	NA	NA	NA	NA	200	1400 - nc	25	0.8	1.3	1.3
Dioxins pg/L (Method 16	13B)																			
2,3,7,8-TCDD TEQ	30	NA	NA	NA	10 (10-day)	700	0.2	30	NA	NA	0.27	2.5	NA	NA	NA	0.45 - ca	10	3.0	10.5	NS

Table 6-4Compliance Criteria for Groundwater Re-injection

Section 6.0 11/12/01 Page 6-17

Table 6-4 (Continued) **Compliance Criteria for Groundwater Re-injection**

- Notes: NS Not sampled
- ND Non Detect
- NA Not Applicable
- ca Cancer

nc-Non-Cancer

- In calculation of the mean, values with a U qualifier were divided by 2.
- The mean for well OFS-5A was calculated using data from sampling events taking place on 14 November 1995 and July 1998.
 The mean for well OFS-5C was calculated using data from sampling events taking place on 14 November 1995, 3 August 1997, and July 1998.
 The mean for well OFS-5E was calculated using data from sampling events taking place on 14 November 1995 and 3 August 1997.
- All criteria, except the Tap Water PRGs, were obtained from <u>A Compilation of Water Quality Goals</u> prepared by the California Environmental Protection Agency, Regional Water Quality Control Board Central Valley Region, August 2000.
- The Tap Water PRGs were obtained from the EPA Region 9 website (http://www.epa.gov/region09/waste/sfund/prg/files/PRG2000.pdf).

¹Draft

²Based on the assumption that 7.2% of Cr is Cr(VI)

³Value for Chromium(VI)

⁴From Ayers, R.S. and D. W. Westcot, Water Quality for Agriculture, Food and Agriculture Organization of the United Nations-Irrigation and Drainiage Paper No. 29, Rev. 1, Rome (1985).

⁵Proposed

⁶child/adult

⁷From J.E. Amoore and E. Hautala, Odor as an Aid to Chemical Safety: Odor Thresholds Compared with Threshold Limit Values and Volatilization for 214 Industrial Chemicals in Air and Water Dilution. Journal of Applied Toxicology, Vol. 3, No. 6 (1983). ⁸From U.S. Environmental Protection Agency, Office of Water, Nation Primary Drinking Water Regulations, Contaminant Specific Fact Sheets-Technical Version (October 1995).

⁹CAL-modified PRG is 0.0015.

¹⁰ Potentially lowered to 10 µg/L

¹¹The results for Well OFS-5A for the sampling which took place 8/8/96 were not used because the quality of the data was questionable.

¹²No compliance criteria were identified in any of the sources for these chemicals.

Section 6.0 11/12/01 Page 6-19

	Zone									
Scenario	Α	В	С	D	Е					
1	7	6	6	3	3					
2	7	6	4	3	3					
3	7	6	4	3	3					

Table 6-5Perimeter Monitoring Wells to Be Sampled

Table 6-6Perimeter Monitoring Frequency

				Zone							
Years ¹	Scenario	Α	В	С	D	Е					
	1										
0-7	2	Semi-annually									
	3										
	1	Annually	Annually	Annually	5-years	5-years					
7-30	2	Annually	Annually	Annually	5-years	5-years					
	3	Annually	Annually	Annually	5-years	5-years					

¹Years are from start of thermal remediation.

7.0 DISCUSSION OF DESIGN

7.1 EVALUATION OF LIQUID-PHASE GAC USAGE AND REGENERATION

The estimated load on the liquid-phase GAC system was based on actual GAC consumption from the Visalia Pole Yard remediation (Southern California Edison 2000), assuming the following:

- The dissolved COC levels in the water separated from the NAPL will be approximately the same at McCormick Baxter, since the water treatment systems are similar (dissolved air flotation [DAF] followed by multimedia filtering prior to GAC).
- The COC levels will be similar to those at Visalia; the water is likely to be saturated with the COCs due to close contact with NAPL.
- The load was determined by the total pumping rate, based on an average rate of 400 gpm at Visalia.
- Since more mass is in place at McCormick and Baxter and the remediation will take longer, the monthly carbon usage was stretched out over longer periods than at Visalia. For example, Visalia used the peak 27,000 pounds per month for only 6 months, but we estimate a proportional maximum usage at McCormick Baxter to last 12 months.
- The carbon usage was decreased so that after 3 years of cool-down, it would be at a level that corresponds to the pump-and-treat usage observed at Visalia prior to thermal treatment. This is a conservative, but safe calculation.

The estimated GAC usages range from 1.1 to 1.6 million pounds of GAC for the three scenarios (Table 7-1). Also, the predicted replacement frequency for the primary GAC vessels is high during the thermal treatment, leading to a high labor demand for this service.

The potential for using on-site regeneration of the primary GAC vessels was evaluated. Estimated regeneration frequencies and GAC replacement and total usage are provided in Table 7-1. The total GAC usage is about 10 percent of the usage for the sacrificial GAC option. Assumption for these estimates include the following:

- The GAC is regenerated on average when the primary vessels are about 50 percent loaded with COCs.
- Each primary vessel can be regenerated 20 times before the GAC is replaced.

Based on capital costs for the GAC units and a potential saving of at least 1,000,000 pounds of GAC for each scenario, which may be regenerated by steam on site, the on-site regeneration option is favorable.

7.2 DISCHARGE OPTIONS FOR EXCESS TREATED WATER

Several options for getting rid of the excess water were considered:

- Injection of the treated water into the E-zone aquifer
- Discharge to surface water (Old Mormon Slough)
- Discharge to the city wastewater treatment facility
- Evaporation in large holding ponds
- Industrial water reuse

This section provides a brief summary of the conclusions.

7.2.1 Injection of Treated Water Into E-Zone

This option is the recommended option. The water quality criteria to be met are summarized in Table 6-4. The general requirements are as follows:

- Organic COCs will be less than background E-zone aquifer levels or MCLs (this is readily met by dual-pass GAC filtration).
- Heavy metal concentrations cannot substantially exceed the existing E-zone levels.

Details of the heavy metals evaluation are provided in Appendix A. It was concluded that heavy metals treatment by a dedicated treatment unit is not needed.

The re-injection design will be finalized at a later design stage. Important considerations will include the following:

- At least three re-injection wells are recommended in order to always have at least one well that can be used for injection, while other wells are being serviced.
- The well locations will be optimized by the use of numerical modeling.
- Both upgradient and downgradient placement of the re-injection wells will be considered. However, downgradient injection is preferred during steam injection to reduce the heat losses that would result from increasing the groundwater flow velocity through the E-zone. Upgradient injection should be considered for oxygen delivery to the target zone.
- Cooling of the water has not been designed, since warmer water will encourage biological degradation.
- Special attention will be paid to the potential for clogging of the re-injection wells due to elevated temperatures and the oxygen content in the water (re-injection of aerobic water into an anaerobic aquifer may lead to precipitation of iron and manganese oxides, as well as biofouling).
- Redevelopment procedures for the wells will be defined (this is routinely done in oil-field applications where process water is injected into disposal wells).

7.2.2 Discharge to Surface Water (Old Mormon Slough)

Surface water discharge was identified as the least desirable option by the State. Treatment for As and Zn may be necessary, and it is likely that Cu and Cr would also require treatment. The capital cost for a dedicated heavy metals treatment system is on the order of \$1 million for a 500-gpm unit, with substantial operating costs as well.

7.2.3 Discharge to City Wastewater Facility

The same arguments as those for surface water discharge are valid for discharge to the city wastewater facility. Heavy metals treatment would potentially be needed. In addition, it was established that the City cannot accept the quantities of water to be generated at the McCormick and Baxter site (discharge rates as high as 400 gpm at times).

7.2.4 Evaporation in Large Holding Ponds

This option was ruled out due to practical constraints, including the following:

- The existing holding ponds cannot be used for this purpose during the winter months during which the capacity will be used for stormwater runoff.
- Evaporation ponds for a 400-gpm evaporation rate would be excessively large and cannot be located on site without great difficulty and cost.

In conclusion, the re-injection option is the preferred alternative. However, it is possible that the final solution will also involve some discharge to the city wastewater treatment facility. The decision will be made at a later design stage.

7.3 POTENTIAL FOR RECYCLING RECOVERED NAPL FOR STEAM GENERATION

The disposal cost for several hundred thousand gallons of recovered NAPL will be substantial. The possibility of using the NAPL as a supplemental fuel deserves evaluation. The final decision on whether to recycle the NAPL will be made at a later design stage. At this point, offsite disposal was included in the cost estimates.

7.3.1 Destruction of NAPL as Supplemental Fuel

The NAPL can be vaporized and sprayed into the combustion flame of the oxidation chamber using standard techniques from oil-field firing with crude oil. In the flame section, the organic COCs will be exposed to high temperatures and oxygen and will burn along with the natural gas. Energy will be reclaimed from the NAPL (estimated order of about 100,000 BTU per gallon, depending on the water content). Inorganic COCs, such as heavy metals, are not expected to be destroyed, but will pass through the flame and be collected with the ash. Particulate-borne COCs will be removed by polishing of the off-gases (scrubbing and particle removal); see Section 7.3.3.

7.3.2 Steam Generator Combustion Chamber Conditions

To prevent NO_x formation by oxidation of N₂, the temperature should be limited preferably below 900°C (1,650°F), and the residence time in the flame should be minimized.

To facilitate complete COC destruction, the temperature should be as high as possible (815 to 900°C, or 1,500 to 1,650°F), and the residence as long as possible (minimum of 1 second in the chamber between the injection point and the thermocouple used for temperature verification).

The desired temperature range is around 1,500 to 1,600°F. This is hot enough to allow thermal destruction of COCs (given enough residence time, 1 second or more), to prevent dioxin/furan formation, and low enough to prevent excessive thermal NO_x formation.

These specifications are independent of the COC load, as long as plenty of oxygen is supplied by the combustion air blower.

7.3.3 Off-Gas Polishing Need

The critical components in the off-gas are as follows:

- COCs
- Dioxins/furans
- NO_x
- SO_x
- CO/CO₂
- HCl
- Hydrogen fluoride (HF)
- Particulates

Chlorinated solvents may form gaseous HCl when oxidized. Acid gas scrubbing is often required to control HCl emissions. This is done by adding caustic (typically NaOH) in a wet quench/scrubber. However, creosote has a relatively low chlorine content. The very dilute HCl gas that would be produced during creosote oxidation is unlikely to result in nuisance conditions that would require an acid gas scrubber.

 NO_x may be formed by oxidation of nitrogen species from air (free nitrogen), from the fuel, or from ammonia. However, most low- NO_x burners and oxidation chambers will run with less than 30 parts per million vapor (ppmv) NO_x emitted with the stack gas, and polishing should not be necessary.

Dioxins and furans form at temperatures between 250°C and 450°C (480°F and 840°F). The temperature and residence time should be controlled to minimize their formation. Dioxins/furans are destroyed at temperatures above 950°C (1,930°F) and also at lower temperatures if the residence time in the chamber is long enough. Gas polishing can be done either by catalytic oxidation or carbon adsorption.

The final criteria and the resulting flue gas treatment typically are determined by means of discussion with the air pollution authority in the area. We cannot assume certain criteria at this point. At this point, if gas polishing is required at all, the minimum flue gas treatment would be the following (Visalia required no treatment):

- Quenching
- Scrubbing of HCl

The emitted air would be cool and have low concentrations of HCl, HF, SO_x and COCs. NO_x concentration would not be significantly affected, but should be less than 30 ppmv anyway if a low-tech flue gas recirculating burner is used. With this setup, dioxins and furans would be destroyed in the oxidation chamber and prevented from forming by the rapid quenching of the flue gas.

If necessary, a more comprehensive vapor treatment can be designed, consisting of the following:

- Quenching
- Scrubbing of HCl
- Ammonia injection and catalytic NO_x removal
- Dioxin removal using either catalytic oxidation or carbon filtration
- High-efficiency particulate air (HEPA) filter or filter bag with associated baghouse to capture particulates (or an electrostatic precipitator)

One catalytic unit may accomplish both NO_x and dioxin removal. The best option we have found so far is the Zeronox D catalyst. A design with heat exchangers is needed to optimize the whole vapor treatment train. But since NO_x treatment should be avoided due to high cost and maintenance, this option can be ruled out.

Carbon filtration for dioxin removal can be performed above 100°C to prevent condensation of water vapors. Preliminary specifications from Calgon call for a unit with 11,000 pounds of activated charcoal with a minimum of 3 seconds residence time, when operated at 250°F at 7,500 standard cubic feet per minute (scfm).

In conclusion, the technology exists to limit dioxin and furan emissions during on-site recirculation of NAPL for energy reclamation. However, crucial permitting issues need to be evaluated before this solution can be chosen.

7.4 REVISION OF TARGET VOLUMES DURING DRILLING/INSTALLATION

For Scenarios 1 and 2, the E-MPA is not included as a priority for thermal treatment. However, since naphthalene was found in concentrations as high as 9 mg/L in well MW-4E, the presence of NAPL in the E-MPA is uncertain. A flexible approach to this problem is suggested for Scenarios 1 and 2:

- During installation of the steam injection wells on the east side of the MPA, soil samples will be collected in the C-, D-, and E-zones to assess the potential presence of NAPL.
- An extraction well will be installed close to MW-4E in the C-, D-, and E-zones, and soil samples will be collected to assess the potential presence of NAPL.
- If NAPL is found in the samples, another set of wells will be installed farther east, and additional samples will be collected to assess the potential presence of NAPL.
- If the outermost NAPL-containing well is a logical injection well (based on the overall pattern of injectors and extractors), then an extraction well will be placed farther east in order to minimize the potential for the spread of NAPL.
- If the outermost NAPL-containing well is a logical extraction well (based on the overall pattern of injectors and extractors), then an injection well will be placed farther east in order to push the NAPL back toward the extraction well.

For example, dissolved naphthalene concentrations in MW-4E were in the 9 mg/L range in November 2000, leading to the selection of this location for an extraction well. The observed groundwater quality will lead to the selection of this drilling location for soil sampling during drilling and well installation.

Should additional data become available on the groundwater and soil concentrations of COCs, this approach may be revised at a later design stage.

7.5 OPPORTUNITIES FOR PHASING CONSTRUCTION AND OPERATION

The three thermal treatment scenarios described in this conceptual design each would have an operational period of 7 years, with the construction of all wells and process equipment during an intensive 2-year period prior to operation. This would lead to a very high spending rate during the first years of the project, potentially exceeding the rate at which funds would become available for remediation.

The proposed scenarios would be feasible and possible provided that budgets allow for the high spending rate. However, there are other options for implementing thermal remediation at this site for which the present value and lifecycle costs would be more acceptable from a funding standpoint.

A screening of the options for a less aggressive spending rate and ways to reduce the present value cost of the thermal remediation was performed (Section 7.5.1). Sections 7.5.2 and 7.5.3 present an example phasing approach with a much lower initial yearly cost. The cost implications are briefly mentioned here and in more detail is provided in the cost estimate (USEPA 2001).

7.5.1 Reason for Discussing Longer Spending Period

The life cycle cost of Scenario 1 for thermal treatment is approximately \$83.2 million. This money would be spent according to the following time line:

- Two years for design, bidding, and contract award (\$2.3 million)
- Three years for procurement, mobilization, field construction and startup (\$31.3 million)
- Four years for subsurface heating and intensive thermal treatment, vapor and groundwater extraction, NAPL/groundwater treatment, and groundwater monitoring (varies from \$10.3 million [year 1] to \$6.8 million [year 4], cumulative cost of \$34.2 million)
- Three years for subsurface cool-down, including ongoing treatment of extracted liquids and vapors and groundwater monitoring (varies from \$2.9 million [year 5] to \$4.9 million [year 7], cumulative of \$10.7 million)
- Twenty-three years of groundwater monitoring (cumulative \$3.0 million, starts after completion of active thermal and cool-down)
- Periodic costs over 30-year period (cumulative \$1.8 million, starts after completion of field construction)

The bulk of the cost (approximately \$79 million, or 95 percent of the LCC) will be incurred during the first 12 years following EPA's decision to proceed with the design of the thermal treatment system. This would be an average spending of about \$7 million per year, with the highest yearly spending during construction (\$31.3 million over 3 years) and active thermal treatment (up to \$10.3 million in a single year).

In reality, other options for managing the design, construction, and operations phases of the thermal remediation effort are available. These options include the following:

- Stretching the construction phase over more years for better match to the availability of funds. This would concern drilling, well installation, and the surface process equipment.
- Stretching out the drilling and installation phase and conducting interim aquifer tests for improved steam injection rate modeling. After limited well testing in each aquifer zone, the well-field design would be revisited. Potential savings would include a longer spending period and a potential for reducing the total drilling and construction effort.
- Treating different depth zones sequentially in order to reduce the overall equipment size and the cost of the initial construction phase.

After the initial screening, we selected the approach described in the next subsection as an example.

7.5.2 Description of Example Phasing Alternative

The Phasing Alternative consists of four steps, each projected to last 3 years, for a total heating period of 12 years:

1. Focus on depth interval from -190 to -270 feet of elevation. Thermal treatment of the E- and D-E-zones with extraction in D-, D-E-, and E-zones and limited hotspot extraction in the C-zone. During this period, steam would be injected in the E-zone aquifer over the area including the MPA and the N-MPA. Electrical heating would be conducted at a reduced rate in the upper two-thirds of the D-E aquitard. The affected volume of material in Step 1 is estimated to be 296,000 yd^3 , or about 19 percent of the Scenario 1 volume. The necessary steam demand was calculated assuming that the target volume would be heated in a period of 360 days, followed by 2 years of continued operation at a reduced average rate. Since only the D-, D-E-, and D-zones would be affected by heating, the necessary extraction rate would be limited to 150 percent of the equivalent steam injection rate (estimated at 23 MM BTU/hr or 46 gpm) plus the pumping rate necessary to maintain hydraulic control in the D- and E-zone aguifers (51 gpm). The resulting design rate for the effluent treatment system would be 150 gpm, and similarly the vapor extraction rate was estimated at 100 scfm (there is no vadose zone in the

treatment interval). All the VEAs covering the ERT and temperature monitoring for Scenario 1 would be installed prior to Step 1 operations.

- 2. Focus on depth interval from -145 to -270 feet of elevation. While continuing injection and extraction in the E-zone, steam would also be injected into the D-zone, and electrical heating of the D-E aquitard would be discontinued. The extraction well system would be expanded to include areal coverage in the C-zone of the MPA and the N-MPA. Selected hotspot locations in the A- and B-zone aquifers may be used for liquid extraction. The affected volume of material in Step 2 is estimated to be 536,000 yd^3 , or about 32 percent of the Scenario 1 volume. The necessary steam demand was calculated assuming that the added target volume would be heated in a period of 360 days, followed by 2 years of continued operation at a reduced average rate. Since only the D-, D-E-, D-, C-D-, and C-zones would be affected by heating, the necessary extraction rate would be limited to 150 percent of the equivalent steam injection rate (estimated at 27 MM BTU/hr or 54 gpm) plus the pumping rate necessary to maintain hydraulic control in the C-, D-, and E-zone aquifers (73 gpm). The resulting design rate for the effluent treatment system would be 150 gpm, and similarly the vapor extraction rate was estimated at 100 scfm. Thus, there is no change in the steam generation or effluent treatment system between Steps 1 and 2.
- 3. Focus on the depth interval from -40 to -270 feet of elevation, while reducing the treatment intensity in the E-zone. While continuing operation in the D- and E-zones at reduced rates, steam would be injected into the C-zone. The extraction well system would be expanded to include areal coverage in the B-, B-A-, and A-zones of the MPA and the CPA. No additional wells would be added in the N-MPA, since the target elevation for Scenario 1 would be deeper than -140 feet. The affected volume of material in Step 3 is estimated at $1,234,000 \text{ yd}^3$, or about 80 percent of the Scenario 1 volume. However, since heating would be slowed in the deeper zones, the volume being treated in Step 3 would be about $937,870 \text{ yd}^3$. The necessary steam demand was calculated assuming that the added target volume would be heated in a period of 360 days, followed by 2 years of continued operation at a reduced average rate. The necessary extraction rate would be limited to 150 percent of the equivalent steam injection rate (estimated at 50 MM BTU/hr or 100 gpm) plus the pumping rate necessary to maintain hydraulic control in the B-, C-, D-, and E-zone aquifers (117 gpm). The resulting design rate for the effluent treatment system would be 350 gpm, and similarly the vapor extraction rate was estimated at 350 scfm. So a treatment system upgrade would be necessary between years 6 and 7 of thermal treatment.

4. The target volume would be identical to that of Scenario 1, but the thermal treatment intensity in the C-, D-, and E-zones would be negligible. If deemed acceptable, steam injection may be discontinued in the D- and E-zones. The volume undergoing active treatment during Step 4 is estimated at 710,000 yd³. The necessary steam demand was calculated assuming that the added target volume would be heated in a period of 360 days, followed by 2 years of continued operation at a reduced average rate. The necessary extraction rate would be limited to 150 percent of the equivalent steam injection rate (estimated at 41 MM BTU/hr or 82 gpm) plus the pumping rate necessary to maintain hydraulic control in all five aquifers (235 gpm). The resulting design rate for the effluent treatment system would be 350 gpm, and similarly the vapor extraction rate was estimated at 700 scfm. The only change in the effluent treatment system would be the addition of a 350-scfm vacuum extraction capacity.

The alternative scenario would extend the total project time as follows:

- Scenario 1 (base scenario) is based on 12 years of work to carry out design, construction, and active thermal treatment (including cool-down).
- The Phasing Alternative has a 19-year duration (7 years added to the base scenario).

Tables 7-2 through 7-8 provide the calculated operating parameters for the Phasing Alternative of Scenario 1. In order to simplify on-site work, we have selected the same steam delivery and extraction rates for Steps 1 and 2 (25 MM BTU/hr), and for Steps 3 and 4 (45 MM BTU/hr). This way, only one steam delivery system expansion is necessary (this will be before the beginning of the 7th year of thermal treatment).

Table 7-2 provides the treatment depths and areas for each step, the duration of each phase, and the volume and total energy demands calculated for each step. Electrical heating occurs only during Step 1. Each step is projected to last 3 years, so the total thermal treatment duration is 12 years, followed by 3 years of cool-down, identical to the design for Scenario 1.

Table 7-3 provides the data and calculation used to determine the treatment volume for each step. It also shows two different estimates of the steam demand for each step and the resulting recommendation for steam delivery. The recommendation is 25 MM BTU/hr for Steps 1 and 2, and 45 MM BTU/hr for Steps 3 and 4.

Table 7-4 summarizes the design parameters for which more detail is provided in Tables 7-5, 7-6, and 7-7. The steam delivery system and treatment system are unchanged for Steps 1 and 2 (the first 6 years of thermal treatment). The number of wells to be installed before onset of each step is also listed. The total number of wells for each step are as follows:

- Step 1: 50 wells including D-E electrodes and all VEAs (to cover the treatment volume)
- Step 2: 154 wells (104 added during Step 2 construction)
- Step 3: 263 wells (109 added during Step 3 construction)
- Step 4: 383 wells (120 added during Step 4 construction)

This provides for stretching of the drilling and well installation, so the added wells can be installed within the last 2 years of the previous operations step.

Table 7-5 provides the data used to estimate the liquid extraction rates and thereby the liquid treatment system capacity. The pump-and-treat rates are included only for those aquifer layers being treated in each step. For instance, Step 1 involves injection of steam into the E-zone and extraction from both the D- and E-zones. Thus, the necessary hydraulic control pumping rates (from the ICF Kaiser Alternative 4) for the D- and E-zones were added to the net extraction rate calculated for extracting 150 percent of the rate of water injected as steam. The resulting rates are 150 gpm for Steps 1 and 2, and 350 gpm for Steps 3 and 4. Thus, only one treatment system expansion is necessary (before year 7 of thermal treatment).

Table 7-6 provides the calculation results for estimating the vapor extraction rates. For the deep treatment in Steps 1 and 2, no atmospheric air is extracted, and the vapor extraction rate is equal to the air injection rate plus a contingency factor. For Steps 3 and 4, wells will span the vadose zone, and some atmospheric air will be extracted. The vapor rates increase to 350 and 700 scfm for Steps 3 and 4, respectively. The rate does not reach the 1,000 scfm designed for Scenario 1, since the steam injection rates (and thus the rate of air injection) are lower for the phased approach.

Table 7-7 provides the design parameters for the effluent treatment system for each of the four steps. All system components are substantially smaller than these for Scenario 1 treatment in 4 years. The installations and upgrades that will be necessary are as follows:

• Step 1: Install small system with 150-gpm liquid capacity and 100-scfm vapor capacity.

- Step 2: No upgrades.
- Step 3: Upgrade liquid capacity to 350 gpm and vapor capacity to 350 scfm.
- Step 4: Upgrade vapor capacity to 700 scfm.

For the following cool-down, the Step 4 treatment system capacity is sufficient.

Table 7-8 indicates the utility demands for each of the four steps. All the demands are substantially lower than those for Scenario 1. However, when the total usage is integrated over the 12 years of thermal treatment, the overall consumption of water, power, and gas is slightly higher than that for Scenario 1.

7.5.3 Cost Implications of Phasing Example

The major cost implications as compared to Scenario 1 are as follows:

- The drilling and well installation would be stretched out into four phases occurring during the following years of operation:
 - Years 3 to 4 (Step 1),
 - Year 7 (Step 2),
 - Years 9 to 10 (Step 3), and
 - Years 12 to 13 (Step 4).
- This reduces the present value of the drilling and construction costs.
- The size of the steam generation system would be reduced to one-fourth of the Scenario 1 system for Steps 1 and 2 (the first 6 years of thermal treatment), and to 45 percent of the Scenario 1 system for the last 6 years (Step 3 and 4). These systems would be substantially smaller and cheaper than the Scenario 1 system. The natural gas supply, the water supply, and the fuel consumption rates would be correspondingly lower.
- The fresh water supply rate would be reduced from 200 to 60 gpm (Steps 1 and 2) and 100 gpm (Steps 3 and 4).

- The yearly power demand would be reduced substantially (by 50 to 70 percent). However, the overall power usage would be somewhat higher than that for Scenario 1 because of the longer operations time and the continuous vacuum applied.
- The effluent treatment system size would be reduced substantially from a 475-gpm liquid capacity system to 150 gpm for Steps 1 and 2, and 350 gpm for Steps 3 and 4. The small system used for the initial 6 years (30 percent of the Scenario 1 capacity) would result in substantial savings on the initial construction phase.
- By using the step approach from deep to shallow, it is possible that the scope of Steps 3 and 4 drilling and well installation may be reduced, as steam migrates upward and leads to remediation in the zones above. Such savings would be realized using a combination of the ERT and temperature monitoring, observation of groundwater COC concentrations during extraction, and limited interim drillback activities.
- The phased approach would prolong the thermal treatment period from 4 to 12 years at a lower intensity, resulting in more labor demand overall.

More detail on the cost implications is provided in the cost estimate (USEPA 2001).

Visalia data used for scaling				
Liquid pumping rate	400	gpm		
First 6 months average	23,333	lbs/month		
Second year average	16,667	lbs/month		
Third year average	8,333	lbs/month		
Pump-and treat usage	1,667	lbs/month		
McCormick-Baxter estimates	Scenario 1	Scenario 2	Scenario 3	
Pumping rate	475	575	700	gpm
Montly usage 1st yr	27,708	33,542	40,833	lbs/month
Montly usage 2nd yr	23,750	28,750	35,000	lbs/month
Montly usage 3rd yr	14,844	17,969	21,875	lbs/month
Montly usage 4th yr	9,896	11,979	14,583	lbs/month
Usage during cooldown yr 1	5,938	7,188	8,750	lbs/month
Usage during cooldown yr 2	3,958	4,792	5,833	lbs/month
Usage during cooldown yr 3	1,979	2,396	2,917	lbs/month
Estimated total, sacrificial	1,056,875	1,279,375	1,557,500	lbs
	, ,	, ,		
Changeout frequency, 10,000 lb primary v	essels (times	per vear)		
Year 1	33	40	49	times
Year 2	29	35	42	times
Year 3	18	22	26	times
Year 4	12	14	18	times
Year 5	7	9	11	times
Year 6	5	6	7	times
Year 7	2	3	4	times
Estimated regeneration frequency (times	per month) ¹			
Year 1	6	7	8	times
Year 2	5	6	7	times
Year 3	3	4	4	times
Year 4	2	2	3	times
Year 5	1.2	1.4	1.8	times
Year 6	0.8	1.0	1.2	times
Year 7	0.4	0.5	0.6	times
Estimated carbon usage using regenerati	on on-site ²			
Year 1	33 250	40 250	49 000	lbs
Year 2	28,500	34,500	42.000	lbs
Year 3	17,813	21,563	26,250	lbs
Year 4	11 875	14 375	17 500	lbs
Year 5	7 125	8 625	10 500	lbs
Year 6	4 750	5 750	7 000	lbs
Year 7	2 375	2 875	3 500	lbs
	2,010	2,070	0,000	.~~~
Estimated total regeneration on-site	105 688	127 938	155 750	lbs
	100,000	121,000	100,700	
1) Assumed that the vessels are regenerated	when they are	e 50% loaded		
	<u> </u>			

 Table 7-1

 Estimates of GAC Consumption by Liquid Stream Treatment

 Table 7-2

 Definition of Zones, Depths, Volumes, and Energy Demand for Phasing Alternative of Scenario 1

	Scenario 1	Step 1	Step 2	Step 3	Step 4	Unit	Comment
Thermal treatment depth	0-260	190-260	145-260	40-260	0-260		
Area affected	MPA,CPA,N-MPA	MPA,N-MPA	MPA,N-MPA	MPA,CPA	MPA,CPA		
Steam into aquifers	A,B,C,D,E	E	D,E	C,D,E	A,B,C,D,E		
Extraction in aquifers	A,B,C,D,E	D,E	C,D,E	A,B,C,D,E	A,B,C,D,E		Plus aquitards between aquifers
Duration of steam injection	4	3	3	3	3		
Duration of ERH	2	3	0	0	0		
Total volume heated	1,541,900	296,287	535,965	1,234,157	1,541,900	yd ³	
Total soil mass	2.29E+09	4.39E+08	7.95E+08	1.83E+09	2.29E+09	kg	
Total BTU need w/losses	2.61E+12	5.02E+11	9.08E+11	2.09E+12	2.61E+12	BTU	
Heat-up BTU need	4.04E+11	7.76E+10	1.40E+11	3.23E+11	4.04E+11	BTU	

Zone/area	Area (ft ²)	Top (ft)	Btm (ft)	Depth (ft)	Volume (yd ³)	Scenario 1	Step 1	Step 2	Step 3	Step 4	
MPA	17,695	0	-120	120	78,651	78,651	0	0	78,651	78,651	yd ³
	28,419	0	-180	180	189,475	189,475	0	36,842	147,370	189,475	yd ³
	74,247	0	-260	260	715,028	715,028	192,508	316,263	605,024	715,028	yd ³
	19,398	-25	-260	235	168,848	168,848	50,295	82,628	158,070	168,848	yd
СРА	97,739	0	-80	80	289,620	289,620	0	0	144,810	289,620	yd ³
N-MPA	11,097	-30	-120	90	36,993	0	0	0	0	0	yd ³
	9,538	-30	-180	150	52,993	12,365	0	12,365	12,365	12,365	yd ³
	20,628	-30	-260	230	175,734	87,867	53,484	87,867	87,867	87,867	yd ³
Total volume					1,707,343	1,541,854	296,287	535,965	1,234,157	1,541,854	yd ³
Volume unde	r active hea	ating					296,287	535,965	937,870	709,603	yd ³
Projected ste	am injectior	n demand b	ased on vo	lume			20	36	63	48	MM BTU/hr
Estimated ste	eam injectio	n rate base	d on heatup	calculatior	IS		23	27	50	41	MM BTU/hr
Design steam	n injection ra	ate for step					25	25	45	45	MM BTU/hr

 Table 7-3

 Estimation of Bulk Volume and Sand Fractions in Priority Treatment Zones
	Volume treated	Steam rate	Liquid extraction rate	Vapor extraction rate	Injection wells	Extraction wells	Monitoring and NAPL wells
	(yd3)	(lbs/hr)	(gpm)	(scfm)	(number)	(number)	(number)
Step 1	296,000	25,000	150	100	10	29	11
Step 2	536,000	25,000	150	100	45	88	21
Step 3	1,235,000	45,000	350	350	58	170	35
Step 4	1,540,000	45,000	350	700	165	170	48

Table 7-4Summary of Design Parameters for the Four Steps

Table 7-5 Summary of Liquid Extraction Rate Calculations for Phasing Alternative of Scenario 1

	Scenario 1	Step 1	Step 2	Step 3	Step 4	
Steam demand, 180 d heat-up	104	25	25	45	45	MM BTU/hr
Equivalent water injection rate, 180 d heat-up	187	46	54	100	82	gpm
Minimal water extraction rate, 150% of injected rate	280	75	75	135	135	gpm
Minimal net extraction rate from ICF Kaiser Alternative 4	235	51	73	117	235	gpm
Total liquid extraction rate needed	422	126	148	252	370	gpm
Estimated miscellaneous water usage	40	10	20	30	40	gpm
Total effluent treatment system size	462	136	168	282	410	gpm
Design rate	475	150	150	350	350	gpm

	Scenario 1	Step 1	Step 2	Step 3	Step 4	Unit
HPO co-injection	1	1	1	1	1	% air/steam
Steam rate	100,000	25,000	25,000	45,000	45,000	lbs/hr
Air injection rate	1,000	250	250	450	450	lbs/hr
Air injection rate	240	60	60	108	108	scfm
Minimium extraction rate	360	90	90	162	162	scfm
Area	268	0	0	100	268	1000 ft ²
Estimated infiltration rate	2	2	2	2	2	scfm/1000ft ²
Infiltration rate	536	0	0	200	536	scfm
Total air extraction rate	896	90	90	362	698	scfm
Design rate	1,000	100	100	350	700	scfm

Table 7-6 Sizing of Vapor Extraction System for Phasing Alternative of Scenario 1

Table 7-7 Sizes and Capacities of Major Effluent Treatment System Components for Phasing Alternative of Scenario 1

Component	Parameter	Scenario 1	Step 1	Step 2	Step 3	Step 4	Unit	Comment/assumption
Vapor phase liquid separator 1	Air flow rate	4,733	534	611	1,280	1,474	scfm air	Non-condensable gas plus 10% of injected steam as condensate
	Liquid flow rate	22	8	8	18	18	gpm condensate	Assuming max 5% of liquid extracted with vapor stream
Liquid phase heat exchanger	Liquid flow rate	435	150	150	350	350	gpm water	Maximum rate
	Cooling capacity	22	7	7	17	17	MM BTU/hr	Assumed 100 F cooling
Vapor phase heat exchanger/condensor	Cooling capacity	12	3	3	6	5	MM BTU/hr	0% of injected energy as steam and energy in 200F condensate
Vapor phase liquid separator 2	Air flow rate	1,000	100	100	350	700	scfm air	
	Liquid flow rate	23	6	7	12	11	gpm condensate	Based on maximum vapor phase cooling capacity
Vacuum pumps	Air flow rate	1,000	100	100	350	700	scfm	
	Vacuum	-10	-10	-10	-10	-10	psig	Need to apply vacuum
Vapor treatment units	Air flow rate	1,000	100	100	350	700	scfm	
Water surge tank	Retention time	1	1	1	1	1	hours	Need settling and separation time for solids and NAPL
	Minimum capacity	26,100	9,000	9,000	21,000	21,000	gallons	
Dissolved Air Floatation	Water flow rate	435	150	150	350	350	gpm	
	NAPL flow rate	11	4	4	9	9	gpm	Assumed max NAPL ratio of 2.5% of total water flow
Multimedia filters	Water flow rate	475	170	170	350	350	gpm	DAF flow rates plus process water
Water treatment unit (GAC)	Water flow rate	475	170	170	350	350	gpm	DAF flow rates plus process water
NAPL holding tanks	Capacity	21,000	21,000	21,000	21,000	21,000	gallons	Tanks added and emptied for disposal as needed
	Minimum number	2	2	2	2	2	#	Empty backup tank on-site

	Scenario 1	Step 1	Step 2	Step 3	Step 4	Unit	Comment
Fresh water demand	200	50	60	100	85	gpm	
ERH power	2,550	1,000	0	0	0	kW	DE ERH in Step 1
Process power	1,000	500	500	700	700	kW	Increases with vacuum extraction rate
Power demand	3,550	1,500	500	700	700	kW	
Gas firing rate	152,381	33,956	39,971	72,757	60,538	scfh	80% firing rate in average

Table 7-8Utility Demand for Phasing Alternative of Scenario 1

8.0 REFERENCES

- Atwater, B.F. 1982. Geological Maps of the Sacramento-San Joaquin Delta, California. U.S. Geological Survey (USGS). Miscellaneous Files Map MF-1401, Sheet 17.
- Bailey, M.M. 2001. Personal communication re: Summary of Sand Fractions in Distinct Depth Zones, McCormick and Baxter Superfund Site. Table.
 - ——. 2000. Personal communication re: Summary of Hydraulic Conductivity Data, McCormick and Baxter Superfund Site. Table.
- California Division of Mines and Geology (CDMG). 1990. Geologic Map of the San Francisco—San Jose Quadrangle. Map No. 5A. Compiled by D.L. Wagner, E.J. Bortugno, and R.D. McJunkin.
- California Regional Water Quality Control Board, Central Valley Region. 1998. The Water Quality Control Plan (Basin Plan) for the California Regional Water Quality Control Board, Central Valley Region, the Sacramento River Basin and the San Joaquin River Basin. 4th ed.
- Department of Water Resources, State of California (DWR). 1967. San Joaquin Valley Ground Water Investigation. July 1967.
- Heron, T., G. Heron, and K.S. Udell. 2000. "Tools for Designing Steam Enhanced Remediation Systems." Presentation at the May 2000 Battelle Conference on Chlorinated and Recalcitrant Compounds, Monterey, California. May 2000.
- Normann, R., J. Weiss, and J. Krumhansl. 2001. "Development of Fibers Optic Cables for Permanent Geothermal Wellbore Deployment." *Proceedings of the Twenty-Sixth Workshop on Geothermal Reservoir Engineering, Stanford University.*

Southern California Edison. 2000. Full-Scale Remediation of Visalia Pole Yard.

- U.S. Army Corps of Engineers (USACE). 2001a. Draft Sampling and Analysis Plan for Groundwater Monitoring Program, McCormick and Baxter Site, Stockton, California. July 2001.
- ———. 2001b. Draft 95% Design Analysis, Thermal Remediation Pilot Study, Soil and Groundwater Units PN C1871, Wyckoff/Eagle Harbor Superfund Site, Bainbridge Island, Washington. February 28, 2001.

- ——. 2001c. 2000 NAPL Field Investigation Report. McCormick and Baxter Superfund Site, Stockton, California. Prepared by USACE and URS Corporation. May 2001.
- ——. 2000a. 1999 NAPL Field Investigation Report, McCormick and Baxter Superfund Site, Stockton, California. June 2000.
 - —. 2000b. Site Characterization and Penetrometer System (SCAPS), Supplemental Field Investigation at McCormick and Baxter Superfund Site. Draft. USACE Tulsa District. November 2000.

—. 2000c. Interim Design Analysis, McCormick & Baxter Superfund Site, Surface Water Operable Unit, Sediment Cap. February 2000.

—. 1999. Draft Site Characterization and Analysis Penetrometer System (SCAPS) Field Investigation at the McCormick and Baxter Superfund Site, Stockton, California. USACE Tulsa District. October.

U.S. Environmental Protection Agency (USEPA). 2001. Thermal Treatment Technology Conceptual Design Cost Estimate, McCormick and Baxter Superfund Site, Stockton, California. 2 vols. Prepared for EPA Region 9, San Francisco, California, by U.S. Army Corps of Engineers, Seattle District, and URS Corporation, Seattle, Washington. November 12, 2001.

-. 1999. Record of Decision, McCormick and Baxter Superfund Site, Stockton, California. EPA Region 9, March 31, 1999.

———. 1998. Soil and Groundwater Remedial Investigation Report, McCormick and Baxter Superfund Site, Stockton, California. 2 vols. Prepared by ICF Kaiser Engineering, Inc. July 1998.

APPENDIX A

Evaluation of Heavy Metals Treatment Need

EVALUATION OF HEAVY METALS TREATMENT NEED

The premise for evaluating the need for treating the effluent water for heavy metals consists of the following:

- The treated water is re-injected into the E-zone through vertical wells.
- The concentrations of chemicals of concern (COCs) may not substantially exceed the concentrations currently in the E-zone.
- The 1997 groundwater data are valid for assessment of the heavy metals concentrations in all aquifer zones from A through E.

EVALUATION PROCESS

The evaluation consisted of the following steps:

- 1. Average concentrations of all heavy metals were calculated for each aquifer based on the tabulated 1997 groundwater samples in the remedial investigation (Table A-1, data from USEPA 1998).
- 2. Average pumping rate for each aquifer zone was estimated based on the steam injection and extraction rates predicted during heat-up, as follows (Scenario 3 is used for illustration, Table A-2):
 - Zone A: 66 gpm
 - Zone B: 107 gpm
 - Zone C: 159 gpm
 - Zone D: 64 gpm
 - Zone E: 183 gpm
- 3. The influent concentration to the treatment system was calculated by weighting all the concentrations with the appropriate flow rate. In this case, values from E-zone data were weighted high (since 183 gpm is a large fraction of the total flow rate), and values from the A-zone were weighted low (since 66 gpm is a small fraction of the total). Table A-2 provides the calculations.

- 4. A heavy metals concentration reduction by a factor of 2 was assumed for the effluent treatment system, based on the fact that solids and nonaqueous-phase liquids (NAPL) will be removed and that a substantial fraction of the heavy metals will be associated with those fractions.
- 5. A dilution factor of 2 was used since more than 2 pore volumes of steam condensate will be flushed through the site during thermal operation.
- 6. The heavy metals concentrations expected at the effluent end of the treatment system were estimated based on the average inlet concentrations and the stated reductions, and compared to the current levels in the E-zone aquifer (Table A-3).

For comparison, the estimated pumped concentrations from the EPA pump-and-treat scenario (USEPA 1998) are included in Table A-3.

The results are as follows for the thermal treatment scenarios:

- No treatment will be warranted for the COCs As, Cr, Cu, and Zn.
- Non-COCs that are likely to be injected into the E-zone at concentrations greater than the current levels include Al (estimated to be injected at 2.0 times the current level of 28 μ g/L) and Fe (estimated to be injected at 1.3 times the current level of 326 μ g/L).

In conclusion, it is likely that heavy metals treatment will not be necessary during thermal treatment. A more detailed evaluation may be conducted at a later design stage, if deemed necessary.

REFERENCE

U.S. Environmental Protection Agency (USEPA). 1998. Soil and Groundwater Remedial Investigation Report, McCormick and Baxter Superfund Site, Stockton, California. Prepared by ICF Kaiser Engineering, Inc. July 1998.

Appendix A 11/12/01 Page A-3

	A		В	С	D	E	
As		19.1	21.5	44.4	36.4	49.5	averages
ua/L		41	25.4	69	31	62	U
· J		6	19.1	31	37	44 4	
		3	3	33.2	41 2	42	
		25	24	00.2			
		20	10				
		33.0	19				
		6	32				
-			28				
	A		В	С	D	E	
Cr		7.15	<3	<3	<3	<3	averages
		3	2.9	3	3	3	
		21	3	3	3	3	
		10	9	2.9	2.9	3	
		3	3				
		29	3				
		3	U				
		5					
	^		B	C	n	E	
<u></u>	A		U 2	C		L	0.4070.00
u	<3		S 3	S 3	S	S	average
			-	2	-	-	
_	A		В	C	D	E	
Zn		393.0	34.2	99.7	31.7	41.7	average
		42	45.45	208	12	37	
		28	37	44	57	48	
		21	17	47	26	40	
		7	39				
		1090	42				
		1170	25				
AI		1022.3	88.6	313.0	<40	<25	averages
		332	78	1160		-	
		4980	28	23			
		736	41	30			
		40	72	30			
		-+0	170				
		23	170				
		23	208				
			23				
_							
Fe		5037.0	1110.4	2993.3	447.5	326.0	averages
		11800	1410	9190	232	352	
		12400	752	692	609	234	
		2790	1180	1590	575	392	
		407	716	501	374		
		505	985				
		2320	1970				
			760				
<u>.</u>							
Mn		464 5	186.0	238.8	373.8	196 7	averages
		1050	100.0	308	222	307	arolugos
		1030	100	100	£222 510	100	
		401	190	190	519	128	
		147	239	192	520	155	
		234	163	175	234		
		308	159				
		647	205				
			147				

Table A-1 Heavy Metals Data From August 1997 Sampling Event

Table A-2 Weighting of Heavy Metals Concentration by Expected Water Extraction Rate From Each Aquifer (Scenario 3 Rate Used)

As	Ava (ua/L)	apm	Average in total flow stream	
A	19.1	66	38.0	
В	21.5	107		
С	44.4	159		
D	36.4	64		
E	49.5	183		
C(avg)/C(E	zone)	0.77		
Zn	Avg (ug/L)	gpm	Average in total flow stream	
А	393.0	66	95.2	
В	34.2	107		
С	99.7	159		
D	31.7	64		
E	41.7	183		
C(avg)/C(E	zone)	2.28		
Cu and Cr	below 3 ug/	Ĺ		
ΔΙ	Ava (ua/L)	apm	Average in total flow stream	
A	1022.3	66	218.9	
В	88.6	107		
С	313.0	159		
D	0.0	64		
E	0.0	183		
Fe	Avg (ug/L)	gpm	Average in total flow stream	
Α	5037.0	66	1753.9	
В	1110.4	107		
С	2993.3	159		
D	447.5	64		
E	326.0	183		
Mn	Avg (ug/L)	gpm	Average in total flow stream	
A	464.5	66	256.4	
В	186.0	107		
С	238.8	159		
D	373.8	64		
	106 7	183		

Table A-3 Summary of Heavy Metals Concentrations Predictions in Effluent Water Streams at McCormick and Baxter (all concentrations are µg/L)

	COC metal	s			Non-COC	Non-COC metals ⁴		
Source	As	Cr	Cu	Zn	AI	Fe	Mn	
ICF Kaiser Pump-and-Treat Alt 4 ¹	39	12	15	1066	828	6396	1032	
August 1997 COC levels weighted with Scenario 3 extraction	38	<3	<3	95	219	1754	256	
Average concentration in E zone, August 1997	49	<3	<3	42	28	326	197	
Estimated removal efficiency in DAF and multi-media filters	50%	50%	50%	50%	50%	50%	50%	
Estimated dilution factor due to steam flushing	2	2	2	3 95 219 1754 3 42 28 326 50% 50% 50% 50% 2 2 2 2 NA 0.4 2.0 1.3 no no ² yes/no ³ yes/no ³ 1.100 ug/L)	2			
Needed concentration reduction for E zone re-injection	0.1	NA	NA	0.4	2.0	1.3	0.3	
Treatment needed	no	no	no	no ²	yes/no ³	yes/no ³	nc	
 2) The preliminary recommendation of no Zn treatment needs 3) Treatment not needed since Al and Fe are not on the COC I 4) Only the ones that would be critical based on ICF Kaiser P& 	to be investig ist T concentrati	ated at a la on estimate	ter design s es are show	n				
Metals sceened to be well below the E zone background:	Sb							
	Ba							
	Be							
	Ca							
	Ca							
	C0 Ph							
	Ma							
	Ha							
	Ni							
	K							
	Se							
	Ag							
	Na							
	TI							
	V							

APPENDIX B

Steam Injection Rate Modeling

STEAM INJECTION RATE MODELING

This appendix documents the choice of well spacing and heat-up rate for the thermal treatment at the McCormick and Baxter site. The intervals between the steam injection wells were chosen based on the following criteria:

- For thermal treatment of the chosen areas, steam will be injected in all of the depth intervals representing the treatment depths, except the upper A-zone in some locations where the A- and B-zone sands are sufficiently connected to allow for steam migration upward from the B-zone into the A-zone.
- Where a zone has a good definition of the outer bounds of the NAPL area, steam injection wells were placed outside the target zones, allowing for an outside-in steam migration.
- For zones in which nonaqueous-phase liquid (NAPL) is confirmed or suspected to be present outside the thermal treatment zone, the steam injection intervals are typically surrounded with extraction wells equipped with both liquid and vapor extraction capability.
- For thick sand zones that need steam flow in the bottom section, the injection screens are designed so they preferentially inject steam in the lower half of the aquifer interval. This is done by using short injection screens, typically screened from the middle of the aquifer to several feet into the underlying silt layer/aquitard.
- Where a sand zone is overlain by a thick silt/aquitard (such as the D-E aquitard in large areas of the site), it is desirable to allow steam injection in the bottom half of the aquitard into sand lenses that may be present. Often, it is not known where such lenses are, but the likelihood of sand sections is recognized. In such cases, steam injection wells in the deeper zone are completed with the top of the screened interval located in the overlying aquitard. As an example, E-zone steam wells may be screened from about –200 feet of elevation (the middle of the D-E aquitard) to the bottom of the E-zone aquifer. This will allow steam heat-up of any sand lenses that allow steam penetration in the lower D-E aquitard, without preventing sufficient heating in the E-zone, since the permeability in the E-zone aquifer is orders of magnitude higher than that of the D-E aquitard.
- For thinner aquifers in which the achievable steam injection rate may be limiting heat-up and performance, the injectors are screened across the entire aquifer, but

not in the overlying aquitard, allowing for a higher injection pressure (due to a greater depth to the top of the screen) and the maximum achievable transmissivity of the aquifer interval.

The steam injection rates are critical for estimating the size of the steam generation equipment, for setting the well spacing, and for predicting the duration of thermal treatment. The procedure used to estimate the rates was as follows:

- The A through E zonation for the dominating aquifers was adapted, with assumption of average depths for each zone as provided by Mike Bailey of the U.S. Army Corps of Engineers (USACE) in a summary table (Bailey 2001).
- The sand fraction in each zone was estimated based on Bailey (2001) and used to make a rough assumption of the steam zone thickness during the initial steam migration and heat-up. Typically, it was assumed that the steam zone filled 50 percent of the average aquifer thickness, with a condensate zone surrounding the steam zone.
- Four-inch injection well screens in 10-inch boreholes were assumed, and the maximum injection pressures were defined as 0.5 psi per foot measured from the surface to the top of the injection screen.
- Permeability averages from Bailey (2000, based on data contained in USEPA 1998) were used for each aquifer zone. Three calculations were made for each depth interval, one in which the average conductivity was used, one in which onethird of the average was used, and one in which three times the average was used. This allows for a simple evaluation of how local heterogeneity may affect the steam injection rates across the site, assuming that the same injection pressure will be applied. Permeability values are provided in Table B-1.
- The simulations are simple radial, cylindrical calculations based on a numerical solution for one steam injection well, as described in Heron, Heron, and Udell (2000) and USACE (2000).

Table B-1 presents the achievable steam injection rates at the maximum allowable injection pressures for the scenarios described above. Table B-2 presents a typical set of input parameters.

Figures B-1 and B-2 show example model output used to construct Table B-1. Sixteen such model simulations were performed.

Average steam injection rates per well in each of the zones after about 180 days of injection were estimated as follows:

- Zone A: 900 lb/hr
- Zone B: 2,400 lb/hr
- Zone C: 4,800 lb/hr
- Zone D: 2,000 lb/hr
- Zone E: 12,800 lb/hr

For the E- and C-zone wells, a reduced injection pressure was used, since the maximum allowable pressure would lead to excessive injection rates (more than 25,000 lbs/hr per well).

For design purposes, the wells will allow for approximately 150 percent of the average injection rate listed above, leading to design injection ranges as follows:

- Zone F: 300 to 1,500 lb/hr
- Zone G: 800 to 4,000 lb/hr
- Zone H: 1,600 to 6,000 lb/hr
- Zone I: 600 to 3,000 lb/hr
- Zone J: 4,000 to 16,000 lb/hr

For the E-zone injection wells, the need to exceed the estimated injection rate is less important than it is for the upper zones, since the permeability is sufficiently high that all injection wells are expected to allow for injection at the design rate.

The radii of the steam zones are calculated simultaneously with the injection rates. For design purposes, the most important parameter is the optimal distance between injection and extraction wells for each depth zone. For this purpose, the time needed for steam breakthrough to the nearest extraction well was estimated by making the steam zone radius equal to the well spacing.

The criteria used to choose the well spacing were the following:

- The well spacing cannot significantly exceed the predicted radius of influence after 90 days of steam injection for the simulation using the average hydraulic conductivity values for that particular depth interval.
- The well spacing will allow for steam breakthrough within 360 days in the scenario using one-third of the average hydraulic conductivity.

Based on the results in Table B-1, the following maximum well spacings were achieved for each zone:

- Zone A: 52 feet
- Zone B: 83 feet
- Zone C: 112 feet
- Zone D: 69 feet
- Zone E: >300 feet

For practical purposes, it is desirable to group the depth zones so only two or three different well spacings are used. This allows for a logical well-field layout and minimizes the area occupied by wells and the piping from the wells to the steam and treatment systems, and it allows for access to the individual wells during operation. In addition, the E-zone injectors were predicted to allow for almost unlimited steam injection rates. For that purpose, a set of simulations was run with a lower injection pressure (840 kPa compared to the maximum allowable 928 kPa). This simulation showed that at injection rates of about 12,500 lb/hr per well, steam will break through to a well 180 feet away within 100 days of injection. Thus, the following well separations were chosen:

- Zone A: 60 feet
- Zone B: 60 feet
- Zone C: 120 feet
- Zone D: 60 feet
- Zone E: 180 feet

For Zone A, the well spacing is slightly greater than the 52 feet calculated above, but in this case we expect a positive effect by upward steam migration from the underlying zones. In conclusion, the 60-foot separation seems to be a good compromise.

The breakthrough times were estimated for each zone as follows, using the average hydraulic conductivity values:

- Zone A: 116 days (60-foot separation)
- Zone B: 30 days (60-foot separation)
- Zone C: 88 days (120-foot separation)
- Zone D: 44 days (60-foot separation)
- Zone E: 105 days (180-foot separation)

These times are for constant injection at the design pressures and may, therefore, be seen as reasonable predictions of the earliest steam breakthrough in areas where the hydraulic

conductivity value is similar to the average value given by Bailey (2000). Due to less than 100 percent operation time for the steam injection, temporary pressure reductions for various reasons, less than average hydraulic conductivity values in many areas, and the intended reduction of steam injection rates in some wells to allow for a more uniform steam distribution, more realistic steam breakthrough times are listed as follows:

- Zone A: 100 to 180 days
- Zone B: 30 to 60 days
- Zone C: 90 to 180 days
- Zone D: 40 to 80 days
- Zone E: 100 to 180 days

During operations, the steam migration in certain zones may be reduced in order to control the direction of vertical pressure gradients, to minimize the risk of downward NAPL migration, and to achieve uniform heating with a minimal fuel demand during the heat-up phase. For design purposes and for sizing the steam supply and effluent treatment systems, the overall steam injection rate and pumping rates were fitted for a heat-up time of 180 days as an average across the site. This is a reasonable compromise among the following factors:

- The desire to heat and remediate the site rapidly (to shorten the overall operations time)
- The need to minimize the number of wells (to reduce drilling and hardware/instrumentation cost)
- The desire to have steam and treatment system sizes in practical and economic ranges (to minimize the capital cost of equipment)
- An allowance for contingencies in the actual field performance of each depth interval and the performance of each of the wells

In conclusion, the chosen well separation allow controlled heating of the five dominant aquifer zones, with steam breakthrough to extraction wells within 180 days after initiating steam injection. Contingencies were built in for poorer performance in some areas of the site.

REFERENCES

Bailey, M.M. 2001. Personal communication re: Summary of Sand Fractions in Distinct Depth Zones, McCormick and Baxter Superfund Site. Table.

———. 2000. Personal communication re: Summary of Hydraulic Conductivity Data, McCormick and Baxter Superfund Site. Table.

- Heron, T., G. Heron, and K.S. Udell. 2000. "Tools for Designing Steam Enhanced Remediation Systems." Presentation at the May 2000 Battelle Conference on Chlorinated and Recalcitrant Compounds, Monterey, California. May 2000.
- U.S. Army Corps of Engineers (USACE). 2000. 1999 NAPL Field Investigation Report, McCormick and Baxter Superfund Site, Stockton, California. June 2000.
- U.S. Environmental Protection Agency (USEPA). 1998. Soil and Groundwater Remedial Investigation Report, McCormick and Baxter Superfund Site, Stockton, California. 2 vols. Prepared by ICF Kaiser Engineering, Inc. July 1998.

Appendix B 11/12/01 Page B-7



Figure B-1. Example Steam Injection Rate Simulation (A-zone, 8 darcy)

Appendix B 11/12/01 Page B-8



Figure B-2. Example Radius of Influence Simulation (A-zone, 8 darcy)

Table B-1 McCormick Baxter Well Spacing Calculation Based on Hydraulic Conductivity and Aquifer Thicknesses

				Hydr. Cond.						Steam	K sensitivity	range (d	arcy)
	Thickness	Тор	Bottom	ft/day	darcy	TOS (ft)	Pinj (psig)	Pinj (Pa)	T(K)	zone (ft)*	Min	Avg	Max
A	30	15	45	23	8.1	25	12.5	187,439	388	15	2.7	8.1	24
A-B	5	45	50										
В	25	50	75	40	14.1	60	30	308,035	407	12.5	4.7	14	42
B-C	40	75	115										
С	20	115	135	43	15.2	125	62.5	531,997	427	10	5.1	15	46
C-D	10	135	145										
D	20	145	165	12	4.2	155	77.5	635,365	433	10	1.4	4.2	13
D-E	55	165	220										
E	30	230	260	136	48.0	240	120	928,239	448	15	16	48	144
E	30	230	260	136	48.0	240	107.2	840,000	444	15		48	

Injection rates,	30 days (I	bs/hr)	Injection rates	, 90 days ((lbs/hr)	Injection rates,	180 days	(lbs/hr)	Injection rates, 360 days (lbs/hr)		
Min	Avg	Max	Min	Avg	Max	Min	Avg	Max	Min	Avg	Max
354	965	2,620	327	898	2,450	313	861	2,357	301	831	2,280
975	2,660	7,346	910	2,490	6,917	873	2,401	6,680	844	2,325	6,486
1,959	5,300	15,004	1,837	4,995	14,200	1,770	4,826	13,750	1,715	4,685	13,373
790	2,162	6,139	737	2,029	5,788	709	1,956	5,592	685	1,895	5,431
20,030	55,800	156,000	18,875	52,800	148,000	18,230	51,100	144,000	17,700	49,700	140,000
	14,160			13,300			12,860			12,480	

RO steam	, 30 day	/s (m)	RO stear	n, 90 day	/s (m)	RO steam,	180 days	s (m)	RO steam, 360 days (m)		
Min	Avg	Max	Min	Avg	Max	Min	Avg	Max	Min	Avg	Max
6.9	11.4	18.7	10.1	16.7	27.5	12.8	21.2	35.0	15.9	26.3	43.5
11.3	18.5	30.7	16.3	27.0	44.8	20.6	34.0	56.6	25.3	41.9	69.8
15.6	25.6	42.9	22.4	36.8	62.0	28.0	46.1	77.6	34.1	56.3	95.0
9.7	16.0	26.9	13.9	23.0	38.7	17.3	28.7	48.7	21.1	35.0	59.2
41.0	68.2	113.9	60.7	101.3	169.5	77.3	129.2	216.0	96.0	160.7	270.0
	34.9			51.7			65.8			81.8	

Well spacing, 90	day breakth	nru (ft)	Well spacing, 180	day breakt	hru (ft)	Well spacing, 360	day breakt	hru (ft)	Max well spacing	Chosen
Min (ft)	Avg (ft)	Max (ft)	Min (ft)	Avg (ft)	Max (ft)	Min (ft)	Avg (ft)	Max (ft)	recommended	spacing
33.1	54.8	90.2	42.0	69.5	114.8	52.1	86.2	142.6	53.4	60
53.4	88.5	146.9	67.5	111.5	185.6	83.0	137.4	228.9	85.7	60
73.4	120.7	203.3	91.8	151.1	254.4	111.8	184.6	311.5	116.2	120
45.6	75.4	126.9	56.7	94.1	159.7	69.2	114.8	194.1	72.3	60
199.0	332.1	555.7	253.4	423.6	708.2	314.8	526.9	885.2	NA	180
	169.5			215.7			268.2		169.5	

Parameter	Symbol	Unit	Value
Thickness of steamed layer	h	m	4.575
Particle density of soil	ds	kg/m ³	2650
Porosity of steamed layer	р		0.35
Absolute permeability of soil	ks	m ²	8.1E-12
Heat capacity of soil	CPr	J/(kg K)	1152
Initial water saturation	Sw		1
Ambient pressure in aquifer	Pamb	Pa	147851
Density of water	dw	kg/m ³	1000
Ambient water temperature	Tamb	К	295
Heat capacity of water	срѡ	J/(kg K)	4187
Overall heat capacity	CDsw	J/(kg K)	2200
Overburden thermal conductivity	k	J/(s m K)	1.3
Overburden thermal diffusivity	а	m²/s	0.00000018
Radius of injection well	rw	m	0.0505
Maximum steam injection pressure	Pw	Pa	187439
Relative permeability of steam	K rs		1
Steam viscosity	Us	kg/(m s)	1.30E-05
Temperature of steam	Ts	К	391
Delta T	Ts - Tamb	К	96
Heat of condensation	ĥ	J/kg	2666000
Time since start of injection	t	S	
Steam mass flow rate	΄m	kg/s	
Volume of steam zone	V	m ³	
Gas constant	R	J/(mole K)	8.314
Initial radius of steam zone	ľ si	m	0.053025
Mole mass of water	Mw	kg/mole	0.018
Density of steam	dsteam	kg/m ³	1.1
Depth to top of injection screen	D _{injscr.}	m.b.g	7.625
Length of injection screen	L _{inj,screen}	m	4.5
Aquifer hydraulic head	D _{gwt}	m.b.g	3

Table B-2 Example Input Parameters for Steam Injection Simulation

A-zone, 2.7 darcy

FINAL THERMAL TREATMENT TECHNOLOGY CONCEPTUAL DESIGN McCormick and Baxter Superfund Site

mail or (balty)

W:(74206/0110.035)Appendix B tables

Table B-4 njection Rate and Steam Zone Growth Estimation of Steam A-zone, 8.1 darcy

Energy Inputed

(And) without a state of the st

Radus of steam zone (meter)

Table B-5 Estimation of Steam Injection Rate and Steam Zone Growth A-zone, 24 darcy Dongs Ispains 180

Not 3

FINAL THERMAL TREATMENT TECHNOLOGY CONCEPTUAL DESIGN McCormick and Baxter Superfund Site

W:(74206/0110.035)Appendix B tables

Description 1
 Desc

W:(74206/0110.035)Appendix B tables

(Apro) organity (Apro) organity 500 400 200 200 100

(meter)



180 Time in days

								Longy	
**		2448770	,	1944 1913	=()())2 0.000	A0 13	-11 2.0	lipited 1840-10	in (bh) METO2
0.00	1.47		0.01	61.38	0.004	13	2.0	1.002-10	12740
0.00	1.40		0.01	21.40	0.042	13	2.1	20/8×10	12007
0.01	1.68		6.01	8.06	0.004		33	2.328 + 10	12830
0.02	1.00		0.02	6.31	0.077		2.4	2728+10	12343
0.00	1.48		0.04	2.48	0.000	33	33	6.028 - 10	11740
0.18	1.42		0.06	1.65	6.607	12	4.1	7.828 = 10	1081
0.36	1.36		0.08	1.44	6.003	100	63	1.348+11	10080
1.21	1.20		0.18	1.36	0.048	232	84	3.778+11	9427
171	1.18		0.18	1.20	0.624	367	8.8	6.108-11	9126
271	1.12		0.21	128	0.76	447	11.0	7498-11	8728
321	1.08		0.26	1.20	6.771	813	12.8	& RGE = 11	8677
621	1.05		0.27	1.07	6.765	69	15.4	1.08-10	825
471	1.05		0.30	1.16	0.733	700	14.0	1.248+12	8336
621	1.04		6.30	1.16	6.722	750	15.5	1.378 - 12	8208
621	1.00		0.36	1.0	6.708	874	16.7	14/8-12	8'09
671	1.02		0.36	1.0	0.496	830	17.3	1738+10	800
10.64	1.01		0.45	1.06	0.640	1344	20.7	2645-12	ADD
14.08	0.06		6.82	1.06	0.404	1671	23.1	3.428 = 12	7793
20.84	0.87		0.58	1.04	6.6%	1876	26.1	418-12	7984
24.37	0.04		0.69	1.01	6.631	2634	28.4	6.6HE - 12	740
27.81	0.80		0.79	1.00	6.613	2794	20.8	6.428 = 12	740
30.08	0.80		0.76	1.00	0.002	2962	30.7	6912-12	7346
3.8	0.02		0.00		6.65	2442	20.1	A368-12	730
40.38	0.01		0.88	0.06	0.401	3067	36.2	0.0ME = 12	7228
42.81	0.00		0.00		0.480	2000	36.2	0.802-13	7163
10.48	0.90		0.99	0.06	0.430	4310	37.6	1.128 - 13	7121
64.11 AT NA	0.00		1.00	0.06	6.01	4813	27.8	1.186-13	7108
42.88	0.88		1.09	0.05	6.405	4904	38.8	1.338 = 13	7967
66.41	0.88		1.12	0.06	0.367	8084	40.3	1408-13	71286
71.27	0.00		1.00		0.3M	5401	41.7	1045-12	100
76.71	0.88		1.20	0.04	0.378	6639	42.4	14/8+13	6379
16.14	0.88		125	2.04	0.315	5965	64	1782-13	6962
85.01	0.87		1.28	0.00	0.361	4155	46.3	1.828 - 13	6102
88.44	0.87		1.30	0.45	0.386	6321	46.8	1.818-13	6918
96.30	0.87		1.36	0.42	0.346	6645	45.0	2038+13	6891
98.74	0.87		1.38	0.40	0.342	6803	45.5	2.108+13	6875
105.40	0.86		1.43	0.62	0.333	7113	47.6	2238+13	6856
108.05	0.86		1.45	0.42	0.329	7265	48.1	2308+13	6845
116.60	0.86		1.80	6.01	0.322	7963	49.1	2445-13	6824
118.33	0.86		1.62	6.01	0.318	7709	49.5	2.5/8 = 13	6814
120.77	0.86		1.04	6.01	6.316	7853	83.0	2.576+13	6805
128.43	0.86		1.88	6.01	0.308	#134	62.8	278+13	6797
133.06	0.85		140	0.01	0.306	8274	81.3	27WE+13	67%
138.65	0.85		1.45	0.00	0.200	8548	12.2	28/8+13	6702
143.36	0.85		1.47	5.00	0.296	8682	52.6	2.885+13	6754
185.23	0.85		1.71	0.00	0.201	8947	53.4	318-13	4730
183.66	0.85		1.75	0.00	0.288	9077	63.7	3.180+13	6752
167.00	0.85		128	100	0.340	8206	04.1	3200-13	678
143.96	0.85		1.78	0.00	0.381	9400	54.9	3.388+13	670
147.30	0.85		1.80	1.00	0.278	8585	65.2 55.6	348-13	6705
174,26	0.84		1.86	0.00	0.2%	8432	45.9	3.588 = 13	6603
127.40	0.84		1.86	0.00	6.212	9954	86.3	348-13	6607
184.00	0.84		1.80	0.00	0.367	10196	67.0	3788+13	6675
187.00	0.84		1.00	0.00	0.365	10013	67.3	3.800+13	6600
104.85	0.84		1.04	0.00	6.302	10648	67.8	3388-13	6400
188.28	0.84		1.06	0.00	0.360	10643	68.2	4.008 = 13	6603
201.12	0.84		1.00		0.204	10010	55.2	410-13	550
208.68	0.84		2.01	0.00	0.214	11005	09.2	4208-13	6424
210.02	0.84		2.00	0.00	6.302	11229	0.5	43/8+13	6620
218.68	0.84		2.08	0.44	0.249	11338	42.1	448-13	6424
222.31	0.83		2.08	0.00	0.348	11440	40.4	4510-13	40.00
228.18	0.83		2.11	0.00	0.344	11046	42.9	668-13	6610
232.41	0.83		2.12	0.00	6.343	10776	41.2	47 H+13	6456
238.48	0.83		2.18	0.00	0.340	11046	41.8	4847-13	6108
242.01	0.83		2.17	0.00	0.238	12091	62.0	4918-13	6003
248.78	0.83		3 20	0.00	0.236	12300	61.6	604F-13	6185
283.21	0.83		3.20	0.87	0.334	12400	42.8	6.108+13	6190
206.64	0.83		2.25	0.87	6.233	12805	63.1	6.178-13	4074
243.61	0.83		3.26	0.67	6.230	12758	43.4	6308+13	4870
208.04	0.83		2.27	0.07	6.229	12838	65.8	6.54E+13 6.63E+13	6166
273.80	0.83		2.30	0.87	0.326	13006	66.3	6.50E=13	6100
277.34	0.83		3.30	0.87	0.326	13107	66.6	6.54E = 13	6106
284.10	0.83		2.36	0.87	0.223	13302	45.1	64W-13	6549
287.63	0.83		2.38	0.87	0.332	13389	45.3	6.7ME = 13	6545
296.47	0.83		2.38	0.87	6.320	13830	6.1	5.62E-13 5.84E-13	6542
287.83	0.82		2.40	0.87	0.218	CMMP	45.0	6.902 - 13	6106
301.27	0.62		142	0.87	6.317	13782	66.2	602E×13	6100
308.13	0.82		2.44	0.87	0.216	13875	46.7	6.102-13	6126
311.66	0.82		2.48	0.87	0.314	14040	45.9	42/E×13	6123
318.43	0.40		2.48	0.00	630	14240	47.3	4348-13	4807
321.86	0.82		2.80	0.00	0.311	14340	47.5	64/E>13	6014
328.29	0.82		2.01	0.M	6.210	14632	67.4	64/8-13	4011
332.16	0.82		2.64	0.00	0.208	14613	48.2	6408-13	6106
336.00	0.62		2.65	0.00	0.307	14733	68.4	6478-13	6963
347.46	0.62		2.00	0.00	0.306	14840	65.8	6.802-13	6407
345.80	0.82		2.00	0.00	0.304	14875	49.0	6.842 = 13	646
342.76	0.60		240	0.00	0.305	10146	48.3	6.998-13	640
306.10	0.82		240	0.00	0.302	16236	49.4	7.048 - 13	6.87
368.42	0.82		246	0.M	6.201	1021	73.0	7/28-13	6494
366.40	0.82		2.67	0.00	6, 199	15485	79.2	7298+13	6479





 Appendix B 11/12/01 Page B-17 C-zone, 15 darcy

Curpt of Data and Curpt of Dat

W::74206/0110.035/Appendix B tables

Table B-10 Estimation of Steam Injection Rate and Steam Zone Growth												
barby												
re 1.0		*N 60	1 (Keyn) 6.00	-	10.00 3440770	7 000	1444	5pr/1/pla 2002	A0 .			
6 an' 20	1	120	8.00	1.17		001	43.07	0.040				
ad - 192	1	480	881	1.16		001	1144	1.000				
		1120	6.62	1.14		0.03	3.77	6.812				
		7980	6.09	1.00		005	1.76	0.040	28			
n (hg) 20		16240	6.18 6.36	0.00		0.07	1.40	6.823 6.814	30			
an Jago 20 kohby h Jan X 1	11	61640 104640	671	0.82		014	1.05	6496	10			
andy characteristic and characte	13	147840	121	0.00		0.32	0.00	6.790	227			
200000		254240	2.71	0.79		0.28	0.00	6.747	328			
	т 17	320640	3.71	0.77		033	0.00	6716	a1			
T. T are E		407040	421	0.76		638	0.04	0.001	101			
	N 13	04CRA 044CRA	621	0.76 0.74		0.38	0.43	14% 1465	800			
	36 77	576640 579640	621 671	0.74		043	0.01	0.646	670			
6 (José K) 6 20		423040	721	0.73		0.45	0.80	6437	720			
8 alweb 6.00		1216210	14.08	0.71		044	0.76	0.040	1185			
ann the star star star		1808380	20.04	0.00		0.78	0.74	6486	1549			
	10 N	2105965	24.37	0.67		0.85	6.73	64%	194			
	-	2649 (38	30.06	9.47		0.04	0.72	0.445	2010			
		2686/30	33.65	0.67		104	6.71	6417	2020			
		3795475	6.0	0.65		1.13	0.70	0.394	2620			
	10 11	4010000	47.26 80.48	0.65		1.18	6.70 6.69	0.3%	2816 2863			
		4679230	84.11 87.04	0.65		126	100	0.306	30 Mil			
Makehur as survision of time		1248400	62.08	0.64		134	140	0.380	2043			
		6861670	0.84	0.64		141	0.48	6.337	35.66			
		64/54740	74.27	0.64		148	0.48	6.326	3622			
	100	7547910	11.17	0.63		144	147	0.314	60			
	101	7541495	ML-01 ML-04	0.03		141	647	6.308	4107			
	100	7537668	81.87 86.30	0.00		144	647 647	6.300 6.2M	4070			
	108	8030438 8927420	96.74 102.17	0.63		170	147	0.210 0.2M	475			
	107	9124305	106.40	0.02		128	0.46	0.2%	673 673			
	128	6717175	112.47	0.42		1.82	0.46	6.277	-			
The second s	111	10210848	118.33	0.02		147	0.46	6270	8158			
	10	10404830	106.30	0.62		180	0.44	0.267	1010			
	114	11000130	133.04	0.02		185	0.46	0.201	8431			
	116	11763270	12640	0.42		200	0.46	6265	808			
	10	12386440 12683025	143.36	0.62		205	0.46	0.280 0.248	6741 6846			
	120	1287M 10 132701M	180.33	0.61		210	140	0.245	6000 6010			
	120	13672780	167.09	0.61		218	646	0.241	6115			
	124	1416080	162.04	0.61		219	0.46	0.236	677			
30 60 90 120 150 180 210 240 270 380 330 30 Time in days	50 126	14700120	170.82	0.61		224	0.45	0.232	6635			
	127	18268726	0426	0.61		226	0.44	0.230 0.238	4014			
Redies of allow more services from	128	10040475	104.05	0.61		235	0.44	0.236	6744			
Policitat un sensite autore versitat ante-	131	14043045	10142	0.61		238	0.44	6.225	6810			
********	133	160.0215	194.85	0.61		239	0.44	6.219 6.218	6968 7041			
	138	170405	201.72 205.15	0.41		243 245	0.44	6.216 6.214	7114 7186			
	127	18221030	204.68	0.00		247	0.44	6213	7218			
	138	18614725	216.48	0.00		241	0.44	0.210	7380			
A CONTRACTOR OF	14	16007816	222.31	0.00		245	0.44	4.08 6.307	7130			
and the second se	14	19804480	226.75	0.00		287	0.44	6.206 6.206	7028			
A STATE OF THE STA	14	2004235	236.04			241		6202	2011			
	147	20887830	256.48	0.00		245	10	6.000	7678			
	148	2 (383840 2 1580675	246.34 249.78	0.00		249	143	6.16F	8211			
/	162	21877140 22173748	263.21 206.64	0.00		272 276	143	6.195	8141 8208			
	10	22470830	260.07	0.00		278	10	6.165	K270			
/	154	23063830	205.04	0.00		2.80	143	6.101	6387			
	100	2300470	273.80	0.00		283	143	6.180	8623			
	107	2001040	277.24 28047	0.00		285	10	6.185	8647			
	100	24846428 24843010	284.15 287.63	0.00		289	10	0.185	8708 8770			
20 60 90 120 150 180 210 240 270 200 230 36	10 141 142	2010/00/00 2040/01/00	280.87 284.40	0.00		282	10	6140 6140	MK3D MR01			
Time in days	143	24/52748 24024580	287.83 301.27	0.00		205	143	6.162	8010 9010			
	145	24023838	304.70 306.13	0.00		209	143	6.180 6.179	9670 9129			
	147	24810126	211.84	0.00		102	143	6178	8187			
	148	279(3279	218.43	0.00		105		6176	104			
	179	27%D484D 2W101448	321.86 326.29	0.88		307	140	6.176	NAD N 19			
	10	20483030 20680415	328.73 332.14	0.00		3.10 3.12	142	6176	9475			
	176	246645250 2469 (198	335.69 339.03	0.00		314	142	6170	95.80 96.45			
	178	200846375 2008-0055	342.44	0.00		3.17	142	6130	8702			
	178	30W1640 304/9126	348.32 362.76	0.00		320 322	142	6.168	NH 13 NH 04			
	140	30752710	204.19 20142	0.00		323	10	6.147	9823 9877			
	142	3 CHEMING	363.05	0.00		3.26	10	6.166	10001			
	142			- 4			- 44					

C-zone, 46 darcy

Appendix B 11/12/01 Page B-19

D-zone, 1.4 darcy

W:(74206/0110.035)Appendix B tables

Appendix B 11/12/01 Page B-20

4 1

Table B-13 Extimation of Steam Injection Rate and Steam Zone Growth												
			(ideal)					and shall be a			Deepy	
	1		0.00	10.03	3440775	600	32.03	6.005	3	1.0	£.120-00	
	1	240	0.00	0.12		601	8.40	0.000	3	1.0	6.310-00	
	1	480	0.01	0.01		601	4.45 2.48	0.000	-	11	8.70E-08 6.31E-08	
	6 7	1820	0.02	0.00		000	1.40	6812	-	13	7.626-00	
		7680	0.08	0.46		0.05	6.73	0.040	10	1.0	1.498=10	
	10	10560	0.04	0.41		6.10	6.60	6.825	26	20	2.348+10 4.046+10	
	1	61640 104640	121	0.36		014	0.41	6.816		47	7.248+10	
	13	147940	1.71	0.36		0.32	6.30	6.780 6.767	90	54	1.848-11	
	16	256240	2.71	0.33		0.28	6.37	6747	100	6.4	2.308+11	
		320640	3.71	0.32		633	0.36	6.716	107	73	3.048+11	
		407040	4.71	0.31		637	6.36	6.688	201	80	3.760+11	
	54 75	480440	6.21	0.31		0.38	0.34	446	207	8.5	4.430+11	
	м 77	52640	4.21	0.30		045	6.34	0.646	249	62	4.828+11 8.138+11	
	31 32	420040 919425	721	0.30		046	6.33	6.637	279 377	84 110	8.818+11 7.888+11	
	80 11	1218210	14.08	020		064	6.31	6.640	-	122	1.038-13	
	10	180K040	20.94	0.28		0.78	6.30	6.496	60	14.1	1.687-12	
	15	210846	24.17	0.28		0.00	6.30	64%	20	164	1.802-12	
		26/00/38	32.08	0.27		0.04	6.30	0.445	804	16.0	2.048+12	
		3/ 823/06	3.66	0.27		1.04	6.20	6.417	108	172	2.4%+12	
	-	3749475	6.0	627		1.13	6.20	6.314	1043	182	2.800+12	
	10 11	40/060	10.56 10.68	0.27		132	0.20	0.3%	1167	18.7	718-0	
		4879230	84.11	0.26		1.26	0.28	0.366	1002	194	3.828+12 3.738+12	
		1248400	40.06 96.41	0.24		134	0.28	0.000	1001	20.3	3.848+12	
		68471870	67.ML	0.24		141	0.28	6.337	10.00	21.1	438-12	
		6454740	74.71	0.24		148	6.28	6.326	1446	21.7	4.588+12	
	100	7547910	81.67	0.24		1.85	0.27	0.314	1673	22.4	6.168+12	
	102	7641040	88.44	0.24		141	6.27	0.304	1607	200	LETE-D	
	104	753568	91.30	0.24		147	6.27	6.500 6.2M	1004	252	6.010-C	
	106	MCXXX30 MK27420	66.74 102.17	026		1.75	6.27	0240 0246	10.75	25.8	6.110+12	
	107	9124005 9x20590	106.40	0.26		1.79	6.27	0.284 0.280	1854	263	6.580-12 6.780-12	
	109	6717175 10013760	112.47	0.26		1.82	6.27	6277	1620	268	6.860-12 7.170-12	
		10210345	118.33	0.26		1.87	6.27	6270	2012	26.2	7.3%+12	
***************************************	113	10823815	126.30	028		1.82	6.27	0.264	2010	267	2238-12	
	118	11494685	133.06	0.26		1.87	0.27	0.218	2141	26.1	8.172-12	
	114	12040405	126.62	0.26		200	6.27	6288	2009	26.5	A.540-12	
	118	12386443	143.36	028		205	6.27	0210 0248	2042 2014	267	8.56E+12 8.66E+12	
	120	12879613 13274195	180.23	0.26		210	6.36 6.36	0248	2307 2330	27.1 27.3	8.182+12 8.382+12	
	00 03	13672783	167.09	028		218	0.36	0.241	2370	27.6	8.882-12	
· · · · · · · · · · · · · · · · · · ·	124	14160800	163.04	0.26		219	6.36	0.236	2633	27.8	8.848-12	
210 240 270 300 330 360	106	14708(20	170.82	028		221	6.36	6232	2414	282	1236-13	
	100	18382290	177.69	028		228	6.36	0.238	2014	283	1270-13	
	130	10940402	104.05	0.26		233	6.36	0236	2612	28.7	1.118-13	
-	132	16263068	101.42	0.26		238	6.26	6221	2641	203	1.182-13	
	133	16404215 17131800	194.85	028		238	6.26	6.219 6.218	2000	283	1.196-13	
	136	1742(6)85	201.72 205.15	0.26		243	6.36 6.36	0.216 0.214	2768	284	1210-13	
	127	18021005	208.68	026		247	0.36	6213	2810	29.9	1288-13	
A A A A A A A A A A A A A A A A A A A	138	18414735	215.45	024		281	6.36	6210	2010	30.2	1288-13	
	141	19207995	222.31	0.24		288	0.36	6.207	2018	30.8	1.328+13	
	143	1940 1965	229.18	024		200	0.26	0.204	2871	307	1.362+13	
	145	20384235	236.04	0.24		243	0.36	6202	3623	31.0	1.436+13	
	147	2018/1405	242.01	0.24		247	6.36	6.190	3074	313	1.448-13	
	148	21283890	246.34 246.78	024		249	6.26	6.167	3100	314	1482-13	
	180	21877163 22173765	263.21 206.64	0.24		272	6.36 6.36	6.196	3180	317	1.800-13 1.820-13	
	102	20470330	260.07	024		278	0.36	6.165	3200 3224	318 320	1.848-13	
	104	25083800	205.04	024		2.80	6.36	6.101	3249	12.2	1.878-13	
	154	20484473	273.80	0.24		2.83	0.36	6.189	3247	32.4	1.618-13	
	158	24249643	28047	024		287	6.36	6.186	3044	324	1492-13	
· · _ · _ · _ · _ ·	103	2686625	284.15 287.63	0.24		289	6.26	6.166	2048	327	1496-13	
210 240 270 300 330 360	161 162	25/39095	280.07 284.40	0.24		282	0.26	6163	3418	353	1.718+13	
	163 164	26/5/2768 26/2762600	287.83 301.27	0.24 0.24		205	0.26	6.162	3461 3494	33.2 33.3	1.782+13	
	165	26323636	304.70 304.13	0.24 0.24		200	0.26	6.180	3607 3630	33.4 33.8	1.780+13	
	347	26910106	211.84	0.24		102	6.25	4.178	30	334	1428-13	
	168	279(2275	21843	0.24		305	0.25	6.17	201	117	Last-13	
	175	27W06860 28/056445	321.84	0.24 0.24		107	0.26	6.176	3610	358	1.802+13	
	172	26403050 26406015	328.73 332.14	0.24 0.24		3.10 3.12	0.26	6.176	3043	34.1 34.2	1.820+13 1.840+13	
	174 175	28/88/200 29/29 (19/5	336.89 339.03	0.24		314	0.26	6.172	3707	343	1.842+13	
	176	201064270 201064000	342.46	0.24		3.17	6.25	6.170 6.170	2780	34.5	1.880-13	
	178	30181540 30479114	349.32	024		3.20	0.25	6.100	2762	347	2.08-13	
	180	30/774710	204.19	0.24		323	6.25	6.167	3636	349	2.270-13	
	142	21367880	20042 20100	0.24		3.26	0.26	0.165	20	38.1	2.110-13	
	943	37464465	306.49	0.24		3.28	0.26	6.146	3887	362	2.138+13	



180 In days

(400) X0 Jack M

W::74206.0110.035/Appendix II tables

Appendix B 11/12/01 Page B-21

W::74206/0110.035/Appendix B tables

Table B-14 Extimation of Steam Injection Rate and Steam Zone Growth																	
D-zone, 13 darcy																	
Panamatar	Ipedial	Quel .	False						100	1 (days)	-	10.00	*	inere and	#49/mp2	Alt	
Techness of disarted layer Packete density of soil	-	*	10					2	40	6.00	1.36	3448770	0.00	90.15	0.985		10
Personally of sciencesia appro-	· · · · ·		0.3					3	240	6.00	1.38		0.01	25.80	0.980		
Annual permendelity of suit Head capacity of suit		4) 2018 X	1381					1	485	601	1.34		0.01	7.45	0.960		5
inital autor salization Antional annuare in another	6. 		54900					4 7	1820	0.02	1.31		0.03	4.38	0.872		10
Density of water	4	igit"	100						7980	6.09	1.24		0.05	2.04	0.945		28
(policed autor levonation Heat separity of water	Les.	K JihgXi	20					10	30720	0.06	1.10		0.07	1.27	0.825		71
Consult head sequely Consistent Research construction	1	(Appl)	200						61640	671	1.00		0.14	1.22	0.856		123
Cardundes Dennal diffusioly		eà.	6.0000001					-	147940	121	0.86		0.22	1.08	0.790		200
Fasilus of injustice and Maximum clearer inter fore arrestore		*	0.00						254240	221	0.82		0.26	1.00	6.747		307
Relative permeability of steam Energy character	· · · · ·	halms)	1,316.0					96 17	277640	321	0.80		0.31	1.00	0.730		413
Sergentize of slean	t	×	60						363840	421	0.88		0.36	0.06	0.751		812
Della T Heal af usulenzation	Ter Tara	i. Jha	2000.00					74	45/540	4.91	0.87		0.37	0.07	0.675		606
Tere due dat d'opelan	· · · · ·							75	483440	6.71	0.86		0.41	0.95	0.665		661
Volume of views zone	e.	*	i						679840	6.71	0.85		0.44	0.80	0.646		738
Gan constant Initial cadius of steam aure		(Made K)	0.00302					78	423040 919625	721	0.80		0.46	0.00	0.607		781
their mans of a stre	94a	alreiz	0.071					80	1218210	14.08	0.82		0.84	0.88	0.549		1305
Cheyde lactop of injection summer	Dige	abg	a 27					10	1808380	20.04	0.80		0.78	0.85	0.495		1790
Langth of injection screen	inger an	*						-	2100968	24.37	0.79		0.85	0.85	0.47%		1953
April Press and				1					25/00 125	2.0	9.77		0.00	0.00	0.445		2200
								86	2816730	33.42	0.77		0.99	0.83	6.430		2448
									348880	6.35	0.76		100	0.00	0.405		2797
									2746475	6.0	0.76		1.13	0.81	0.3%		2948
									4279448	82.48	0.76		1.22	0.80	0.374		3266
			and a section of	Time .					4071815	17.14	0.75		1.30	0.80	0.368		366
12000							1		1048400	46.41	0.74		1.34	0.79	0.362		3485
19770								96	6661670	0.14	0.74		141	0.79	0.327		3965
									6/108100	71.27	0.74		148	0.79	0.326		4213
10000								80	676(326	78.14	0.73		1.01	0.78	0.319		4328
								901	7344496	86.01	0.73		1.64	0.7%	0.309		4582
								100	7641080 7937688	88.44	0.73		141	0.78	0.304		4701 4817
0000								104	1234310	85.30	0.73		147	0.77	0.2%		4802
								126	8827420	162.17	0.72		1.73	0.77	0.288		6196
1 martine								107	9/24006 N/20800	106.40	0.72		1.79	0.77	0.284		6265
3 6000		******						120	0717175	10.47	0.72		1.82	0.78	6.277		6479
					*****	*****************		111	10210345	118.33	0.72		1.87	0.7%	6.279		6487
2								112	10K04830 10KE3615	102.77	0.72		1.80	0.7%	0.267		6290
4000								114	1/200130	12643	0.72		1.85	0.7%	0.261		0.060
-								116	11783279	136.80	0.71		200	0.76	0.268		6186
								117	12080655	128.82	0.71		203	0.76	6.263		6280
2000								110	12683026	146.79	0.71		2.07	0.75	0.248		6468
								121	13274195	163.66	0.71		2.12	0.75	0.243		6612
1000								120	13072780	167.09	0.71		2.16	0.75	0.247		6743
								124	1416090	163.04	0.71		2.10	0.75	0.236		6821
0 30 60	90 120	150	180 Time in day	210 240	270	300 330 3	60	126	14700120	170.82	0.71		221	0.75	0.254		7209
								127	18266726	174.26	0.71		226	0.75	6.200		7183
								129	10040675	101.12	0.71		2.30	0.74	0.226		7363
		Radius	of steam zone vers	us time				130	1642045	181.00	0.70		233	0.76	6.225		7120
								132	1603830	191.42	0.70		2.37	0.7%	0.201		7802
55								126	17131830	198.28	0.70		2.41	0.7%	0.218		7765
m.								136	17/24879	20172	0.70		243	0.7%	0.216		7845
-			· ······					137	1821055	208.68	0.70		2.47	0.7%	6.213		8004
45		- And the second						130	18014725	215.45	0.70		2.81	0.74	6.210		8160
×								140	18011310	218.88	0.70		2.83	0.74	0.208		8207
1	and the second s							142	19804480	226.75	0.70		2.87	0.7%	0.206		8380
5 ²⁵								144	20087680	232.41	0.70		241	0.74	0.203		8540
š								145	2094235	236.04	0.70		243	0.73	6.202		8415
								147	2087105	242.01	0.70		247	0.73	0.199		8792
								140	21580675	249.78	0.70		271	0.79	6.187		8907
ž 20								190	21877140	283.21 206.64	0.70		272	0.73	0.195		8879
								112	22470830	260.07	0.69		2.76	0.75	6.190		0121
° /								185	22764818 23063830	20101	0.69		2.78	0.75	0.180		6182
12								195	23060045	270.37	0.49		2.82	0.73	0.190		6001 9400
								187	23963255	277.24	0.69		2.85	0.75	6.187		6489
								158	2404940	280.67	0.49		2.87	0.75	0.186		9425
		100		242	220		4	140	24843010	287.63	0.69		2.80	073	0.184		9672
v 30 60	-0	-av 1	180	210 240	2/0	an 330 3		162	25424130	284.40	0.69		234	0.75	0.162		0406
			Time in c	lays				145	24/32745	287.83	0.49		2.85	0.75	6.18D 6.181		64/2 9906
								145	2020000	304.70	0.00		2.86	0.73	0.180		12003
								147	24810105	211.04	0.69		3.02	0.72	0.12%		10133
								148	272110030	316.00	0.49		304	6.72	6.177 6.178		10188
								179	27804840	321.86	0.00		3.07	0.72	0.175		10026
								172	20402030	328.73	0.69		3.10	0.72	0.176		104/02
								175	20000015	332 14 335.09	0.49		3.12	6.72	6.173		12616
								175	2120 1785	326.03	0.69		3.16	0.72	6.171		12628
								177	21004005	345.89	0.00		3.19	0.72	6.175		10742
								178	30181540	349.32	0.69		320	0.72	0.169		10823 10884
								180	30774710	204.19	0.69		3.23	0.72	0.167		12048
								142	3/06/1940	30842 363.05	0.68		326	0.72	0.166		11048
								183	31664445	306.49	0.68		3.28	0.72	G 165		11126

Appendix B 11/12/01 Page B-22
FINAL THERMAL TREATMENT TECHNOLOGY CONCEPTUAL DESIGN McCormick and Baxter Superfund Site

W:(74206)0110.035/Appendix B tables

Table B	15											
E-zone, 16 darcy	te and Steam	Zone Grow	th									
Pounder Bonial Liber Islam	116	11(ev)				1000	et/ml	40		Every	in the	
Submit is descent by: A A A A A A A A A A A A A A A A A A A			0.00	348.30	3448773	0.00	348.30	0.987	22	24	6.528 - 10	27380N
Config datased law 6 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1	240	0.00	4.28		0.01	88.62	0.943	25	2.7	6.73E+10 6.73E+10	33691
Sand Anguadhy of And Anguade Ang		80	0.01	4.20		0.01	26.64	0.986	2	2.0	6.547 - 10	33301
Name of person is again the second seco	7	3840	0.04	4.08		6.62	0.46	0.873	-	3.6	0.000-10	32323
delay and base of the second sec		1040	0.18	3.81		0.05	6.26	0.947	-	13	2.188+11	30163
Para (angula) of a state. 90 - 2016 1. 1.10 Donal hand angula) 90 - 2016 1. 2010 Donal hand angula	-	61440	0.36	143		6.10	3.84	0.000	282	8.0	5.64E+11 6.43E+11	2/243
Analysis in the analysis of the second se	10	147940	121	3.12		6.16	3.80	0.849	620	120	1.388+12	24762
nalizar diplana na la constanza de la constanz La constanza de la constanza de	-	234240	271	2.00		6.19	3.32	0.816	761	164	2078-10	23041
Notice personality of class to the second se	10	277440 320640	321	2.86		6.23	3.26	0.803	95	14.7	2.21E+10 2.71E+10	23213
Improvide of datase In K	-	407540	421	2.87		0.26	3.16	0.776	1200	18.5	27.8×0	22040
Ref Conductions Part of the Pa	75	40040	621	2.81		0.28	3.04	0.768	1404	204	373E+13 408E+13	2221
Natura da la mais de la constancia de la c	77	679640	471	177		6.30	3.04	0.734	1604	224	4708-10	22002
La nominational B John Reg. B John Reg. B La Construction	2	019626	10.64	278		0.31	2.83	0.727	20.08	233	50'E+0 7.18E+10	21784
Martenan da autor 6.000 Dennajar da autor 6.000 da autor 6.00		1612796	17.41	248		6.49	2.87	0.623	2626	304	1.142+13	20041
Dight long of pipeline searem Diru Ab.g 77.3 angla of pipeline searem pipeline a.d 4.4		1800/380 2105/865	20.M 24.37	2.40		6.63	2.79	0.800	3996	35.7	1.548+13	20545
Apple System Seal		2412/000	27.81	2.65		0.61	2.75	0.843	400	20.8	1.708+13	20185
	-	2896/20	33.42	2.60		647	2.01	0.837	8724	437	2088+13	19943
	-	34MIND 31MIND	63	240		0.74	2.66	0.510	425	458	24/8+13	18716
		4040980	47.26	2.45		6.80	240	0.4%0	7387	48.5	2.882+13	19633
		471220	64.11	2.44		0.00	2.41	0.471	8156	80.0	3208+13	18342
40000 Massflux as function of time		0940	40.88	243		0.01	2.88	0.418	800	832	3641+13	160
	-	6847.070	67.84	2.47		0.04	2.87	0.640	9687	86.3	6028+13	19141
2000	1	6456740	76.71	240		1.00	2.88	0.427	16277	872	4408-10	18043
	100	TOUTHED TOUCHE	8.17	2.38		1.06	2.54	0.418	1004	880	4.7WE>13	10000
	102	7647.040	88.44	2.38		1.00	2.83	0.404	11670	60.7	6.162-13	18676
	104	10100	96.30	2.37		1.13	2.02	0.3%	12187	623	6.538 + 13	18804
2000	106	M27420	102.17	2.57		1.17	2.81	0.388	12/16/7	63.8	6.802+13	18738
8	108	0420100	108.83	2.8		121	2.00	0.376	12271	65.2	6200+13	18077
	110	10013760	115.80	2.38		128	2.0	0.368	13840	664	6408-13	18621
2 m	10	10604830	122.77	236		120	2.48	0.361	1486	67.0	7028+13	18040
	114	11200100	129.43	2.56		1.32	2.47	0.383	18230	69.2	7.388+13	18520
	116	112902230	136.80	2.33		1.36	2.47	0.347	18670	704	7798+13	18474
	116	12586440	143.36	2.32		130	2.46	0.341	1600	714	A12E+13 A12E+13	18431
500	120	12079610	180.23	2.32		140	2.45	0.338	16600	72.7	A488+13 A478+13	18201
	122	13072780	187.00	2.31		145	2.45	0.328	17100	73.8	AME+13	1612
	134	141638400	163.86	2.31		1.49	2.44	0.324	12081	74.8	0.228 + 13	18216
יסט גע גע אין	136	14758120	175.82	2.31		112	2.44	0.318	18073	75.8	0.582-13	18281
	128	153622160	177.40	2.30		1.85	2.0	0.314	18547	76.8 77.3	0.00E+13 10/E+14	18248
Radus of steam zone versus time	130	15045440	184.85	2.30 2.30		1.88	2.43	0.310	18213	77.8	1038+14	18217
¹⁰	122	1482/06/30	191.42	2.29		141	2.4	0.308	19471	78.7	1078+14	18187
20	134	1713/W00 1702/000	198.28	2.29		143	2.41	0.301	19822	79.6	1.128+14	10100
5	136	1712-0110	205.15	2.29		144	2.41	0.2107	20364	80.0	1.140+14	18130
	138	18078140	213.82	2.28		149	2.41	0.210	20804	114	1.100-14	18105
2	140	18011010	218.88 202.31	2.28 2.28		1.72	2.40	0.200	21034	82.2 82.6	1218+14	18275
40 60	142	1960-6480 1960-1065	206.76 206.18	2.28 2.28		1.74	2.40	0.286	21881	#3.0 #3.4	1208+14	18043
	144	2008/NIB0 2008/208	202.41 206.04	3.27		1.77	2.40	0.282	222881	818 842	1208+14	18220
	140	20804820 2098/3408	208.48	3.27		1.80	2.38	0.279	22486	844	1.328+14	18006
	148	21283940 21580K75	248.34	2.27 3.27		142	2.38	0.27N 0.27N	22864	85.4 85.7	1.348+14	17984
	180	21877160 22173748	263.21 266.64	3 27 3 26		1.85	2.38 2.38	0.275 0.271	23367	86.1	1.208+14	17943
	10	22479330 22794815	265.67	2.26 2.26		1.87	2.38	0.270	23706 23804	850 872	1438+14	17942
2	194	230638000	205.84 279.37	3.26 3.26		1.80	2.38	0.247	24/00	874	1.488+14	17922
	196	23654670 23963298	273.80 277.24	3.26 3.26		1.82	2.38	0.244	2489	88.3	1,528 - 14	17962
	198	2424/8440 2484/6428	280.47 284.10	3.26 3.25		1.84	2.37 2.37	0.240 0.240	24874 29886	80.0	1.030+14	17643
	140	248X3010 25/2008	287.83 290.87	2.28 2.28		1.87	2.37 2.37	0.218	21266 25443	884 800	1.078 = 14 1.098 = 14	17846
Time in days	10	25626180 26752766	294.40 297.83	3.26 3.26		1.89	2.37 2.37	0.218	29631 29818	90.3 90.6	1428-14	17847
	164	24024030 24024838	301.27 304.70	2.28 2.28		2.01	2.37 2.38	0.264	24003	81.0 91.3	1.648+14	17830
	146	24020120 24019105	308.13	2.28 2.28		2.04	2.38 2.38	0.210	24072	81.4 81.0	1498-14	17913
	148	275/16090 275/2275	3/8.00 3/8.43	2 2N 2 2N		2.04	2.38 2.38	0.248 0.248	24736 24817	82.2 82.6	1.718×14 1.738×14	17786
	170	27504840 28105648	321.84 326.29	3 2N 3 2N		2.08	2.38 2.38	0.247 0.248	2/58/7 2/52%	82.0 83.2	1.798×14 1.778×14	17780
	12	28402030 28608015	328.73 332.16	3 2N 3 2N		2.10	2.38 2.38	0.248 0.244	27488 27902	83.6 83.8	1.700+14	177MB 177MF
	176	2804(20) 2829 (788	226.89 229.83	2 2N 2 2N		2.13	2.38 2.38	0243 0242	27908	84.1	1.828 - 14	17740
	136	296840370 296864908	342.44 345.89	2 2N 2 2N		2.16	2.38 2.38	0.241	28100 28003	84.7 84.0	1.808+14	17734
	178	30/8/840 30/79/28	368.32 362.76	2 20 2 20		2.17	2.38 2.38	0.238 0.238	20006 20078	86.2 86.6	1.898 - 14	17730 17713
	140	30774710 31071096	364.19 368.42	3 25 3 25		2.19 2.20	2.38 2.38	0.237 0.238	2000 20021	86.0	1808+14	17706
	142	31367680	363.05 366.49	3.25 3.25		231 233	2.34	0.236	20101	96.4 96.7	1.002+14	17642

W:(74206/0110.035)Appendix B tables

Table B-16 on of Steam Injection Rate and Steam Zone Growth E-zone, 48 darcy, 928 kPa Energy Inputed Radus of steam zone (meter)

W:(74206)0110.035/Appendix B tables

E-stimation of Steam	Table B-17 Injection Rate and Stean	1 Zone Gr	owth	
Parameter Bandard Bandar		101	1 (Says) 'm	
being sender at an and an an and an		100	0.00	
Turanji il shanani lugir 0 0.02	3	240	000	-
And capacity of all of		80	001	
nide statute and the statute of the	4	1620	0.02	3
Density of autor 4. sp ^{an} 100		7680	0.09	3
Accident data Internation Tou E 200 Deal capacity of safety	-	16340	0.18	3
Constituted space of the second	-	61440	0.71	-
Annual and annual and annual and annual a	5	147940	121	
Tables of padamant		101040	221	3
Mining percentility of shares 4		277680	321	3
Temperature of dears		363640	421	
Data T 5. Tan 6 16 Nationalism 6 An 2000	-	407540	471	
Interiore dark dirighten	2	480440	671	
Notes of these sees		671660	471	
En conteil 1 2000 1 200	24	623040 01M26	721	
Advise manual and advised and advised a	-	1214210	14.08	
	-	1808.080	20.84	
angli of lopinion scenars equipment of the state of the s		2105/665	26.37	
içan yasan xa		2010/00	20.08	
	-	2896/20	33.42	
	<u> </u>	348880	6.3	
		176625	6.8	
	-	4379645	52.48	
Manifux as function of time		4071010	67.54	
2000		104440	40.58	
2000		68411070	67.84	
		6454/742	76.21	
200		6751326 7067910	78.14	
2000	101	734648	85.01	
	100	7527965	91.47	
	104	823x280	96.30 96.71	
e mort base	138	8827-00	102.17	
5 HOLD	- 108	0420100	108.43	
	100	6F12125	112.47	
		10010348	118.33	
⁻ 1000	10	10803616	126.20	
		11200100	128.43	
	116	11790270	136.60	
600	114	125866440	143.36	
400-	- 10	12683026 1287WE10	180.23	
	121	13274106	183.44	
200		13802080	162.43	
	14	14462636	167.30	
o ao oo ao ao ao Tina indaya. Tina indaya	30 38	14750120	176.62	
	128	15352210	177.49	-
	130	15545450	184.85	
90 Hoadduu o'r Anlam Zone Wethu's triffe		14242046	187.80	
		16804215	194.85	- 3
80-		17121800	201.72	
		1772-0170	205.15	
70		1807/8140	212.82	- 3
and the second	140	180112-10	218.88	
S 0		18207846	222.31	
§	10	19601045	208.18	
§ S		20364238	206.04	
1 Andrew Andre	141	20890820 20987408	208.48	
		21283040	268.34	
	10	21877160	263.21	
^e 2	10	22173748 22479330	266.64 260.07	
	103	22764915	263.61 265.94	
20	1 3	23360546	276.37	
V	166	23654670 23963256	273.80 277.34	
		24249840	280.47 284.10	
	140	24843010	287.83	
0 20 60 90 120 150 180 210 240 270 200 230	360 16	26/3606 25636180	265.67	1
Time in days	10	28752748 2822830	287.83 301.27	
	145	24221838	304.70	-
	146	24822830	308.13	
	148	27210600	3/8.00	
	110	27506840	321.86	
	171	28105445 28402030	328.29 328.73	
	13	2809815	302.16	
	174	29291785	329.43	- 1
	176	29684070 29684965	342.44	
	178	30181840	368.32	
	140	30774710	384.19	
	141	31071016	388.42 363.05	
	16	31854465	306.49	-

N	((linged)	-	10.00	7	inane .	with the second	A(1)	-0	Energy Injusted	(m)(late)
100	0.00	238.30	3448173	6.00	238.30	0.967	16	23	3.8W-10 3.8W-10	20613
240	0.00	3.07		441	61.88	0.943		2.3	3.ME+10	24004
NO	001	3.00		601	17.74	0.346		2.6	4.540-10	24000
1620	0.02	2.99		642	6.65	0.941	20 29	2.7	6.5'E-10 6.8'E-10	23710 23240
7680	0.08	2.84		643	4.78	0.142	-	3.4	8.728 - 10	22838
30720	0.36	2.68		0.07	3.98	0.926	106	14	2.598-11	20130
61440 104840	121	2.45		610	2.81	0.871	286	2.7	7.248-11	1828
147940	1.71	3.20		0.16	2.49	0.849	360	11.0	0.828 = 11	17942
234240	271	3.12		6.10	2.36	0.816	554	133	1478-10	16766
277440 320640	321	2.08		631	2.32 2.38	0.803	408 720	143	1.718-12	16542
36360	421	2.04		0.24	2.28	0.779	799	15.0	2.188-12	16166
490240	6.21	2.00		6.26	2.21	0.799	961	17.4	248-12	19845
10040	621	1.00		028	2.10	0.740	1006	18.1	3.118-10	16540
673660	4.71	1.87		6.30	2.16	0.734	1140	18.3	3.338-12	10540
010026	10.64	1.00		0.38	2.08	0.684	1743	23.3	6.108-10	16431
1010210	12.08	1.60		0.44	2.03	0.610	2130	26.0	6.608-10	1610
1828380	20.84	1.86		643	1.87	0.600	2908	304	0.528 - 12	14942
2105948	26.37	1.40		641	1.85	0.540	3248	323	1.248+13	14203
2010/135	30.08	1.79		0.64	1.62	0.852	3837	34.0	1.338=13	14163
2816750	33.0	1.77		647	1.00	0.020	477	364	14/8-13	14066
348880	40.38	128		674	1.00	0.812	4710	380	1.798 - 13	12834
4042040	47.26	1.75		6.60	1.8	0.490	6248	41.3	2038-13	13802
4075200	55.48	1.12		645	1.0	0.480	100	44	2.308-13	13748
4071010	67.54 40.00	1.72		0.00	1.84	0.40	47.94	44.4	2.648+13	13844
0044786	66.41	1.71		6.63	1.42	0.447	67.0	46.2	278-13	13856
6158155	17.47	170		0.04	1.0	0440	49/71	47.0	2.842-13	13610
6456/ND 6731328	76.71	1.70		1.00	1.0	0.427	7684	48.7	3.118-13	13448
7047910	81.87	140		1.06	1.40	0.418	7940	80.3	3.388+13	12246
7647060	85.44	148		1.00	129	0.404	800	81.7	3647-13	13327
7507465 6236210	0.47	1.48		1.11	1.28	0.389	MOR MIR	124	3.7WE=13 3.8YE=13	13201
8132428	98.74	1.47		1.16	1.78	0.389	9047	83.7	4047-13	13281
0124005	105.60	147		1.10	1.27	0.340	9476	86.0	4308-13	13206
6420100 6717175	108.83	1.46		121	1.77	0.379	9706	66.2	4.645+13	13164
10013760	116.80	1.66		128	1.8	0.368	10118	86.7	4.708-13	13144
10004830	122.77	148		120	128	0.341	1001	87.0	6 ME-13	12126
12003015	126.20	1.45		1.30	1.76	0.367	10710	55.0	6.0MF=13 6.22E=13	13048
11404685	133.06	1.45		1.34	1.76	0.340	11124	88.5	6.308+13	13064
120610636	128.83	1.64		1.37	1.74	0.344	1140	60.5	64/8×13	13625
12586440 12683026	143.36	1.64		130	1.N	0.341	11656	61.0	6.74E+13 6.87E+13	13007
12079610	180.23	144		142	1.73	0.338	12046	61.0	6008-13	12878
13072780	187.09	143		148	1.73	0.329	12408	62.8	6268+13	12890
14163860	162.65	143		149	1.73	0.325	12046	633	6.500-13	12837
14402138	167.30	140		1.60	1.72	0.321	12838	64.2	668-13	12011
1806/0706	174.26	143		143	1.72	0.316	13286	65.0	6.802-13	12887
1964875	181.12	142		1.66	1.72	0.312	134218	65.8	7.148-13	12814
15545460	184.85	142		1.88	1.21	0.310	12763	66.2	7208-13	12863
1453(630)	191.42	142		141	1.21	0.308	14/08	67.0	7.848-13	12831
1713/1800	198,28	1.42		143	1.21	0.301	14401	67.8	7.808+13	12810
17/24088	201.73	141		146	1.70	0.287	14773	68.5	7.83E-13 8.08E-13	12800
18021688	208.88	141		148	1.70	0.298	1402	65.0	A 180-13 A 310-13	12781
1801-0726	218.48	1.41		1.70	1.70	0.201	18247	69.7	8.648+13	12/12
18207996	202.31	1.41		173	1.70	0.200	1996.7	70.4	8498-13	12744
1960-6680 1960-1068	226.76	140		1.74	1.00	0.286	18710	70.7	8.828-13	12738
2008/1680	202.61	140		1.77	1.0	0.282	14014	714	9278-13	12718
20690820	208.48	1.40		1.80	1.40	0.279	14214	72.0	0.328 = 13	12/04
20967405 21283940	242.81 246.34	140		141	1.40	0.277 0.278	16403	724 72.7	0.100-13 0.500-13	12045
21580K75 21877160	248.78	140		143	1.00	0.274	16767	73.0	9.70E+13 9.80E+13	12678
22173748	256.64	1.40		1.86	1.48	0.271	17947	73.6	9.3ME = 13	12642
22/76/015	263.81	140		1.85	1.44	0.270	17394	74.0	1008-14	12646
23063600	206.84	1.00		180	1.48	0.247	174256	744	108-14	12640
23656670	273.80	1.00		1.82	1.68	0.264	12784	78.2	1.0481-14	12626
2424/840	280.67	1.60		184	1.48	0.242	18236	75.6	1.040-14	12813
24843010	284.10	1.00		186	1.07	0.240	18012	760	1.100-14	12606
25/39096	200.87	1.00		1.86	1.07	0.258	18445	76.6	1.1281-14	12040
26752766	287.83	1.88		2.00	1.07	0.268	18710	77.2	1.108-14	12680
24024380	301.27 304.70	1.00		2.01	147	0.264	10004	77.8	1.108-14	12676
20022030	308.13	1.00		2.04	1.07	0.242	18/20	78.0	1.180-14	12042
21216680	3/6.00	1.58		2.06	1.07	0.249	18384	78.5	1218-14	12680
276/2276 2760860	378.43 321.86	1.08		2.07	1.42	0.248	19646	78.8	1208+14	12644
28/05445	126.20 326.77	1.00		2.09	146	0.248	18775	79.3	1208+14	12633
20000010	332.16	1.00		211	14	0.244	2003	768	1278+14	12822
21091785	208.03	1.00		2.14	1.00	0.242	20287	80.3	1.308+14	12011
29684070 29684965	342.46	1.08		2.16	1.66	0.241	20414 20840	804	1.3/8+14	12606
30181840 30479128	34832	1.00		2.17	14	0.238	20885	811	1338+14	1248
30774710	354.19	1.07		2.10	146	0.237	20814	81.6	1.368+14	12485
31071016	368.42 363.05	1.07		230	1.46	0.236	2108	82.1	1.378+14	12480
31664465	366.49	1.07		232	1.65	0.234	21283	82.3	1400-14	12470



Table B-18 Estimation of Steam Injection Rate and Steam Zone Growth

Parameter	Symbol	Unit	Value
Thickness of steamed layer	h	m	4.5
Particle density of soil	ds.	kolm ³	265
Porosity of steamed layer	P		0.35
Absolute permeability of soil	ks.	m ²	1.44E-10
Heat capacity of soil	ф.	J(kgK)	115
initial water saturation	5-		
Ambient pressure in aquifer	Pank	Pa	81212
Density of water	d*	kgim ³	1000
Ambient water temperature	Tanà	к	295
Heat capacity of water	cp.	J(kg K)	418
Overall heat capacity	cp++	J(kg K)	2200
Overburden thermal conductivit	k	J(s m K)	1.3
Overburden thermal diffusivity	8	m ² /a	0.00000018
Radus of injection well	t=	en .	0.0505
Maximum steam injection press	P	Pa	92823
Relative permeability of steam	k.s.		
Steam viscosity	U%	kg((m s)	1.31E-05
Temperature of steam	T.	к	44
Deta T	Tu - Tank	ĸ	153
Heat of condensation	'n	Jikg	2566000
Time since start of injection	t	3	
Steam mass flow rate	'n	kgis	
Volume of steam zone	v	m3	
Gas constant	R	J(mole K)	8.314
Initial racius of steam zone	tw	m	0.053025
Mole mass of water	M-	kgimole	0.018
Density of steam	duinan	kgim ³	4.0
Depth to top of injection screen	Dages.	mbg	73.2
Length of injection screen	Lisjaareen	m	4.5
Aquifer hydraulic head	Dget	mba	
and a second			

(b.b.)
 (b.b.)



APPENDIX C

Safe Injection Pressures Under Old Mormon Slough

SAFE INJECTION PRESSURES UNDER OLD MORMON SLOUGH

Due to the low elevation of the slough bottom and the presence of water in the slough, it is not safe to inject steam at shallow depths below the slough. This brief analysis was performed in order to determine the shallowest safe and practical steam injection depth, and also the shallowest safe electrical heating depth.

The analysis is based on the following physical situation:

- To inject steam, the injection pressure must exceed the hydrostatic pressure at the top of the well screen (otherwise there is not a pressure gradient away from the well screen, and fluids cannot be injected).
- The rule of thumb for estimating the maximum safe injection pressure of 0.5 psi per foot of overburden was used to set the maximal steam pressure at a given depth.
- Electrical heating may produce steam locally at depth without producing a significantly elevated pressure compared to hydrostatic.
- The soil liquefaction pressure is equal to the overburden pressure minus the hydrostatic pressure at a given depth.
- If the steam injection pressure exceeds the soil liquefaction pressure, steam injection is considered unsafe and likely to result in soil lifting and shallow steam escape.
- If the soil liquefaction pressure is negative, electrical heating is considered unsafe and likely to result in soil lifting and shallow steam escape.
- For steam injection to be practical at a given depth, the available injection pressure must exceed the static water pressure by at least 5 psi in order to allow substantial injection rates.

At most locations below grade, the soil overburden pressure is greater than the groundwater hydrostatic pressure. This would be a positive "liquefaction pressure." However, at shallow depths below the slough, the overburden pressure is less than the hydrostatic pressure; the most extreme case is at the mudline, where the overburden pressure is zero, yet the hydrostatic pressure is equal to about 10 feet of water. If a steam bubble moves into a region of negative liquefaction pressure, it is very likely to liquefy the soil as it moves upward.

The results are provided in Table C-1 and Figure C-1. The conclusions are as follows:

- 1. Steam should not be injected directly into the A- or B-zone aquifers under the slough (shallowest safe injection location under the slough is at an elevation of -90 feet).
- 2. The A-B aquitard and the B-zone aquifer can safely be heated by electrical heating using electrodes placed under the slough (deeper than an elevation of -40 feet).

These are the principles used to design the heating pattern for the shallow N-MPA for Scenarios 2 and 3.



Figure C-1. Schematic of the injection and overburden pressures under Old Mormon Slough with indication of safe injection depths for steam and electrical heating (ERH).

Table C-1Calculations Used to Estimate Safe Depths for Steam Injection and
Electrical Heating Under Old Mormon Slough

	overlying				ovb - hyd	GRAS		
	soil	elevation	hydrostatic	overburden	pressure	pressure		
location	density	NVD88	pressure	pressure	differential	differential		
description	(lb/cu ft)	(ft)	(psig)	(psig)	(psid)	(psid)		
MHHW	N/A	3	0	0	0	0		
MLLW	N/A	-3	2.6	0	-2.6	6.5		
mud line	N/A	-10	5.6	0	-5.6	7.8		
bottom of mud	70	-16	8.2	2.9	-5.3	8.2		
bottom of slough deposits	90	-31	14.7	12.3	-2.4	9.2		
bottom of A aquifer	100	-45	20.8	22.0	1.2	10.2		
bottom of A-B aquitard	100	-56	25.6	29.7	4.1	10.9		
bottom of B aquifer	100	-81	36.4	47.0	10.6	12.6		
bottom of B-C aquitard	100	-115	51.2	70.6	19.5	14.8		
bottom of C aquifer	100	-132	58.5	82.4	23.9	15.9		
bottom of C-D aquitard	100	-138	61.1	86.6	25.5	16.3		
bottom of D aquifer	100	-165	72.8	105.3	32.5	18.1		
bottom of D-E aquitard	100	-220	96.7	143.5	46.9	21.8		
bottom of E aquifer	100	-260	114.0	171.3	57.3	24.5		
MHHW = Mean higher high	water							
GRAS = Generally Regarde	ed As Safe							
GRAS Assumptions - It is ge	enerally safe to i	nject steam a	it a site if:					
1) five feet of vadose zone	1) five feet of vadose zone is present (uncapped)							
2) the injection pressure is limited to 0.5 psig per foot of depth								
3) GRAS pressure differential also assumes that an additional 5 psid is required for proper steam flow								

APPENDIX D

Electrical Resistivity Tomography

ELECTRICAL RESISTIVITY TOMOGRAPHY

Electrical resistivity tomography (ERT) has revolutionized the monitoring of subsurface processes and has allowed the acquisition of three-dimensional images of the subsurface. ERT produces a cross section of the distribution of information between measuring points, the distribution having, in some cases, nearly as much certainty as the measuring points themselves. In ERT, this distribution is of resistivity, and the measuring points are electrodes placed at various intervals around a target area. After numerous measurements are made from current transmitted and received from all possible combinations of electrodes, a resistivity distribution is reconstructed using a finite element mesh model to produce a two-dimensional cross section of the area underground. Data collected in two or more planes at once can generate three-dimensional images.

ELECTRICAL PROPERTIES OF ROCKS

Resistivity is a physical property of a material describing how much that material resists the flow of electric current. It is measured in ohm-meters. Conductivity is the inverse of resistivity, and it is measured in 1/ohm, or siemens, per meter (m). The better the current flows through an object, the higher its conductivity, and the lower its resistivity.

In mineral grains, there are two ways in which current flows: through electrons, which is called electronic conductivity; and through ions, which is called electrolytic conductivity. However, the pore fluids in rocks are actually what drive the rock's resistivity, as mineral grains are poor conductors of any type. A rock's resistivity is therefore determined by the following factors:

- Porosity
- Permeability
- Saturation
- Resistivity of the pore fluid

In monitoring steam-enhanced remediation, ERT detects changes in resistivity that reflect saturation changes, which are caused by the movement of steam through the subsurface and the corresponding temperature changes therein. The porosity of soil and rock remains constant before and during steaming. According to Archie's law, bulk resistivity is an inverse power function of both porosity and saturation (Archie 1942).

$$\rho_t = F * \rho_w / S_w = 0.62 / \phi^{2.15} * \rho_w / S_w^2$$

Where:

- ρ_t = bulk resistivity of partially saturated clay-free sediments
- F = formation factor, a function of porosity
- $\rho_{\rm w}$ = pore fluid resistivity
- $S_w = saturation$
- ϕ = porosity

This equation is graphed in Figure D-1. With water resistivity assumed to be 1 ohm-m, the general trend is for bulk resistivity to decrease as saturation increases. A medium that is more than 80 percent saturated at any porosity may have more than an order of magnitude increase in resistivity when its saturation decreases to less than 20 percent.



Figure D-1: Resistivity in ohm-m versus saturation in percent for a range of porosities. Water resistivity is assumed to be 1 ohm-m.

Pore fluid resistivity also decreases as temperature increases, due largely to the fact that ion mobility increases with temperature. The resistivity of a saturated rock is an exponential function of the reciprocal of temperature (Llera et al. 1990). A simplified relationship is indicated by the following equation:

$$\rho_t = \alpha / \phi^{2.15} * e^{k/T}$$

Where:

 α = a function of water resistivity, viscosity, hydration, ionic radius, concentration, elementary charge, Faraday's constant, and valency k = a function of Boltzmann's constant and activation energy of viscous flow T = absolute temperature ϕ = porosity

A plot of resistivity versus temperature is shown in Figure D-2 for a wide range of porosities. The values for pore fluid resistivity (ρ_w) may be substituted into Archie's law to determine how the bulk resistivity changes with temperature.

There is an apparent risk that resistivity increases due to dewatering during steaming of a saturated zone and resistivity decreases due to the high temperatures of steam will ultimately produce no change in resistivity. Typically, this problem is avoided because one regime will dominate over the other, such that even small changes in resistivity (e.g., 20 percent) lie within the sensitivity range for ERT detection. The dominant regime may change over time. For instance, a steamed volume may initially experience a period of heat-up, followed by intense drying out, both of which would be detected by ERT.

WHAT IS ERT?

ERT (electrical resistivity tomography, also known as electrical impedance tomography) is an adaptation of the more standard surface DC resistance surveys that involve electrodes buried just under the ground surface. ERT adds a level of complexity not only by changing the array configuration to that of a cross-borehole style, but also by incorporating a finite element mesh analysis and sophisticated algorithms for inversion of the collected data. This ultimately results in much better resolution than surface surveys, greater spatial coverage, and accuracy that extends to greater depth.



Figure D-2: Resistivity in ohm-m versus temperature in degrees Celsius.

An ERT survey is set up by emplacing colinear electrodes, called vertical electrode arrays (VEAs), into boreholes surrounding the area to be imaged. Each VEA is typically grouted into place, resulting in a permanently placed array. These electrodes are each independently wired to the surface data collection system. To collect data, one pair of adjacent electrodes acts as a transmitter of electric current, while pairs of electrodes in the same hole and in neighboring holes act as receivers. Data are collected between every possible combination of transmitter-receiver pairs between all neighboring boreholes. Data are then inverted using an algorithm to calculate the resistivity structure that best fits the measured voltages (see the following subsection, ERT Data Reduction). The resistivity cross sections produced are subsurface image planes that can be viewed in a two-dimensional fence diagram between wells, or as combined into a three-dimensional structure, depending on which optimally displays the target. During interpretation of these images, the resistivity signature is correlated to known processes, such as steam location, for tracking the steam migration.

The result is a near realtime geophysical technology for environmental monitoring and site characterization. Data collection is automated and relatively fast, typically taking less than 1 day to collect and image a site containing many VEAs. The expected resolution of ERT is around half of the vertical electrode spacing (typically 2 to 3 ft [0.6 to 0.9 m]). Surface electrodes may be added to improve near-surface resolution.

ERT has two advantages over most other geophysical methods. First, it is probably the only geophysical method applied to environmental sites where the data are routinely collected and interpreted fully three-dimensionally. The ERT methodology lends itself to the collection of large data sets using automated, relatively inexpensive equipment. Most other geophysical methods that would be used for monitoring have a limitation on the quantity of data that can be collected in a short period of time and, in general, are labor intensive with equipment that is currently available. The second advantage of ERT is that the electrodes are permanently placed in the subsurface, ensuring a high level of repeatability of the measurements, making the method suited for monitoring subsurface changes over time.

ERT DATA REDUCTION

Using an inverse algorithm, the resistivity structure that best fits the measured voltages is calculated. This method uses a finite element mesh design with typically two elements between electrodes. Thus the accuracy of images generated by ERT is said to be usually one-half of the electrode spacing. The inversion code calculates a model of the smoothest resistivity distribution that fits the data within estimated standard deviations using an inversion of the solution. More information is provided in LaBrecque and Yang (2000) and in LaBrecque et al. (1995).

The inversion produces a solution that is non-unique, meaning that several different interpretations could result from the collection of one data set. This is usually not a problem if experienced geophysicists are interpreting the data and if as many known factors are accounted for, facilitating the distinction between true anomalies and spurious artifacts. A strong difference inversion code makes use of the high degree of repeatability of ERT data, thereby reducing the amount of inversion artifacts.

ERT CASE STUDIES

ERT has become an effective technology for in situ monitoring of many processes, such as water movement in the vadose zone (Yang 1999), air-sparging, joule (electrical) heating, remediation processes (Ramirez et al. 1993; LaBrecque et al. 1998) and steam injection (Newmark et al. 1994; Southern California Edison 2000). Two examples are presented in the following subsections.

Thermal Remediation Monitoring at Portsmouth, Ohio

A pilot study was conducted at the X-701B Area at the Portsmouth Gaseous Diffusion Plant. During the study, steam and air were injected into the subsurface through wells, and water and vapor were extracted. The steam delivered energy to the target area, heating it to steam temperature, thereby accelerating the removal of nonaqueous-phase liquids.

The treatment area was located at the west end of a trichloroethene (TCE) plume emanating from a holding pond. TCE moved through permeable fine sands, silts, and clays of the Minford and Gallia Members and along the upper interface of a nearly impermeable shale layer, the Sunbury Shale. The contamination was present within the top of the Sunbury Shale. TCE levels in the upper Sunbury were as high as 1,600 mg/kg, generally 10 to 100 times those in the overlying layers. Steam was injected via shallow wells, into permeable, unconsolidated sediments (Gallia and Minford), heating these layers advectively. The Sunbury Shale was heated by conductive movement of heat from these overlying layers. ERT was used to monitor the movement of heat throughout the site.

This project presented a number of challenges in terms of ERT data collection and processing. Large amounts of electric power are used at the site, creating very high noise levels. In addition, much of the site was covered with large metal containers of low-level radioactive waste. This created a source of "cultural interference" and made access difficult. The site also contained a large number of metal well casings and pipes. Next, the contrasts in background resistivity were very large. The background ERT image showed highly conductive zones in the Sunbury Shale and Berea Sandstone. These corresponded to pyritic zones within the two formations. As a result, the upper 2 m (7 ft) of the Sunbury displayed a nearly 100-fold gradient in resistivity. It should be noted that the primary zone of interest was a thin permeable zone immediately on top of the Sunbury Shale.

Figure D-3 shows a two-dimensional sequence of percent change images over 1 month between the same two ERT boreholes (6 and 7). Steam injection commenced a few days prior to February 18, 1999, from the right side of the plane. Here, the results are displayed as percent differences from the background of conductivity, which is the inverse of resistivity. The only area changing is that which is affected by the steam. The difference inversion clearly shows this progression of the steam heated zone along the base of the Gallia. Over time, the temperature of the surrounding formations began to increase due to the conductive flow of heat from the steam zone and the upward migration of steam into the upper Gallia.

Midway through the project, ERT images revealed a linear belt of cooler water in the target zone. Substantial water flow through this channel prevented steam migration into the channel. The images were used to identify ideal locations for supplementary steam injection wells. After these wells were installed and steam was injected, the heating of the target zone was completed.



Figure D-3: Percent difference of electrical conductivity between boreholes ERT06 and ERT07 at the Portsmouth X701-B site. Depth in meters.

Monitoring Infiltration at the Socorro-Tech Vadose Zone Facility, New Mexico

Researchers from Sandia National Laboratories, the New Mexico Institute of Technology, the University of Arizona, and SteamTech Environmental Services collaborated on an interdisciplinary research project to develop a hybrid hydrologic-geophysical inverse technique (HHGIT) for vadose zone characterization. This project was funded by the U.S. Department of Energy's (DOE's) Environmental Management Science Program. SteamTech developed a threedimensional stochastic inversion program for ERT data interpretation and a three-dimensional cokriging algorithm to estimate three-dimensional moisture content distribution from ERT data and neutron-derived moisture contents. SteamTech also provided ERT monitoring for the infiltration experiment at the Socorro-Tech Vadose Zone (STVZ) facility.

The STVZ site is located on the campus of New Mexico Tech in Socorro, New Mexico. Soil deposits at the site exhibit contrasting textures and sedimentary structures typical of deposits found in various DOE waste sites in western United States. The test site was installed in relatively unconsolidated, heterogeneous, fluvial deposits consisting of gravel, sand, and clays of fairly high hydraulic permeability. The near-surface layer consists of poorly consolidated sand and gravel. There are multiple interbedded clay, sand, and gravel layers between 2 and 6 m. The rest of the site consists of fine sands with variable contents of clay and iron oxides.

In addition to the ERT survey conducted at this site, dense arrays of hydrologic probes such as time-domain reflectometer (TDR) probes and tensiometers, along with arrays of surface and subsurface ERT electrodes, were installed at the STVZ site. Neutron probe measurements and EM-39 conductivity logs were obtained through 13 polyvinyl chloride (PVC) access tubes. Cross-borehole ground penetrating radar (XBGPR) surveys were carried out along a northeast-southwest transect through five PVC access tubes. There were a total of 8 VEAs and 36 surface electrodes installed at the site. ERT data provided the capability to construct three-dimensional images of the subsurface electrical conductivity, from which estimates of the subsurface geology, temporal moisture content distributions, and advancement of wetting front could be derived.

The experimental site was 10.5 m (34 ft) long, 10.5 m wide, and 13 m (43 ft) deep. A square infiltration pad of 3 x 3 m (10 x 10 ft), made out of 900 18-gauge medical needles, was installed in the center of the site. The infiltration began on March 11, 1999, with a constant flux of 2.7 cm/day (1.2 inches/day).

The three-dimensional conductivity percent difference images, shown in Figure D-4, provide a clear picture of the water movement in the vadose zone. A 30 percent cutoff for volume rendering was used for image construction. The downward growth and lateral development of the wetted area can be seen clearly from these difference images.

IMPROVEMENTS TO INVERSION CODE AND ELECTRODE DESIGN

Inversion Code

One of the difficulties in comparing images created using Occam's inversion is that the resolution of the image is dependent on the noise in the data. This problem is exacerbated in situations such as the one at Portsmouth where there are thin, strongly contrasting layers. The resolution is typically not great enough to see the smaller features.

However, the recent development of a new modeling code has helped reduce these problems (Yang and LaBrecque 2000). Prior inversion codes inverted the electrical potential data of each data set, including the background, and then difference images were created afterward. ERT data are very unique among the electrical methods due in part to the fact that they exhibit a high degree of measurement precision, or repeatability. In the new difference inversion code, the differences in data sets are inverted, rather than the data sets themselves. The data are fit more closely than in previous algorithms, and systematic errors tend to cancel out. The result is faster convergence and better resolution in the images.

FINAL THERMAL TREATMENT TECHNOLOGY CONCEPTUAL DESIGN McCormick and Baxter Superfund Site Appendix D 11/12/01 Page D-9



Figure D-4: Percent change of electrical conductivity at the STVZ site. The cut-off for the volume rendering is 30 percent.

W:\74206\0110.035\Appendix D.doc

The artifacts that result from using Occam's inversion in an environment of thin, strongly contrasting layers are shown in the top images of Figure D-5. These top images are data from the Portsmouth Gaseous Diffusion Plant; these resemble data from operations at the Visalia Pole Treatment Yard (Southern California Edison 2000). There is a general trend of a resistivity anomaly growing in size over time, but with the presence of other extraneous artifacts that could confuse a valid interpretation. Along the bottom are the same data as those at the top, but inverted using the difference inversion code. These bottom images show the clean, easily interpreted tomographs resulting from the difference inversion, which are the same images shown in color in Figure D-3.

The difference code of Yang and LaBrecque is a published code, with property rights belonging to SteamTech and Lawrence Livermore National Laboratory. This code may be made public in the future. There are most likely other difference codes available today that are similar to that of Yang and LaBrecque.

Electrode Design

Ensuring collection of good quality data with a high level of repeatability means removing systematic errors wherever possible. One place where errors can be easily remedied is in electrode design. Errors can result from electrodes that are designed inconsistently, have intermittent connections, or sometimes have short circuited or lost their connections entirely. These problems can be eradicated by designing electrodes as identically as possible, especially making sure the surface area of metal exposed is exactly the same for each electrode. Also, creating a robust wire connection to each electrode and protecting it from corroding substances, are essential for preventing the loss of electrical connection. Short circuits can be prevented by making sure there are no nicks on any of the connection wires and that any conductive bodies, such as cable shielding, are not emplaced along with the VEA.

DISTRIBUTED TEMPERATURE MEASUREMENT

Taking the temperature at various points along a borehole underground is very useful and complementary to ERT to further delineate the nature of the subsurface during thermal remediation. One method for taking temperature measurements is the use of thermocouples. Each thermocouple consists of a pair of TeflonTM-coated wires of dissimilar metals stripped back and welded together at the end in the borehole, with the free ends running up to the surface. The temperature received at the weld junction produces a voltage difference at the free ends, thus enabling temperature to be read at the surface for each weld location down-hole. Though the data are generally considered accurate, the disadvantages of using thermocouples are that the construction can be laborious, data collection is time consuming, and the probes can corrode, resulting in loss of data.



Figure D-5: Comparison of Occam's inversion (top) to difference inversion using percent difference conductivity images from Portsmouth X701-B site. Darker areas represent increased conductivity.

Another method for taking underground temperature that has been in the oil field industry for some time is the use of fiber optics. The method, known as distributed temperature sensing (DTS), makes use of an optical fiber that runs down-hole in the area where temperature readings are to be taken. An optical pulse sent down the fiber undergoes scattering, the strongest of which is called Rayleigh, which is the type used in telecommunications. Secondary to that is the Raman scattering, consisting of the Stokes and anti-Stokes components. The Stokes component has a longer wavelength and does not vary with temperature, while the anti-Stokes component, at a shorter wavelength, is temperature variant. The ratio of these signals, as measured in the backscatter, is used to determine the temperature at known intervals, which are determined by the length of the optical pulse. Typically, this laser pulse is 10 nanoseconds, resulting in a measurement interval of 1 m (Normann, Weiss, and Krumhansl 2001).

In a DTS installation, it is important above all else to keep water away from the fiber. Exposure to water, or any other source of hydrogen, enables free hydrogen to get inside the fiber and create hydroxyl group molecules, which causes irreversible damage to the fiber and decreases its lifespan. For this reason, fibers should be installed using high-pressure air or heat-transfer fluid.

One continuous length of fiber can measure temperature at 1-m intervals up to 5 km. Therefore, individual wells can be outfitted with fibers, each ending at the surface, or if there are a lot of wells needed at a site for temperature measurement, several can be daisy-chained together with one single length of optical fiber.

DTS is a proven technology for taking underground temperature readings in oil field and geothermal applications. Conditions during thermal remediation are not unlike the conditions experienced in these applications, making DTS a promising method for subsurface monitoring.

REFERENCES

Archie, G. E. 1942. *Electrical Resistivity Log as an Aid in Determining Some Reservoir Characteristics*. American Institute of Mining and Metallurgy (English translation) 146:54-62.

LaBrecque, D. J., Bennett, J., Heath, G., Schima, S., and Sowers, H. 1998. "Electrical Resistivity Tomography Monitoring for Process Control in Environmental Remediation." In *Proceedings of the SAGEEP, Chicago, Illinois, March 22–26, 1998.* pp. 613-622.

LaBrecque, D. J., G. Morelli, A. Ramirez, W. Dailey, and P. Lundegard. 1995. "Occam's Inversion of 3D ERT Data." In *Proceedings of the International Symposium on Three-Dimensional Electromagnetics 1995, Ridgefield, Connecticut.* pp. 471-477.

LaBrecque, D., and X. Yang. 2000. "Difference Inversion of ERT Data: A Fast Inversion Method for 3-D In situ Monitoring." In *Proceedings of the Symposium on the Applications of Geophysics to Engineering and Environmental Problems, Environmental and Engineering Geophysical Society*. pp. 907-914.

Llera, F. J., M. Sato, K. Nakatsuka, and H. Yokohama. 1990. "Temperature Dependence of Electrical Resistivity of Water-Saturated Rocks." *Geophysics* 55:576-585.

Newmark, R. L. et al. 1994. *Demonstration of Dynamic Underground Stripping at the LLNL Gasoline Spill Site.* Final Report UCRL-ID-116964. 4 vols. Lawrence Livermore National Laboratory, Livermore, California.

Normann, R., J. Weiss, and J. Krumhansl. 2001. "Development of Fiber Optic Cables for Permanent Geothermal Wellbore Deployment." In *Proceedings of the Twenty-Sixth Workshop on Geothermal Reservoir Engineering, Stanford University.*

Ramirez, A., W. Daily, D. LaBrecque, E. Owen, D. Chesnut. 1993. "Monitoring an Underground Steam Injection Process Using Electrical Resistance Tomography." *Water Resources Research* 29:73-87.

Southern California Edison. 2000. Full-Scale Remediation of Visalia Pole Yard.

Yang, X. 1999. Stochastic Inversion of 3-D ERT Data. Ph.D. dissertation. Department of Mining and Geological Engineering, University of Arizona, Tucson, Arizona.

Yang, X., and LaBrecque, D. J. 2000. "Estimation of 3-D Moisture Content Using ERT Data at the Socorro Infiltration Site." In *Proceedings of the SAGEEP, Arlington, Virginia, February 20–24, 2000.*

APPENDIX E

Slough Dewatering Evaluation

SLOUGH DEWATERING EVALUATION

PURPOSE

Technical discussions regarding three thermal treatment scenarios for the McCormick and Baxter site raised concerns about the potential for mobilization of non-aqueous phase liquids (NAPLs) into Old Mormon Slough. Addition discussions focused on the ability to place injection and extraction wells in targeted subsurface treatment zones under the slough. Damming and dewatering of the slough was discussed at the EPA/USACE kick-off meeting on March 7, 2001, as a method to address these concerns. URS Corporation (URS) was asked to evaluate the capital and operation and maintenance (O&M) costs for this effort for presentation at a later date. This appendix is a summary of the information presented by URS at Meeting #1 on April 5, 2001.

URS evaluated the capital costs of constructing a sheetpile dam across Old Mormon Slough and building a dewatering system to remove water from an approximately 2,800-foot section of the slough for up to 5 years. The following information was developed for the April 5, 2001, presentation.

CAPITAL COST OF TEMPORARY DAM IN OLD MORMON SLOUGH

URS evaluated the capital cost of constructing a temporary sheetpile dam across Old Mormon Slough, which is documented in Table E-1. A review of existing documents for the site yielded a prior assessment by the Pacific Northwest National Laboratory (PNNL 1998), which was compared to URS' independent evaluation. The order-of-magnitude capital cost estimated by URS ranged from \$703,000 to \$943, 000. The PNNL estimate was \$859,000.

CAPITAL COST OF DEWATERING SYSTEM

URS developed an order-of-magnitude cost estimate for constructing a dewatering system to remove water from the inside face of the temporary sheetpile dam (estimated to be 56 million gallons [MG]) and to keep the slough dewatered for 5 years (4 years of active thermal treatment in the McCormick and Baxter site and the first year of cool-down). Estimates by PNNL (1998) were used for inflow to the slough from groundwater recharge (0.84 MG per day), and URS estimated that stormwater inflow from surface runoff and permitted/unpermitted discharge pipes in the slough contributed an average of 0.2 MG per day. Long-term dewatering efforts are assumed to be 1 MG per day or a total of 365 MG per year. The order-of-magnitude capital cost of the dewatering system is estimated by URS to be \$333,000 (Table E-2).

O&M COST FOR SLOUGH DEWATERING

URS estimated the labor, equipment, and material costs of dewatering the slough for 5 years (Table E-3). This estimate is for pump operation and maintenance only; no water treatment costs prior to discharge were included in this evaluation. The total cost for 5 years of dewatering system O&M is estimated to be \$2,350,000.

WATER TREATMENT COST FOR SLOUGH DEWATERING

Water treatment requirements and costs, if any, are a key factor in determining if dewatering the slough is feasible for the site. The removal of water from inside the dam poses a risk of remobilizing contaminants of concern during dewatering and during the thermal treatment. Technical input received during the kick-off meeting suggested that direct pumping from one side of the dam to the other would not be allowed, and the discharge would likely require a National Pollutant Discharge Elimination System (NPDES) permit. The level of treatment that would be required is not known at this time, and detailed evaluations would be required to determine the actual costs of a suitable treatment system.

Based on the estimated costs of the dam and dewatering system construction, URS assumed that the costs of direct discharge, reflecting dewatering infrastructure and equipment O&M costs, are \$0.0019 per gallon (total capital and O&M costs divided by the total volume of water discharged from initial drawdown of the slough and 5 years of continuous dewatering) or about \$1,900 per MG discharged. Costs increase as the level of treatment increases. For comparative purposes, URS assumed costs of \$3,000 per MG for minimal physical treatment (settling, filtration) prior to discharge. The addition of physical and chemical treatment (targeting NAPL and select contaminants of concern) is expected to increase costs to \$10,000 per MG. Treated discharge quality equivalent to a publicly owned treatment work (POTW) was assumed to be \$60,000 per MG. Subsequent discussions with the city of Stockton suggest these costs would be more than \$104,000 per MG under the commercial fee structure for its treatment system; however, the local system does not have the capacity to accept this discharge volume.

Table E-4 is a comparison of the estimated costs for each level of treatment prior to discharge, including initial dewatering of the slough (56 MG) and 5 years of continuous dewatering (1,825 MG). The cost of pumping only (no treatment) is estimated to be \$3.6 million; addition of physical treatment (settling, filtration) increases costs to \$5.6 million. Additional treatment increases costs to levels of between \$18.8 million (minimal treatment such as settling, oil-water separation, and filtration) and \$112.9 million (treatment comparable to a POTW).

CONCLUSIONS

The information presented in this appendix was discussed in Meeting #1 (April 5, 2001). The potential capital and O&M costs for dewatering the slough are considered too high when compared to the incremental costs of addressing concerns about injection/extraction well placement and NAPL remobilization by other means. Dewatering of the slough will not receive further consideration in the conceptual design analysis.

REFERENCE

Pacific Northwest National Laboratory (PNNL). Surface Water—Sediment Feasibility Study Report for the McCormick and Baxter Superfund Site, Stockton, California. July 1998.

Table E-1: Capital Cost of Temporary Dam in Old Mormon Slough

Description: Evaluate the capital and operation/maintenance (O&M) costs of dewatering Old Mormon Slough for up to 5 years during active thermal remediation by steam and six-phase heating of subsurface soil and groundwater zones at the McCormick & Baxter site. Assume construction of a temporary sheetpile dam, a long-term dewatering system, and operation for 4 years. The average depth of the slough is approximately 15 feet, and the width is 180 feet to 200 feet at the proposed dam location. The dam will isolate a 2,800 linear foot section of the slough. Prepared by: Date: Harry Ehlers, URS Corporation June 21, 2001

Capital (Costs
-----------	-------

		URS Estimate						3	
Cost Item	Description/Basis for Costs	Units	Quantity	Unit Cost	Cost Item Price	Units	Quantity	Unit Cost	Cost Item Price
1.0	Mobilization/demobilization (10 - 15% of construction)	LS	1	\$45,000 - \$85,000	\$45,000 - \$85,000	LS	1	\$73,000	\$73,000
2.0	(URS: 40 feet deep, 250 feet wide; key into shore, 20 feet above sediment and 20 feet below sediment; PNNL - 60 feet deep, 230 feet wide)	SF	10,000	\$25 - \$30	\$250,000 - \$300,000	SF	13,800	\$20	\$276,000
3.0	Geotextile & 30-mil Liner (URS: place at inside face of the sheetpile, 30 feet wide, 250 feet long)	SF	7,500	\$2.50 - \$3.50	\$18,750 - \$26,250	LS	1	\$35,000	\$35,000
4.0	Rock, 6-inch or larger (URS: place on inside face of sheetpile)	CY	1,480	\$15 - \$25	\$22,225 - \$37,040				
5.0	Demolition, remove sheetpile (URS: 40 feet deep, 250 feet wide; PNNL: 60 feet deep, 230 feet wide)	SF	10,000	\$15 - \$20	\$150,000 - \$200,000	SF	13,800	\$13	\$179,400
				Subtotal	\$485,975 - \$648 290			Subtotal	\$563,400
	Engineering (URS: \$35,000 - \$50,000; P	NNL, 159	% of subtota	l)	\$35,000 - \$50,000				\$73,000
				Subtotal	\$520,975 - \$698,290				\$636,400
		Cons	struction con	tingency (30%)	\$156,300 - \$209.500				\$190,920
	Owner's cost (administra	ition, cons	struction ma	nagement - 5%)	\$26,000 - \$35.000				\$31,820
				Capital Cost	\$703,300 - \$942,800			Capital Cost	\$859,140

Table E-2: Capital Cost of Dewatering System Prepared by: Harry Ehlers, URS Corporation Date: June 21, 2001

Description: Evaluate the capital and operation/maintenance (O&M) costs of dewatering Old Mormon Slough for up to 5 years during active thermal remediation by steam and six-phase heating of subsurface soil and groundwater zones at the McCormick & Baxter site. Assume construction of a temporary dewatering system and long-term dewatering system operation for 4 years. The average depth of the slough is approximately 15 feet, and the width is 180 feet to 200 feet at the proposed dam location. Assumptions are as follow:

- The dam will isolate a 2,800 linear foot section of the slough, 15 feet deep, average 180 feet wide. Calculate as square channel - 7.56 million cubic feet or 56.0 million gallons.
- Use recharge estimates by PNNL 0.84 million gallons per day; add 0.2 million stormwater runoff from surrounding area, permitted and unpermitted stormwater discharges to Old Mormon Slough.
- 3.0 Assume total dewatering effort of 1.0 million gallons per day or 365 million gallons per year.
- Assume dewatering occurs for 5 years 4 years of active thermal treatment and one year of the cool-down period; operate 24 hours per day, 7 days per week.
- 5. No water treatment is included in this estimate; see Table E-4.

Capital Costs

				URS Estimate	•
Cost Item	Description/Basis for Costs	Units	Quantity	Unit Cost	Cost Item Price
1.0	Pump Building, temporary structure, 40 by 60 feet, Sprung	SF	2,400	\$20	\$48,000
2.0	Concrete slab, 40 by 60 feet, 6-inch, moderate reinforcement	SF	2,400	\$5	\$12,000
3.0	Equipment pads, 200 square ft, 12-inch, heavy reinforcement	SF	200	\$10	\$2,000
4.0	Intake structure in slough	LS	1	\$20,000	\$20,000
5.0	Pipe, 8-inch DIP	LF	300	\$60	\$18,000
6.0	Discharge structure to slough, downstream of temporary dam	LS	1	\$10,000	\$10,000
6.0	Misc. fittings, valves	LS	1	\$2,000	\$2,000
7.0	Pump & motor, duplex, 700 gpm, with controls and misc piping	LS	1	\$80,000	\$80,000
8.0	Demolition of dewatering system	LS	1	\$20,000	\$20,000
8.0	Mobilization/demobilization	LS	1	\$10,000	\$10,000
				Subtotal	\$222,000
		Eng	ineering (15	% of subtotal)	\$33,300.0
	Constru	ction man	agement (10	% of subtotal)	\$22,200.0
	Continge	ncy (scope	e and bid, 25	% of subtotal)	\$55.500.0

Capital cost

\$333,000

Table E-3: O&M Cost for Slough Dewatering

Prepared by: Harry Ehlers, URS Corporation Date: June 21, 2001

Description: Evaluate operation/maintenance (O&M) costs of dewatering Old Mormon Slough for up to 5 years during active thermal remediation by steam and six-phase heating of subsurface soil and groundwater zones at the McCormick & Baxter site. Assumptions are as follow:

- 1. Initial dewatering of slough assume 56.0 million gallons (MG).
- Assume total dewatering effort of 1.0 million gallons per day or 365 million gallons per year.
- 3. Assume dewatering occurs for 5 years 4 years of active thermal treatment and one year of the cool-down period; operate 24 hours per day, 7 days per week.
- 4. Assume a O&M lead (equipment operator), \$60 per hour, and laborer, \$45 per hour; assume operations are concurrent with thermal treatment operations, allocate 0.25 full-time operator for each shift, 0.5 laborer per shift composite crew rate is \$37.50 per hour.

Operation and Maintenance (O&M) Costs

		URS Estimate					
Cost Item	ion/Basis for Costs	Units	Quantity	Unit Cost	Cost Item Price		
1.0	Labor, composite crew, 3 shifts per day, 7 days/week, 365 days/ week, 5 years.	MHR	43,800	\$37.50	\$1,642,500		
2.0	Small tools, disposables, 2% of labor	LS	1	\$32,850	\$32,850		
3.0	Equipment repair & parts, 10% of equipment cost (\$130,000) each year, 5 years	LS	1	\$65,000	\$65,000		
				Subtotal	\$1,740,350		
		Constructi	ion management	(10% of subtotal)	\$174,035.0		
		Contingenc	y (scope and bid,	25% of subtotal)	\$435,087.5		
			O	- &M cost, 5 years	\$2,349,473		

Appendix E 11/12/01 Page E-6

Description: The pumping costs for dewatering of Old Mormon Slough are estimated to be \$3.54 million for 5 years of operation (\$0.86 million for dam [PNNL], \$0.33 million for dewatering system, and \$2.35 million for 5 years of O&M), handling 1 MG per day or 1,825 million gallons of water over the operating period. This is an equivalent per gallon cost of \$0.0019 per gallon or \$1,900 per million gallon. This does not include treatment of the water to reduce suspended sediment loads or to remove contaminants entrained in the water by dewatering or the thermal treatment operations. For comparison purposes, a range of treatment costs are shown in the table to show the potential costs of handling and treating this water:

Cost Item	Quantity	Pumping	Phys. Only	Phys/Chem	POTW
	(MG)	(\$1,900 per MG) ¹	(\$3,000 per MG) ²	(\$10,000 per MG) ³	(\$60,000 per MG) ⁴
Initial dewatering of Old Mormon Slough	56	\$106,400	\$168,000	\$560,000	\$3,360,000
5-years of dewatering from slough	1,825	\$3,467,500	\$5,475,000	\$18,250,000	\$109,500,000
	Dewatering total by method	\$3,573,900	\$5,643,000	\$18,810,000	\$112,860,000

Notes:

1. Pumping cost, \$0.0019 per gallon or \$1,900 per million gallons.

- 2. Minimum physical treatment filtration or sedimentation, no chemical
- or biological treatment assume \$0.003 per gallon or \$3,000 per million gallons 3. Minimum physical/chemical treatment - assume \$0.01 per gallon or \$10,000
 - per million gallons.
- 4. Moderate physical/chemical treatment, such as publicly owned treatment works (POTW) \$0.06 per gallon or \$60,000 per million gallons

Appendix E 11/12/01 Page E-7

APPENDIX F

Memorandum: Evaluation of Angled Drilling Under Old Mormon Slough McCormick and Baxter Superfund Site, Stockton, California





Subject:	Evaluation of Angled Drilling under Old Mormon Slough McCormick & Baxter Superfund Site, Stockton, CA
From:	Harry Ehlers
To:	Steve Carroll, SteamTech/Denver CO Jack Spear, URS/Cranford NJ
Date:	July 16, 2001

Purpose

Technical discussions regarding three thermal treatment scenarios for the McCormick & Baxter site raised concerns about the potential for mobilization of non-aqueous phase liquids (NAPLs) into Old Mormon Slough. Additional discussions focused on the ability to place injection and extraction wells in targeted subsurface treatment zones located under the slough. Use of angled borings to position the wells was discussed at the project kick-off meeting on March 7, 2001. This memorandum summarizes information obtained regarding the feasibility of using angled wells to reach the depths targeted by Steam Tech for subsurface injection and extraction.

Background

The three thermal treatment scenarios include treatment of the North – Main Processing Area (N-MPA), which extends under Old Mormon Slough. Each scenario includes installation of steam injection wells and electrical resistance heating electrodes at depths below the B-C aquitard, at depths of -120 feet or more below mean sea level (MSL). The angles required for installation of these wells are generally less than 30 degrees from vertical, with several approaching 40 degrees. The maximum length of these borings are under 300 linear feet.

Discussion

Drilling contractors were contacted by Steam Tech and URS personnel to determine if the boring angles anticipated for the under-slough wells and electrodes posed a concern for construction or operation. Lane Christiansen (Fontana, CA) provided budgetary numbers for injection/extraction well construction, including the angled borings. Boart Longyear (Dayton, NV) also provided technical feedback to Steam Tech regarding construction techniques and maximum angles for conventional drilling equipment.

Based on these discussion, rotary (mud or air) and sonic drilling equipment appear capable of installing these wells at the angles anticipated for the thermal treatment scenarios. Horizontal boring equipment, typically used for trenchless utility and horizontal well installation, were not considered appropriate for the planned installations. Phone memos regarding this input are enclosed as attachments to this memorandum.



"Steve Carroll" <carroll@steamtech.c om>

To: Harry_Ehlers@URSCorp.com cc: Subject: RE: angled drilling at McCormick and Baxter site

07/02/01 09:22 AM

Good Morning Harry,

< A

further to our telephone conversation of Friday afternoon, I have made some enquiries and found that San Francisco - Sacramento area environmental drilling contractors typically had doubts about drilling to depths of 300 feet at the angles of interest. On the face of it, this might require some changes to the design, possibly with additional, deeper, drilling from a barge in the slough.

I am assured by Patrick Langan (775-246-0296) of Boart Longyear in Dayton, NV, that they would anticipate no difficulty in drilling to these depths at angles as low as 45 degrees using rotary or sonic rigs with down-hole casing advance and that 4 or 6-inch casing and screen could be installed. You may wish to talk to him.

Let me know if I can be of any assistance.

Steve Carroll

JUN-21-2001 THU 03:50 PM SENT BY:URS CONST SERVICES

June 12, 2001

(661) 3226552 Fax (908) 653-1313 Fax

John E. Spear URS Corporation 12 Commerce Drive Cranford, NJ 07016-1101

Tim Cechini Lane Christiansen Fontana, CA

Re: McCormick and Baxter Superfund Site Stockton, CA Drilling Pricing

Dear Tim:

molion Well

450

450

sumps 725

As we discussed in early May we need budget pricing for the drilling and installation of wells at the referenced site. We are working on an Engineers estimate for the Feasibility of this option under contract with the USACOE.

The pricing could be broken down in the following unit prices:

Unit Price 36,851 LFT 1. Drilling wells, 12" dia. (288 Wells) These are extraction and injection wells with 6" casing/screens all Steel. Note that 15 of these wells will be drilled off barge in the Old Mormon Slough. Also 15 of these wells will be drilled at an angle from the bank under the Slough. 44 EA 2. Install URS furnished Electrodes These electrodes will go in the bottom of some of the 288 walls above. 124 EA 3. Install URS furnished Extraction Fumps 9000 4. Install URS furnished Thermocouples are $40^{\frac{99}{2}}/lft \rightarrow 38$ EA These thermocouples (VEA's) will be installed in 4" dia holes and grouted solid. Total drilling for these 38 will be 8,560 vertical feet. 288 EA 5. 12" Protective Case and Well head up to flange 289 EA 6. Install URS furnished Down Hole Instruments 288 EA 7. Well Development 208 Extraction and Injection wells. Monitoring wells below 4 EA 8. Pump tests

2

9. Monitoring wells (39 each with 4" SS Piping/Screens)5,580 LFT ______ Complete including drilling, developing, piping, locking caps,

All wells above would include packing, grout etc required in ea. Well.

.. .

• • •

٠.

If you have any questions please call me in my NJ office at 908-653-3154 or 908-653-1300. My fax number in NJ 18 908-653-1313.

Cordially, M John E. Spear